

APPENDIX B

Modelling Technical Report

WINCHESTER SOUTH PROJECT

Groundwater Modelling Technical Report

Prepared for:
Whitehaven Coal Ltd

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1 Introduction

SLR Consulting Pty Ltd (SLR) was engaged by Whitehaven Coal Limited (Whitehaven) to prepare a Groundwater Assessment as required for the Winchester South Project (the Project). For this purpose, numerical groundwater modelling is being undertaken to predict impacts of the Project on the local groundwater regime. The overall objectives of this modelling are to:

- assess the groundwater inflow to the mine workings as a function of mine position and timing;
- simulate and predict the extent of dewatering due to the Project and the level and rate of drawdown at specific locations; and
- identify areas of potential risk where groundwater impact management measures may be necessary.

Conceptualisation of the groundwater regime and the calibration of the model against observed data are key to achieving a reliable numerical model. Conceptualisation is a simplified overview of the groundwater regime (i.e. the distribution and flow of groundwater) based on available data and experience. Consistency between numerical model results and the conceptual understanding of the groundwater regime increases the credibility of the numerical model predictions. The conceptual model for the Project has been provided in **Section 5.7** of the Groundwater Impact Assessment report (SLR, 2022) prepared in accordance with the requirements of the EIS. Confidence in the numerical model is increased by calibration of numerical model results against observed data. A well calibrated model has demonstrated the ability to simulate groundwater levels that approximate observed levels at specific locations.

The numerical groundwater model for the Project builds on the groundwater model used in the groundwater impact assessments for the Moorvale South Project (SLR, 2019) and Olive Downs Project (HydroSimulations, 2018). The Moorvale South Project model adopted the Olive Downs Project model for the Olive Downs South and Willunga domains incorporating updates where necessary.

2 Model Construction and Development

2.1 Model Code

MODFLOW-USG Transport was used as the model code (Panday *et al.* 2013). MODFLOW-USG is the latest version of industry standard MODFLOW code and was determined to be the most suitable modelling code for accomplishing the model objectives. MODFLOW-USG optimises the model grid and increases numerical stability by using unstructured, variably sized cells. These cells take any polygonal shape, with variable size constraints allowing for refinement in areas of interest (i.e. geological or mining features).

Where previous MODFLOW versions restricted interlayer flow to vertical connectivity, MODFLOW-USG offers lateral connectivity between model layers. Lateral connectivity enables more accurate representations of hydrostratigraphic units, particularly those that pinch out, outcrop, or cross geological faults.

MODFLOW-USG is also able to simulate unsaturated conditions, allowing progressive mine dewatering and post closure rewetting to be represented by the model. For the Project model, vadose zone properties have been excluded, and the unsaturated zone was simulated using the upstream-weighting method.

Fortran code and a MODFLOW-USG edition of the Groundwater Data Utilities (Watermark Numerical Computing) were used to construct the MODFLOW-USG input files.

2.2 Model Extent and Mesh Design

The model extent has been updated from the Moorvale South Project model for the Project through the expansion of the original model domain into the north-west (**Figure 2-1**). Herein, this report will use the term “north-west model expansion” to refer to the additional area now included in the model. The model domain was updated so that boundary conditions are sufficiently distant from the Project to not affect the modelling results (i.e. no edge effects). Elsewhere along the model perimeter, boundary locations are consistent with those of the Moorvale South Project model.

The model encompasses the Project and elongation is in the direction of geological strike (north-west to south-east). At its widest extents, the model is approximately 65 kilometres (km) x 70 km. The model domain was selected based on the following considerations:

- The western and eastern boundaries are represented by the outcrop of the Back Creek Group, which is considered the regional low permeability basement for the purpose of this modelling.
- The northern boundary contains the primary aquifers being mined by the Project and is at least 10 km away from the proposed pits.
- The southern boundary is at least 35 km from the mining lease and is expected to be far outside the range of predicted Project related drawdown.

The above boundaries include surrounding mines listed in **Section 2.5** for cumulative impact assessment.

The area occupied by the model is large, resulting in the need for an unstructured grid. The unstructured grid comprises varying cell sizes allowing for refinement in areas of interest, reducing the model cell count to an optimal size. AlgoMesh (Merrick & Merrick, 2015) was used to construct the model grid and is presented in **Figure 2-1**.



The following features have been included in the grid design:

- The Isaac River is represented in the model with a 50 metre (m) Voronoi cell size constraint.
- Open cut mining for the Project is represented with a 100 m cell size constraint.
- Open cut mine areas for the Olive Downs Project have a 100 m Voronoi cell size constraint.
- Open cut mining at all other sites (Lake Vermont, Poitrel, Daunia, Caval Ridge, Peak Downs, Saraji and the Moorvale South Project) have a maximum cell size of 200 m.
- Longwall mining at Eagle Downs Mine has an oriented regular grid of 350 m width squares to represent longwalls. Proposed mining at Saraji East is represented similarly by 400 m squares.
- Faults are represented using a 100 m Voronoi cell constraint.

The active cell count for a layer encompassing the entire model domain is 72,700, which would result in over 1,000,000 cells. However, over the 14 model layers, pinch-out areas (where a layer is not present) in Layers 3 to 14 bring the total active cell count of the model to 787,789.

2.3 Model Layers

Topography within the model domain has been defined using numerous sources of varying accuracy. Data extents of the sources used to construct model topography are shown in **Figure 2-2**. High resolution (1 m) Digital Elevation Model (DEM) data, provided by Whitehaven, was used to define local surface elevation within the Project area. The DEM data is centred over the Project, and at maximum extents, extends approximately 26 km north-south and 29 km east-west.

Outside the extents of the DEM dataset for the Project, LiDAR data from the Moorvale South Project and the Olive Downs Project were used to define surface elevation, where available. In areas where datasets overlap, priority was given to the LiDAR data from the Moorvale South Project. Public domain DEM data sourced from Geoscience Australia (with 3m subtracted for consistency between datasets) was used to define topography in the remainder of the model domain.

The model domain is discretised into 14 layers, as listed in **Table 2-1**. Model layer extents (lateral and vertical) have been defined using data from the following sources:

- Whitehaven – the Project site geological model (as of November 2019);
- Whitehaven – Exploration drill hole logs;
- Whitehaven – the Project TEM alluvial surveys and slope break analysis;
- Peabody – Moorvale South Project groundwater model (2019), includes:
 - Peabody – Moorvale South Project site geological model;
 - Pembroke – Olive Downs Project site geology model and numerical groundwater model;
- CSIRO Regolith depth survey;
- Queensland Globe bore hole logs; and
- Queensland surface geology and basement geological maps.

Table 2-1 Model Layers and Thicknesses

Model Layer	Formation	Unit	Average Thickness (m)	Max Thickness (m)
1	Surface cover	Alluvium, colluvium, Tertiary basalt	8.4	37.0
2	Regolith	Tertiary and minor Triassic Clematis, weathered Permian, Tertiary basalt	21.9	221.4
3	Rewan Group	Triassic	119.4	658.4
4	Rangal Coal Measures	Leichhardt overburden	45.3	269.2
5		Leichhardt seam	4.9	5.5
6		Interburden	40.0	139.1
7		Vermont seam	3.9	5.6
8		Vermont underburden	25.5	170.4
9	Fort Cooper Coal Measures	Fort Cooper overburden	73.2	180.5
10		Fort Cooper seams (combined)	73.2	
11		Fort Cooper underburden	73.1	
12	Moranbah Coal Measures	Moranbah overburden	46.9	211.2
13		Moranbah seams (combined)	46.9	
14		Moranbah underburden	46.9	

The geological layering in the model is generally consistent with the Moorvale South Project model. Layering was updated to include the Project site-specific geology model, data from surrounding exploration drill holes and the updated alluvium extents. In the north-west model expansion, layers were constructed using data from CSIRO regolith depth surveys, exploration drill logs, Queensland Globe bore logs and average thicknesses where data was unavailable.

Model Layer 1 is fully extensive across the model with an assumed depth of 3 m for colluvium. Model Layer 2 is also fully present across the model area with a minimum thickness of 1 m. Base of weathering elevation from the site-specific geology model was used to define the elevation for base of model Layer 2 at the Project. Elsewhere, the Moorvale South Project model was used to define the base of model Layer 2. In the north-west model expansion, the base of Layer 2 was interpreted from CSIRO regolith survey depths and Queensland Globe bore log lithology data.

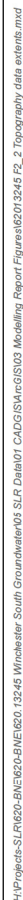
The underlying Triassic and Permian layers are present only to their outcrop extents, with some inference made for the presence of older units beneath the surface outcrop due to folding and faulting. The Back Creek Group is considered the regional low-permeability basement for the purpose of this modelling and defines the base of the model, and the western and eastern model boundaries.

It is not possible to represent every individual coal seam (typically <1 m thickness) in a regional groundwater model, therefore a “combined thickness” totalling the individual seam thicknesses for each relevant seam has been simulated. Site specific information for the Leichhardt and Vermont seams at the Project, Moorvale South Project and Olive Downs Project has been included in the model. Outside these sites, limited regional layer thicknesses information is available. The following values were used to define the combined seam thicknesses in the local geology at the Project:

- Leichhardt Seam thickness: 3.8 m
- Vermont Seam Thickness: 5.6 m

There is no additional data regarding thicknesses below the Rangal Coal Measures. As such, thicknesses from the Olive Downs Project model were used, with average thicknesses extrapolated out into the extended model area.

Model Layers 1 and 2 exist over the entire model extent. For other layers the minimum model layer thickness is 0.15 m. Model cells with thickness below this 0.15 m threshold are pinched out and removed from the model. **Table 2-1** presents the average and maximum thicknesses across the model domain for each layer.



2.3.1 Geological Faults

The modelling of faults has been updated from the Moorvale South Project model at the Project area through the inclusion of major regional and local scale faults, interpreted by SLR (2022) from the site-specific geology model. Mesh refinement (100 m) has been used along fault lines to allow for isolated changes of hydraulic properties along fault zones during calibration. Fault zones have been assigned to all model layers below model Layer 2 (base of regolith). **Figure 2-3** shows the locations of geological fault zones represented in the model.

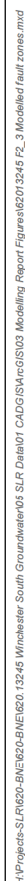
As discussed in Section 5.2.3 of the main Groundwater Impact Assessment report (SLR, 2022), faults in the vicinity of the Project are unlikely to act as conduits for flow given faulting in the Bowen Basin has been inactive for over 140 million years and drill core indicates that many fractures and faults have been “healed” with calcite and siderite.

Two drillholes that intersected faults in the Project area were redrilled for the purpose of packer testing to characterise hydraulic properties of the faults downhole. The packer test results are presented in Section 5.2.3 of the main Groundwater Impact Assessment report (SLR, 2022) and summarised as follows:

- Hydraulic conductivity results from bore WS3182 ranged from 9.48×10^{-4} to 1.02×10^{-3} m/day.
- Hydraulic conductivity results from bore WS3189 ranged from 6.93×10^{-5} to 2.07×10^{-3} m/day.

This hydraulic testwork aligns with the conceptualisation of faults in the vicinity of the Project. Notwithstanding, a broad range for hydraulic conductivity (1.00×10^{-6} m/day to 1.00 m/day) was conservatively used in the calibration process to allow the model to provide the best match to historical water level observations in the vicinity of the faults.

The calibrated hydraulic parameters for faults are discussed further in **Section 2.8**.



2.4 Model Stresses and Boundary Conditions

2.4.1 Regional Groundwater Flow

General Head Boundary (GHB) have been specified along the northern, eastern and southern model boundaries. A drain boundary condition was used along the western model boundary. It is appropriate to use this condition due to the abundance of open cut mining along the western boundary.

The GHB boundary condition is used to represent the regional flow into and out of the model area and has been assigned using GHB cells in all layers. Groundwater will enter the model where the head set in the GHB is higher than the modelled head in the adjacent cell and will leave the model when the water level is lower in the GHB. GHB conductance is calculated using the hydraulic conductivity and the dimensions of each GHB cells and is therefore variable in this model due to variable cell-size.

2.4.2 Watercourses

The Isaac River is the primary watercourse relevant to the Project. It is represented in the MODFLOW USG model using the Stream (STR) package. All other watercourses, as shown in **Figure 2-4**, are represented using the River (RIV) package. The rivers are set with the riverbed 1 to 11 m below the surrounding topography to represent the steep-banked incised channels.

Surveyed river stage data was available at several locations along the Isaac River. The closest gauging station to the site, located at Deveril, records monthly water levels which have been averaged for all available months and presented in **Table 2-2**, along with the annual average. These averages were extrapolated to provide continuous stage elevations used for the calibration and predictive model periods. Simulated stage heights are variable with time and fixed for each model stress period.

Table 2-2 Average Stage Heights (m) Used to Develop Transient Sequence

Station	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual Average
Isaac at Deveril	0.90	1.13	0.89	0.65	0.53	0.43	0.35	0.27	0.24	0.24	0.41	0.65	0.56

2.4.3 Rainfall Recharge

Rainfall recharge was applied to the model using the MODFLOW-USG recharge (RCH) package. The model distributed the recharge in zones across the model domain according to outcropping geology. The model assigned a proportion of annual rainfall to each of these zones. The proportion of rainfall entering the model as recharge varied through the calibration process.

The calibrated recharge rates are discussed in **Section 2.10**.

2.4.4 Evapotranspiration

The MODFLOW Evapotranspiration (EVT) package was used to simulate evapotranspiration from the groundwater system. Extinction depths were set to 2 m below ground across the model domain. Maximum potential rates were set using actual evapotranspiration values (from the Bureau of Meteorology), with the average value (600 millimetres per year [mm/year]) used as the transient calibration evapotranspiration rate.

2.4.5 Groundwater Use

Private groundwater pumping bores have not been included in the model due to lack of information regarding abstraction rates. Due to low groundwater abstraction across the model area, it is likely that the bores have very localised drawdowns and will not significantly impact model results.

2.4.6 Mining

The MODFLOW Drain (DRN) package is used to simulate mine dewatering in the model for the Project and surrounding mines. Boundary conditions for drain cells allow one-way flow of water out of the model. When the computed head drops below the stage elevation of the drain, the drain cells become inactive. This is an effective way of theoretically representing removal of water seeping into a mine over time, with the actual removal of water being via pumping and evaporation.

To simulate open cut mines in the model, drain cells are applied to all active layers from the surface to the base of the lowermost mined seam. The longwall extraction at Eagle Downs Mine and Saraji East is represented as drain cells in model Layer 13 (combined Moranbah Coal Measures) and the fracture zone extended up to Layer 8. The drain cells representing the surrounding mines were interpolated from mine schedule information available from relevant approval documentation and changes in aerial imagery over time.

2.4.6.1 Variation in Hydraulic Properties due to mining

For open cut mining, Hawkins (1998) and Mackie (2009) indicate that spoil and waste rock are more permeable than the undisturbed strata. Completed open cut mining areas will be backfilled with waste overburden as the extraction proceeds. Backfill was given uniform hydraulic conductivity of 0.2 metres per day (m/day), specific yield (Sy) of 0.05 and rainfall recharge set to 1 % of average rainfall. In the transient calibration and prediction model, backfill properties are applied two years behind the mine face.

The hydraulic properties were varied with time using the Time-variant materials (TVM) package of MODFLOW-USG Transport. For the underground mines, the hydraulic properties were changed with time in the goaf and overlying fractured zone directly above each longwall panel.

2.5 Calibration Model Simulation Period and Temporal Discretisation

Both steady-state and transient calibration models have been developed to meet the model objectives. For steady-state conditions, the average of observed conditions prior to 2006 were used. The transient calibration model was based on temporal pre-mining data at quarterly intervals from the end of the steady-state calibration (January 2006) until December 2021.

The groundwater model has been calibrated against measurements from 179 bores (including VWPs) across the Study Area. The dataset of calibration observations comprises site specific data from the Project area, measurements from the Moorvale South Project transient calibration model, which includes the bores from the landholder bore census survey (October 2017), newly added Queensland Globe bore monitoring observations and data from the Eagle Downs Mine and Moorvale South Project. Together, the steady-state and transient calibrations comprise 70 stress periods. **Table 2-3** summarises the calibration model stress periods and simulated active mine timings.

Table 2-3 Calibration model stress period setup

Calibration Period	Interval	Stress Period	Date (from)	Date (to)	Winchester South (OC)	Moorvale South (OC)	Olive Downs (OC)	Caval Ridge (OC)	Peak Downs (OC)	Saraji (OC)	Saraji East (UG)	Lake Vermont (OC)	Eagle Downs Mine (UG)	Poitrel (OC)	Daunia (OC)
Steady-State		1	Steady-state					x	x	x					
Transient	Quarterly	2	01-01-2006	02-04-2006				x	x	x					
	Quarterly	3	02-04-2006	02-07-2006				x	x	x					
	Quarterly	4	02-07-2006	01-10-2006				x	x	x					
	Quarterly	5	01-10-2006	31-12-2006				x	x	x					
	Quarterly	6	01-01-2007	02-04-2007				x	x	x				x	
	Quarterly	7	02-04-2007	02-07-2007				x	x	x				x	
	Quarterly	8	02-07-2007	01-10-2007				x	x	x				x	
	Quarterly	9	02-10-2007	01-01-2008				x	x	x				x	
	Quarterly	10	01-01-2008	01-04-2008				x	x	x				x	
	Quarterly	11	01-04-2008	01-07-2008				x	x	x				x	
	Quarterly	12	02-07-2008	01-10-2008				x	x	x				x	
	Quarterly	13	01-10-2008	31-12-2008				x	x	x				x	
	Quarterly	14	31-12-2008	01-04-2009				x	x	x		x		x	
	Quarterly	15	02-04-2009	02-07-2009				x	x	x		x		x	
	Quarterly	16	02-07-2009	01-10-2009				x	x	x		x		x	
	Quarterly	17	01-10-2009	31-12-2009				x	x	x		x		x	
	Quarterly	18	01-01-2010	02-04-2010				x	x	x		x		x	
	Quarterly	19	02-04-2010	02-07-2010				x	x	x		x		x	
	Quarterly	20	02-07-2010	01-10-2010				x	x	x		x		x	
	Quarterly	21	01-10-2010	31-12-2010				x	x	x		x		x	
	Quarterly	22	01-01-2011	02-04-2011				x	x	x		x		x	
	Quarterly	23	02-04-2011	02-07-2011				x	x	x		x		x	
	Quarterly	24	02-07-2011	01-10-2011				x	x	x		x		x	
	Quarterly	25	02-10-2011	01-01-2012				x	x	x		x		x	
	Quarterly	26	01-01-2012	01-04-2012				x	x	x		x		x	
	Quarterly	27	01-04-2012	01-07-2012				x	x	x		x		x	
	Quarterly	28	02-07-2012	01-10-2012				x	x	x		x		x	
	Quarterly	29	01-10-2012	31-12-2012				x	x	x		x		x	
	Quarterly	30	31-12-2012	01-04-2013				x	x	x		x		x	
	Quarterly	31	02-04-2013	02-07-2013				x	x	x		x		x	x
	Quarterly	32	02-07-2013	01-10-2013				x	x	x		x		x	x
	Quarterly	33	01-10-2013	31-12-2013				x	x	x		x		x	x
	Quarterly	34	01-01-2014	02-04-2014				x	x	x		x		x	x

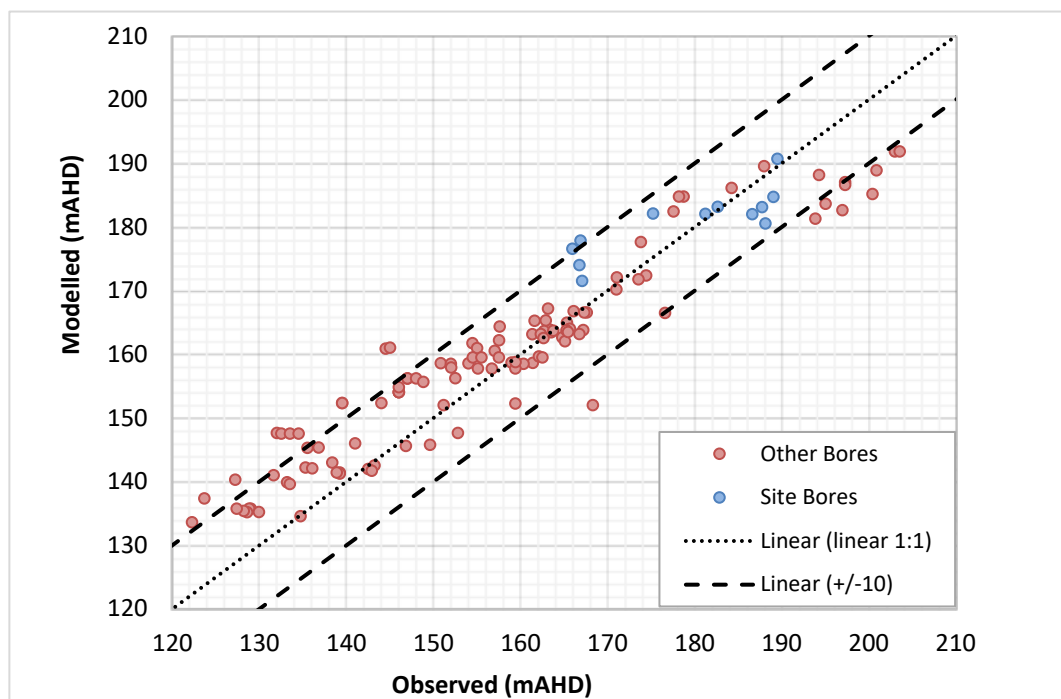
Calibration Period	Interval	Stress Period	Date (from)	Date (to)	Winchester South (OC)	Moorvale South (OC)	Olive Downs (OC)	Caval Ridge (OC)	Peak Downs (OC)	Saraji (OC)	Saraji East (UG)	Lake Vermont (OC)	Eagle Downs Mine (UG)	Poitrel (OC)	Daunia (OC)
	Quarterly	35	02-04-2014	02-07-2014				x	x	x		x		x	x
	Quarterly	36	02-07-2014	01-10-2014				x	x	x		x		x	x
	Quarterly	37	01-10-2014	31-12-2014				x	x	x		x		x	x
	Quarterly	38	01-01-2015	02-04-2015				x	x			x		x	x
	Quarterly	39	02-04-2015	02-07-2015				x	x			x		x	x
	Quarterly	40	02-07-2015	01-10-2015				x	x			x		x	x
	Quarterly	41	02-10-2015	01-01-2016				x	x			x		x	x
	Quarterly	42	01-01-2016	01-04-2016				x	x			x		x	x
	Quarterly	43	01-04-2016	01-07-2016				x	x			x		x	x
	Quarterly	44	02-07-2016	01-10-2016				x	x			x		x	x
	Quarterly	45	01-10-2016	31-12-2016				x	x			x		x	x
	Quarterly	46	31-12-2016	01-04-2017				x	x			x		x	x
	Quarterly	47	02-04-2017	02-07-2017				x	x			x		x	x
	Quarterly	48	02-07-2017	01-10-2017				x	x			x		x	x
	Quarterly	49	01-10-2017	31-12-2017				x	x			x		x	x
	Quarterly	50	31-12-2017	01-04-2018				x	x			x		x	x
	Quarterly	51	01-04-2018	01-07-2018				x	x			x		x	x
	Quarterly	52	01-07-2018	30-09-2018				x	x			x		x	x
	Quarterly	53	01-10-2018	31-12-2018				x	x			x		x	x
	Quarterly	54	31-12-2018	01-04-2019				x	x			x		x	x
	Quarterly	55	01-04-2019	01-07-2019				x	x			x		x	x
	Quarterly	56	02-07-2019	01-10-2019				x	x			x		x	x
	Quarterly	57	01-10-2019	31-12-2019				x	x			x		x	x
	Monthly	58	31/12/2019	30/01/2020				x	x	x		x		x	x
	Monthly	59	30/01/2020	01/03/2020				x	x	x		x		x	x
	Monthly	60	01/03/2020	31/03/2020				x	x	x		x		x	x
	Monthly	61	31/03/2020	01/05/2020				x	x	x		x		x	x
	Monthly	62	01/05/2020	31/05/2020				x	x	x		x		x	x
	Monthly	63	31/05/2020	30/06/2020				x	x	x		x		x	x
	Monthly	64	30/06/2020	31/07/2020				x	x	x		x		x	x
	Monthly	65	31/07/2020	30/08/2020				x	x	x		x		x	x
	Monthly	66	30/08/2020	30/09/2020				x	x	x		x		x	x
	Monthly	67	30/09/2020	30/10/2020				x	x	x		x		x	x
	Monthly	68	30/10/2020	30/11/2020				x	x	x		x		x	x
	Monthly	69	30/11/2020	30/12/2020				x	x	x		x		x	x
	Annually	70	30/12/2020	30/12/2021				x	x	x		x		x	x

2.6 Steady-state

Steady-state calibration was undertaken using the automated calibration utility PEST (Doherty, 2010) with 125 groundwater targets, including 12 bores and VWP's from the Project monitoring network. Manual parameter adjustment was then undertaken to validate that the calibrated parameters were consistent with the conceptual understanding of the hydrogeological system. Hydraulic conductivity, recharge and river/stream conductance were adjusted to achieve the steady-state calibration. Manual adjustments to the Isaac River stream conductance were made to maintain consistency between modelled stream behavior (i.e. gaining/losing river) and the conceptual understanding of the Isaac River. Vertical hydraulic conductivity (K_v) was calibrated as a factor of horizontal conductivity (K_v/K_x). Reduced vertical hydraulic conductivity is typically observed due to sedimentary layering throughout the sequence, and by aggregation of strata in a numerical model.

2.6.1 Statistics

A scattergram of observed vs simulated groundwater levels for the steady-state calibration targets is presented in **Figure 2-5**.



mAHd = metres Australian Height Datum.

Figure 2-5 Steady-state Calibration – Modelled vs Observed Groundwater Levels

The industry standard method to evaluate the calibration of the model is to examine the statistical parameters associated with the calibration. This is done by assessing the error between the modelled and observed (measured) water levels in terms of the root mean square (RMS) error. An RMS error is expressed as:

$$RMS = \left[1/n \sum (h_o - h_m)_i^2 \right]^{0.5}$$

Where:

- n = number of measurements
- h_o = observed water level
- h_m = simulated water level

The RMS error is considered to be the best measure of error, if normally distributed. The RMS error for the calibrated steady-state model is 6.49 m.

When considering if this achieved RMS is acceptable, the RMS should be assessed in the context of the range of the observed head changes over the model domain. If the ratio of the RMS error to the total head change in the system is small, the errors are only a small part of the overall model response. The total measured head change across the model domain is 122.5 m; therefore, the ratio of RMS error to the total head loss (i.e., the scaled root mean squared [SRMS] error) is 5.31%. This indicates a good calibration as it is within the Australian guidelines' indicator of 10% SRMS error (Middlemis *et al.*, 2001; Barnett *et al.*, 2012).

2.6.2 Water Balance

The water balance for the steady-state simulation is presented in **Table 2-4**.

Table 2-4 Steady-state Model Mass Balance

Component	Inflow (ML/d)	Percent of Total Inflow (%)	Outflow (ML/d)	Percent of Total Outflow (%)
Recharge (RCH)	4.74	50.64	-	-
ET (from GW) (EVT)	-	-	0.55	5.88
SW-GW Interaction Isaac River (STR)	1.91	20.41	6.83	72.97
SW-GW Interaction Minor Rivers (RIV)	-	-	0.61	6.52
Regional GW Flow (GHB)	2.71	28.95	1.32	14.10
Mines (DRN)	-	-	0.05	0.53
Storage	-	-	-	-
Total	9.36	100.00	9.36	100.00

ML/d = megalitres per day.

The water balance for the steady state calibration indicates that recharge is the largest inflow contributor to the groundwater system, providing 4.74 ML/d. Regional groundwater flow into the model domain is another net positive contributor of inflow to the groundwater system and contributes a net of 1.39 ML/d (i.e. difference between the regional groundwater inflow and outflow).

A net outflow of 4.92 ML/d from the model occurs due to baseflow seepage to the Isaac River (i.e. surface water and groundwater interaction in the Isaac River). This is the largest component of outflow from the model during steady state calibration. Other factors that contribute to outflow from the groundwater system are evapotranspiration (0.55 ML/d outflow), baseflow seepage to minor drainage systems (0.61 ML/d outflow) and groundwater take from mining activities (0.05 ML/d outflow). The mass balance error for the steady state calibration is within the 1% error threshold recommended by the *Australian Groundwater Modelling Guidelines* (Barnett *et al.*, 2012).

2.7 Transient Calibration

Automated calibration utility PEST and manual calibration were used to match the available transient water level data. In all, 26,820 target heads were established for 179 locations, including 20 bores and VWP's from the Project monitoring network, and 159 other registered bores as identified through bore censuses, the QLD Globe database and surrounding mine monitoring networks. PEST was used to adjust horizontal and vertical hydraulic conductivity, specific storage (Ss), specific yield, recharge and river/stream conductance in order to match the observed and simulated water levels. To begin each transient model calibration run, a steady-state simulation was undertaken. The steady-state heads for each calibration scenario were transferred into the transient calibration model as initial groundwater levels. This approach confirmed that initial conditions (steady-state groundwater levels) for the transient run were derived from the corresponding parameter set being applied in the transient simulation. Discrepancies between these two parameter sets would disrupt groundwater flow budgets as the transient version of the model settles to pseudo steady-state conditions outside the mining areas throughout the simulation.

2.7.1 Statistics

Figure 2-6 presents the observed and simulated groundwater levels as a scattergram for the initial steady-state and transient calibration (beginning 2006 to end of 2021). The scattergram indicates site bores have been adequately represented by the calibration model (simulated water levels typically within 10 m of observed).

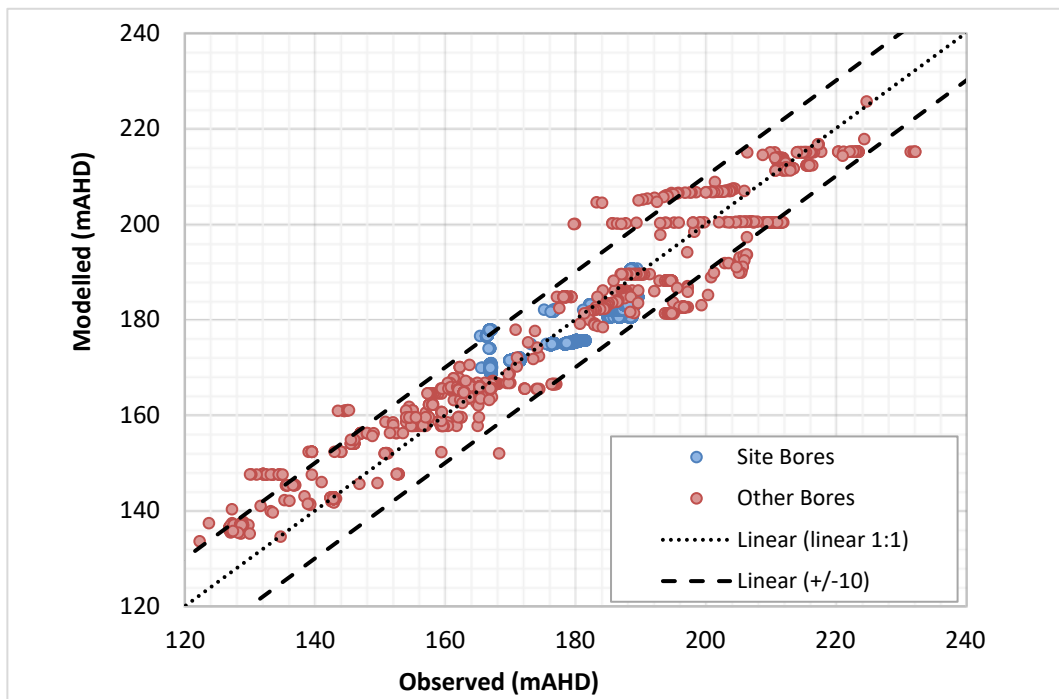


Figure 2-6 Transient Calibration – Modelled vs Observed Groundwater Levels

Calibration hydrographs, showing the fit between modelled and observed groundwater levels are presented in **Appendix A**. Seasonal water level fluctuations are to some extent replicated by the groundwater model. This can be seen in the hydrograph for bores such as 13040180, which intersects the Isaac River alluvium. For site bores R2008 and Winnet Bore, the hydrographs show the observed and simulated water levels generally align. R2008 and Winnet Bore are screened in the Vermont Seam and the Isaac River alluvium, respectively. Hydrographs at S series bores most notably S6, S8 and S10 show the model matches well the water levels in alluvial bores near the Olive Downs Project area. The average (arithmetic mean) residual for bores in layer 1 (alluvium and colluvium) across the entire model domain is 1.02 m, while the average residual for alluvial bores near the Project Area is -0.50 m. The average alluvial residual was calculated by taking the averaging the residual at each alluvial bore in the numerical groundwater model. This ensures all bores were weighted evenly, and the reported average was not skewed by bores with large numbers of observations. Observed measurements for Permian Coal Measure bores 162166 and 162172 are closely matched by the simulated water levels, indicating strong calibration at these sites.

Resulting statistics for the transient simulation are shown in **Table 2-5** and average residuals in each layer are shown in **Table 2-6**. Residuals have been calculated as the observed water level minus the modelled water level. The model SRMS error across all observations is 2.45%, again considered a good fit using statistical targets suggested by Middlemis *et al.* (2001) and Barnett *et al.* (2012).

For bores within the Project Area, the residual errors range from -11.25 m to 8.17 m, with an average residual of 0.02 m. The average residual was calculated as the average of the average residuals at each bore. The model results show a good balance between overprediction and underprediction of groundwater levels within the Project Area, as indicated by the small average residual. There is a high level of variability in observed water levels in the bores within the Project Area, which is considered to likely be a result of the complexity of the structural geology (i.e. faulting) in the vicinity of the Project. The residual error is resulting from the inability of the model to fully replicate this complexity. The model is a simplification of reality in this regard as the grid resolution will never be of the finite degree required to replicate all the structure.

The aim of the model calibration was to obtain a good fit to the regional spread of data, in order to replicate the regional groundwater gradients and to provide the best possible constraint to the model boundary conditions across the entire model domain. The calibration hydrographs and statistics indicate that a reasonable calibration has been achieved across the model domain, regardless of the discrepancies noted in the calibration for some of the bores within the Project Area.

The spatial distribution of residuals is shown in **Figure 2-7**. By examining the scatter distribution in **Figure 2-6**, the spatial distribution in **Figure 2-7** and the transient calibration summary in **Table 2-5**, the model is shown to demonstrate no significant tendency overall for over predicting or under predicting groundwater levels within the model domain. **Appendix B** contains a table of average, maximum and minimum residuals for each bore in the transient calibration.

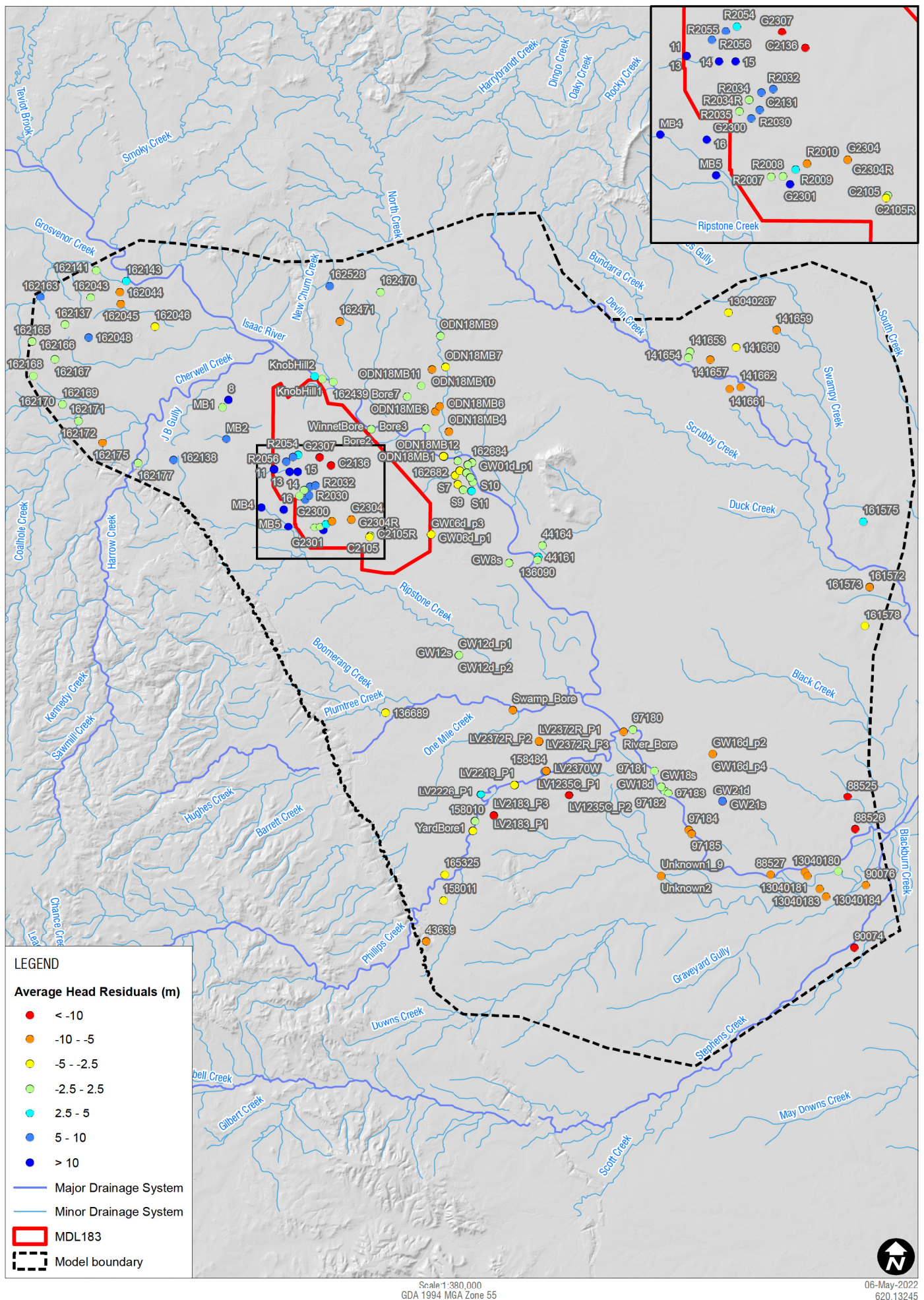
Table 2-5 Transient Calibration Statistics

Statistic	Value
Sum of Squares (m ²)	15977.63
Mean Sum of Squares (m)	9.03
Root Mean Square (m)	3.00
Scale Root Mean Square (%)	2.45
Root Mean Fraction Square (%)	0.48
Scaled Root Mean Fraction Square (%)	0.70
Sum of Residuals (m)	7206.30
Mean Sum of Residuals (m)	0.27
Scaled Mean Sum of Residuals (%)	0.22
Coefficient of Determination (tend to unity)	1.53
Number of Targets within ± 2 m	25807
Number of Targets within ± 5 m	26723
Number of Targets within ± 20 m	26820
Number of Targets above or below 20m	0

Table 2-6 Average Residual by Model Layer

Model Layer	Formation	Average Residual (m)	Number of Observation Targets
1	Alluvium, colluvium	1.02	6072
2	Regolith	3.85	123
3	Rewan Group	-2.39	32
4	Rangal Coal Measures	1.37	75
5		-1.89	6759
6		4.19	2725
7		-1.72	8441
8		3.74	1856
9	Fort Cooper Coal Measures	-5.95	47
10		0.59	22
11		8.89	11
12	Moranbah Coal Measures	-1.40	51
13		3.87	371
14		-5.87	3

Note: Negative residuals indicate modelled heads are higher than observed, positive indicates modelled heads are lower than observed.



Transient Calibration
Average Head Residuals (m)

FIGURE 2-7

2.7.2 Water Balance

The water balance for the transient simulation averaged over the duration of the calibration period is presented in **Table 2-7**. The maximum absolute mass balance error across all timesteps in the transient calibration was 0.04%, with cumulative absolute error remaining below 0.01%. This level of error is well within the recommended 1% error (Barnett *et al.*, 2012), indicating the model is stable and the numerical solution achieved is accurate.

Table 2-7 Transient Model Mass Balance

Component	Inflow (ML/d)	Percent of Total Inflow (%)	Outflow (ML/d)	Percent of Total Outflow (%)
Recharge (RCH)	5.76	17.01	-	-
ET (from GW) (EVT)	-	-	0.62	1.83
SW-GW Interaction Isaac River (STR)	15.68	46.31	16.64	49.14
SW-GW Interaction Minor Rivers (RIV)	-	-	0.67	1.98
Regional GW Flow (GHB)	2.58	7.62	1.66	4.90
Mines (DRN)	-	-	1.58	4.67
Storage	9.84	29.06	12.69	37.48
Total	33.86	100.00	33.86	100.00

The water balance for the transient calibration indicates that recharge was the largest net inflow contributor to the model, contributing an average of 5.76 ML/d to the groundwater system. Modelled net seepage outflow along the length of the Isaac River from the groundwater system is 0.96 ML/d. However, closer to the Project Area, the river is simulated as a losing system (net inflow during transient calibration equals 0.50 ML/d). Minor drainage systems contribute to a loss of approximately 0.67 ML/d from the groundwater system, and 0.62 ML/d of groundwater is removed due to evapotranspiration. Additionally, surrounding mines remove 1.58 ML/d of groundwater. Over the total duration of the transient calibration, there was a simulated gain in storage of approximately 2.85 ML/d.

2.8 Calibrated Hydraulic Parameters

Table 2-8 provides a summary of the calibrated values for horizontal and vertical hydraulic conductivity used in the model. Hydraulic zone distribution maps are provided as **Appendix C**.

Table 2-8 Calibrated Hydraulic Parameters

Model Layer	Formation	Unit	Horizontal Hydraulic Conductivity (m/day)	Anisotropy Kv/Kx
1	Alluvium	Surface cover	10.0	2.9×10^{-1}
1	Colluvium	Surface cover	1.0	2.5×10^{-1}
1 & 2	Tertiary Basalt	Tertiary basalt	1.5×10^{-1}	1.8×10^{-1}
2	Regolith	Tertiary and minor Triassic Clematis	7.3×10^{-1} to 2.0	9.7×10^{-3} to 6.7×10^{-2}
3	Rewan Group	Triassic	1.0×10^{-3}	2.7×10^{-2} to 1.0×10^{-1}
4	Rangal Coal Measures	Leichhardt overburden	1.0×10^{-5} to 2.2×10^{-1}	4.2×10^{-3}
5		Leichhardt seam	1.0×10^{-4} to 6.0×10^{-1}	3.8×10^{-2}
6		Interburden	1.0×10^{-5} to 1.0×10^{-3}	1.3×10^{-2}
7		Vermont seam	1.0×10^{-4} to 2.0×10^{-2}	9.1×10^{-2}
8		Vermont underburden	1.0×10^{-5} to 9.0×10^{-4}	8.0×10^{-2}
9	Fort Cooper Coal Measures	Fort Cooper overburden	1.0×10^{-5} to 1.0×10^{-3}	8.0×10^{-3}
10		Fort Cooper seam	1.0×10^{-4} to 4.3×10^{-4}	1.0×10^{-1}
11		Fort Cooper underburden	1.0×10^{-5} to 8.6×10^{-5}	1.0×10^{-1}
12	Moranbah Coal Measures	Moranbah overburden	1.0×10^{-5} to 9.6×10^{-5}	1.0×10^{-2}
13		Moranbah (Goonyella) seam	1.0×10^{-4} to 3.4×10^{-3}	5.0×10^{-2}
14		Moranbah underburden	1.0×10^{-5} to 9.4×10^{-4}	1.9×10^{-2}
all	Undivided Intrusives	Igneous intrusion	1.0×10^{-3}	1.4×10^{-2}
Below L02	Faults	All below Layer 2	5.0×10^{-5} to 8.3×10^{-3}	1.0×10^{-3} to 1.5×10^{-1}
-	Waste Rock/Spoil	-	2.0×10^{-1}	2.0×10^{-2}

Note: * upper hydraulic conductivity derived from depth of 20 m below surface and using depth formula

The hydraulic conductivity of the Permian interburden material in the model reduces with depth in order to reflect field observations. As the decrease of horizontal hydraulic conductivity within the interburden rock units is driven by an increase in overburden pressure, the relationship between horizontal hydraulic conductivity and depth is different from that of coal seams. The hydraulic conductivity for the interburden material is capped at a minimum of 1.0×10^{-5} m/day and the hydraulic conductivity of the coal seams is capped at a minimum of 1.0×10^{-4} m/day. The hydraulic conductivity of the interburden/overburden and coal seam layers decreases with depth according to Equations 1 and 2 (exponential):

Coal: $HC = HC_0 \times e^{(-0.015 \times \text{depth})}$ (Eq. 1)

Interburden: $HC = HC_0 \times e^{(-0.018 \times \text{depth})}$ (Eq. 2)

Where:

- HC is horizontal hydraulic conductivity at specific depth;
- HC_0 is horizontal hydraulic conductivity at depth of 0 m (intercept of the curve);
- depth is depth of the floor of the layer (thickness of the cover material); and
- slope is a term representing slope of the formula (steepness of the curve).

HC_0 was estimated in the calibration. It varies for the coal seams and for the interburden and overburden units in the model. The slope function and coefficient of the coal and interburden depth dependence equations were calibrated. The horizontal hydraulic conductivity against depth relationships for the interburden/overburden are presented in **Figure 2-8**, while the calibrated relationships for coal units are presented in **Figure 2-9**. The figure also presents the Olive Downs Project data (2018), Coffey (2014) Bowen Basin data trends and the Isaac Plains groundwater calibrated model parameters (Hansen Bailey, 2016).

Figure 2-9 shows lower hydraulic conductivities in Fort Cooper and Moranbah coal measures. It was not possible to represent every individual coal seam in Fort Cooper and Moranbah coal measures in the model. Therefore, a “combined thickness” totalling the individual seam thicknesses for each relevant seam has been simulated.

Figure 2-10 illustrates the range in horizontal hydraulic conductivity obtained from site testing and publicly available data. The data are focused on the key site units, being the alluvium, regolith, Rewan Group and the coal and interburden sequences of the Rangal Coal Measures. The data are compared to the horizontal hydraulic conductivity values used in the model. A depth dependence equation for the Rangal Coal Measures was used in the numerical groundwater model and therefore the calibrated hydraulic conductivity values vary across the model domain. Accordingly, the average value for the Rangal Coal Measures at the Project is displayed. As shown in **Figure 2-10**, the modelled horizontal hydraulic conductivity values are all within the range of field data.

The range of calibrated hydraulic conductivity values (5.0×10^{-5} m/day to 8.3×10^{-3} m/day) used to represent faults in the model domain (**Table 2-8**) is consistent with the packer test hydraulic conductivity range obtained for Project Area drillholes WS3182 and WS3189, both of which are confirmed to be intersecting faults. Packer test ranges for hydraulic conductivities across these two sites were between 6.93×10^{-5} m/day and 2.07×10^{-3} m/day (Hydrogeologist, 2019).

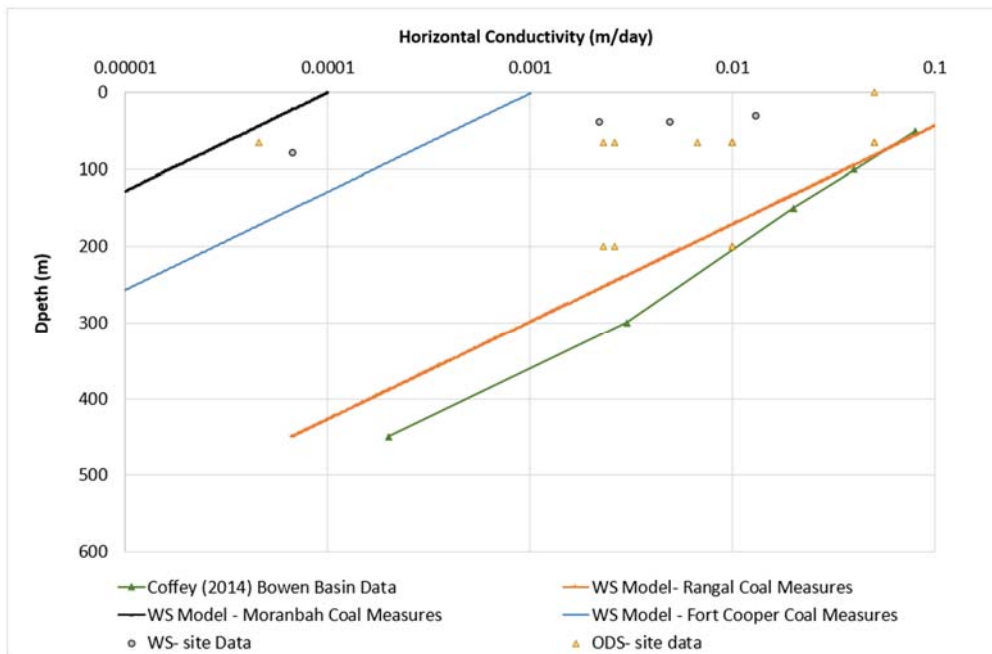


Figure 2-8 Hydraulic Conductivity vs Depth – Interburden/Overburden

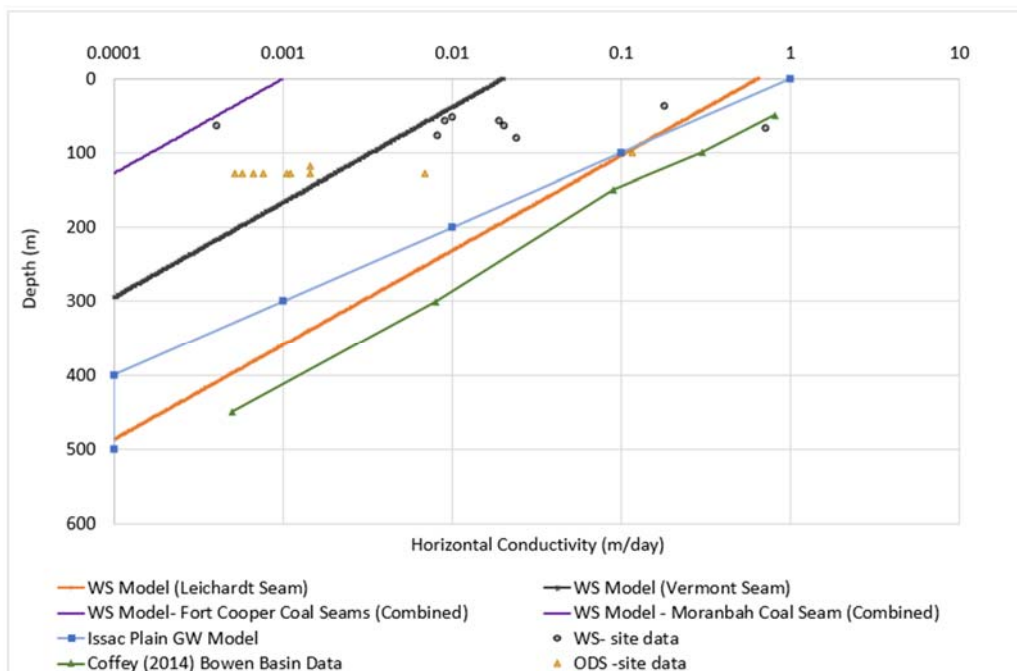


Figure 2-9 Hydraulic Conductivity vs Depth – Coal

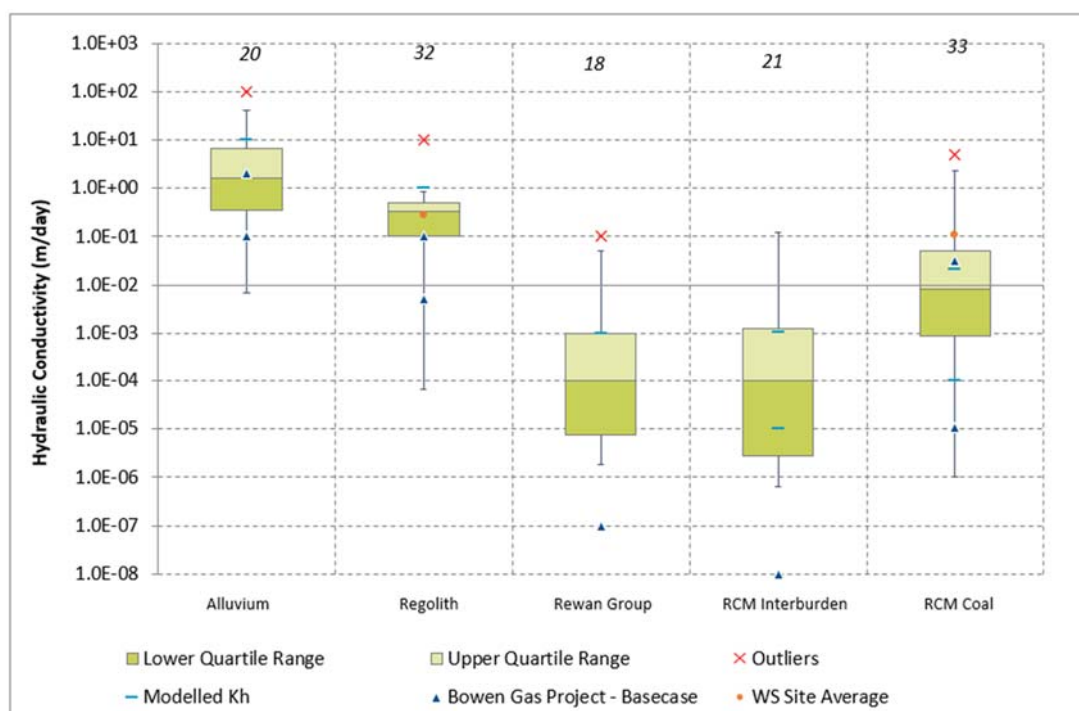


Figure 2-10 Hydraulic Parameters Estimates vs Calibrated Hydraulic Parameters

2.9 Calibrated Storage Properties

Table 2-9 summarises the calibrated values for specific storage and specific yield.

Table 2-9 Calibrated Storage Parameters

Model Layer	Formation	Unit	Specific Yield (SY) (%)	Specific Storage (SS) (m ⁻¹)
1	Alluvium	Surface cover	5.0	1.0×10^{-4}
	Colluvium	Surface cover	0.4	1.0×10^{-5}
1 & 2	Tertiary Basalt	Tertiary basalt	2.9	7.3×10^{-5}
2	Regolith	Tertiary and minor Triassic Clematis	0.1 to 2.0	1.1×10^{-5} to 1.5×10^{-5}
3	Rewan Group	Triassic	0.3 to 0.5	1.5×10^{-5} to 5.0×10^{-5}
4	Rangal Coal Measures	Leichhardt overburden	0.1	5.0×10^{-5}
5		Leichhardt Seam	0.2	3.8×10^{-6}
6		Interburden	0.2	1.8×10^{-6}
7		Vermont Seam	0.1	1.2×10^{-6}
8		Vermont underburden	0.4	1.3×10^{-6}
9	Fort Cooper Coal Measures	Fort Cooper overburden	0.2	1.4×10^{-5}
10		Fort Cooper seam	0.9	5.0×10^{-5}
11		Fort Cooper underburden	0.2	1.0×10^{-6}
12	Moranbah Coal Measures	Moranbah overburden	0.2	1.3×10^{-6}
13		Moranbah (Goonyella) Seam	0.4	1.3×10^{-6}
14		Moranbah underburden	0.1	1.0×10^{-6}
All	Undivided Intrusives	Intrusives	<0.1	1.0×10^{-6}
Below L02	Fault	All below Layer 2	0.1 to 1.0	1.0×10^{-6} to 1.00×10^{-5}
-	Waste Rock/Spoil	-	5.0	1.0×10^{-5}

2.10 Calibrated Recharge

Table 2-10 presents the calibrated (Base Case) recharge rates to each geological unit in the model, compared to the Bowen Gas Project (BGP) recharge rate range. These calibrated recharge rates have been adopted into the predictive model.

Figure 2-11 illustrates the range in recharge values for the model domain, as annual rainfall (mm/year). The recharge rates were calculated using the chloride mass balance (CMB) method for the various units. The CMB calculations were based on available water quality results (chloride concentrations) collected from site monitoring bores and landholder bores. The CMB calculation assumed average annual rainfall of 577 millimetres (mm) as modelled. The calculations also assumed a mean annual rainfall chloride flux of 3 milligrams per litre (mg/L). No site data is available for the low permeability Rewan Group. Outliers were identified as readings more than four standard deviations above the mean (USEPA, 2009) and were excluded from the calculations.

Table 2-10 Rainfall Recharge Ranges

	BGP Low		BGP High		Project Base Case		MVS/ODP Base Case	
	mm/year	% rain	mm/year	% rain	mm/year	% rain	mm/year	% rain
Stream Channel	3	0.48	26	4.35	3.19	0.55	2.8	0.45
Flood Plain Alluvium	2	0.32	17	2.90	1.44	0.25	5.1	0.82
Other Alluvium	1	0.16	9	1.45	1.44	0.25	3.1	0.49
Tertiary Sediments	0.3	0.05	3	0.48	0.1	0.02	0.15	0.02
Tertiary basalt	0.5	0.1	28	4.85	28.87	5.00	-	-
Rewan Group	0	0.00	0	0.00	0.06	0.01	0.01	<0.01
Outcropping Coal Measures	0.3	0.05	3	0.48	0.06	0.01	0.06	0.01

BGP = Arrow Energy Bowen Gas Project

MVS = Moorvale South Project model

ODP = Olive Downs Project model

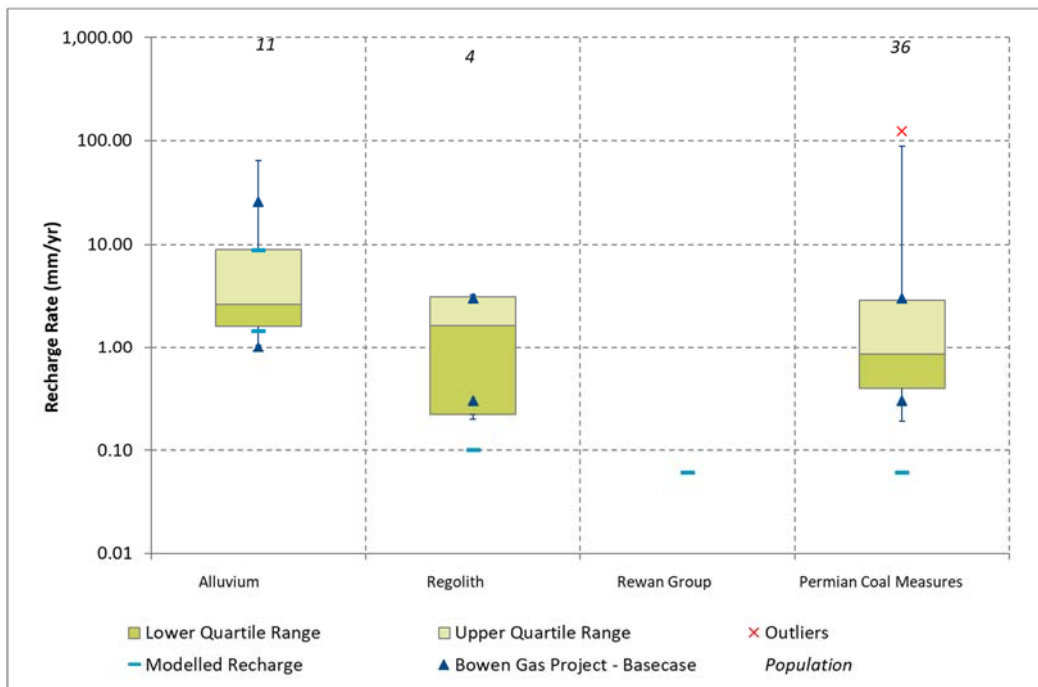


Figure 2-11 Site Recharge Estimates vs Modelled Recharge

This is consistent with the recharge applied in the BGP modelling and has been used as a guide to applicable recharge ranges for each outcropping geological unit. As per the conceptual model, higher recharge occurs through the alluvium and lower recharge in regolith and Permian outcrops. Increased recharge through the alluvium of the Isaac River channel has been used to simulate the potential for the Isaac River to provide rapid recharge to the alluvial groundwater system during rainfall events. For comparison, other nearby projects have used modelled recharge as a default value across the domain, with Lake Vermont simulating recharge equivalent of 2% mean annual rainfall, and Isaac Plains simulating 0.5% to alluvium and 0.25% elsewhere. These values indicate overall rainfall recharge to the groundwater system is limited. Recharge rates in regolith and outcropping coal measures are similar between the Moorvale South Project base case and the Project base case. Recharge rates to the stream channel is 3 times higher in the Project base case relative to Moorvale South Project base case and recharge to Rewan group is also 5 times higher in the Project base case. Conversely, flood plain alluvium and other alluvium recharge rates are more than 2 times higher in the Moorvale South Project base case relative to the Project base case.

3 Predictive Modelling

3.1 Timing and Mining

Transient predictive modelling was used to simulate the proposed mining at the Project as well as mining at other approved and foreseeable mines within the model domain. The predictive model comprises 32 stress periods, from 31 December 2021 until 30 December 2053 with mining cells progressing annually. The predictive model stress period setup is detailed in **Table 3-1**, alongside simulated mine-timings. The planned timing progression for coal seam mining at the Project is presented in **Figure 3-1**.

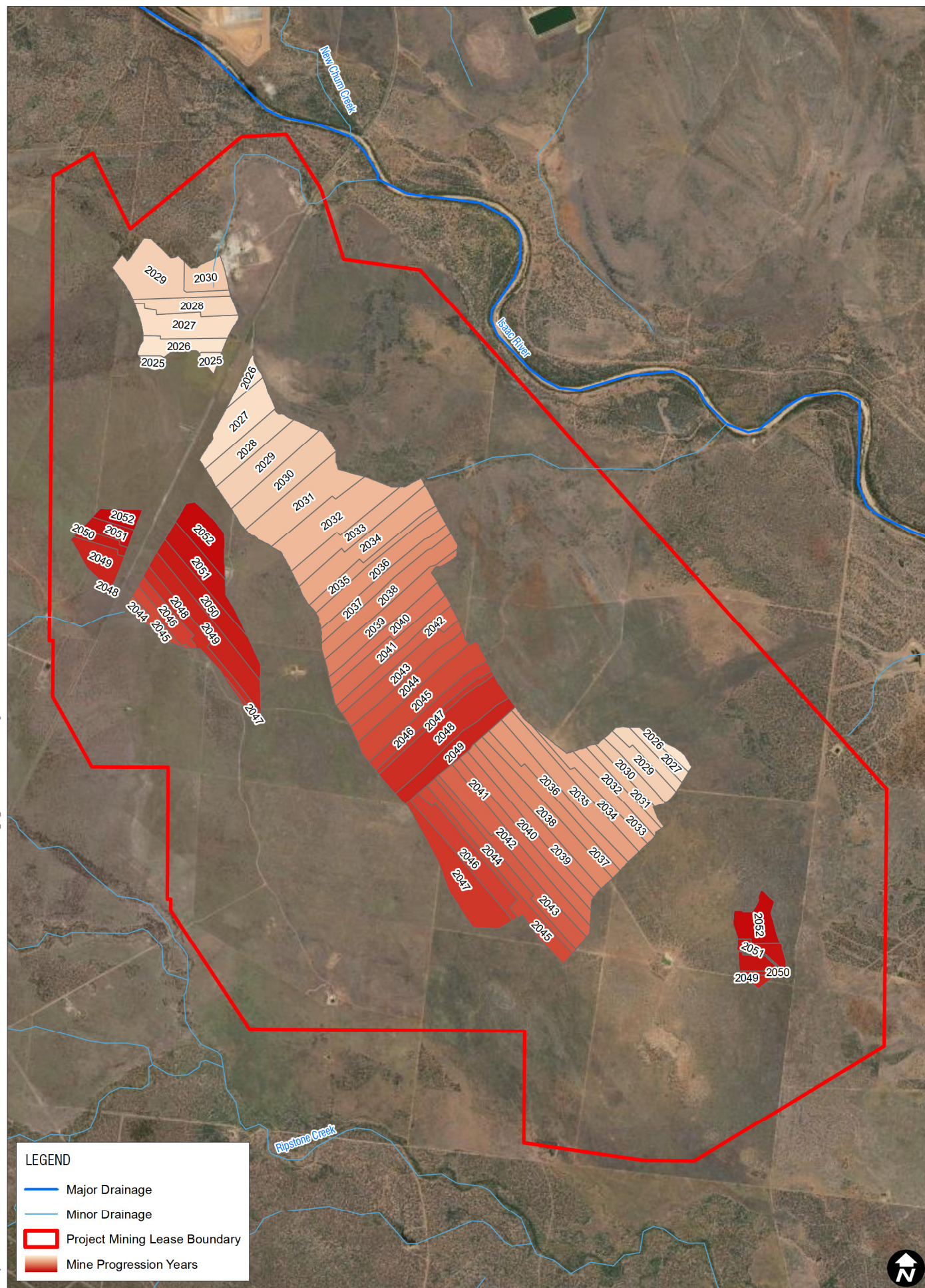
Timings of active drain cells at the Project were based on annual mine progression stage plans. Pre-stripping was simulated 1 year prior to active seam mining by applying drain cells down to the base of Rewan. Following pre-stripping, drain cells were projected down to the base of the lower most target coal seam (i.e. the Vermont seam). A two-year operational window was assumed for mine cells at the Project, after which time the drains were removed and the MODFLOW Time Varying Materials (TVM) package was used to assign spoil properties to the cells. **Table 3-1** details simulated mine timings for the Project and surrounding mines used in the predictive model. All mines included in the model were simulated using the MODFLOW Drain (DRN) package. A nominally high drain conductance of 100 square metres per day (m^2/day) was applied to drain cells to simulate rapid removal of water from the system.

Table 3-1 Predictive Model Stress Period Setup and Mining

Interval	Stress Period	Date (from)	Date (to)	Winchester South (OC)	Moorvale South (OC)	Olive Downs (OC)	Caval Ridge (OC)	Peak Downs (OC)	Saraji (OC)	Saraji East (UG)	Lake Vermont (OC)	Eagle Downs (UG)	Poitrel (OC)	Daunia (OC)
Annual	1	31-12-2021	31-12-2022		x	x	x	x	x		x	x	x	x
Annual	2	01-01-2023	31-12-2023		x	x	x	x	x		x	x	x	x
Annual	3	01-01-2024	30-12-2024	x	x	x	x	x	x		x	x	x	x
Annual	4	31-12-2024	30-12-2025	x	x	x	x	x	x		x	x	x	x
Annual	5	31-12-2025	31-12-2026	x	x	x	x	x	x		x	x	x	x
Annual	6	01-01-2027	31-12-2027	x	x	x	x	x	x		x	x	x	x
Annual	7	01-01-2028	30-12-2028	x	x	x	x	x	x		x	x	x	x
Annual	8	31-12-2028	30-12-2029	x	x	x	x	x	x		x	x	x	x
Annual	9	31-12-2029	31-12-2030	x	x	x	x	x	x		x	x	x	x
Annual	10	01-01-2031	31-12-2031	x		x	x	x	x		x	x	x	x
Annual	11	01-01-2032	30-12-2032	x		x	x	x	x		x	x	x	x
Annual	12	31-12-2032	30-12-2033	x		x	x	x			x	x	x	x
Annual	13	31-12-2033	31-12-2034	x		x	x	x			x	x	x	x
Annual	14	01-01-2035	31-12-2035	x		x	x	x			x	x	x	x
Annual	15	01-01-2036	30-12-2036	x		x	x	x			x	x	x	x
Annual	16	31-12-2036	30-12-2037	x		x	x	x			x	x	x	x
Annual	17	31-12-2037	31-12-2038	x		x	x	x		x	x	x	x	x

Interval	Stress Period	Date (from)	Date (to)	Winchester South (OC)	Moorvale South (OC)	Olive Downs (OC)	Caval Ridge (OC)	Peak Downs (OC)	Saraji (OC)	Saraji East (UG)	Lake Vermont (OC)	Eagle Downs (UG)	Poitrel (OC)	Daunia (OC)
Annual	18	01-01-2039	31-12-2039	x		x	x	x		x	x	x	x	x
Annual	19	01-01-2040	30-12-2040	x		x	x	x		x	x	x	x	x
Annual	20	31-12-2040	30-12-2041	x		x	x	x		x	x	x	x	x
Annual	21	31-12-2041	31-12-2042	x		x	x	x		x	x	x	x	x
Annual	22	01-01-2043	31-12-2043	x		x	x	x		x	x	x	x	x
Annual	23	01-01-2044	30-12-2044	x		x	x	x		x	x	x	x	x
Annual	24	31-12-2044	30-12-2045	x		x	x	x		x	x	x	x	x
Annual	25	31-12-2045	31-12-2046	x		x	x	x		x	x	x	x	x
Annual	26	01-01-2047	31-12-2047	x		x	x	x		x	x	x	x	x
Annual	27	01-01-2048	30-12-2048	x		x	x	x		x	x	x	x	x
Annual	28	31-12-2048	30-12-2049	x		x	x	x		x	x	x	x	x
Annual	29	31-12-2049	31-12-2050	x		x	x	x		x	x	x		x
Annual	30	01-01-2051	31-12-2051	x		x	x	x		x	x	x		x
Annual	31	01-01-2052	30-12-2052	x		x	x	x		x	x	x		x
Annual	32	31-12-2052	30-12-2053	x		x	x			x	x	x		x

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Project Mine Progression

FIGURE 3-1

3.2 Water Balance

Table 3-2 details average flow rates for water transfer into and out of the predictive model period (December 2021 to end of December 2053) for two scenarios:

- Scenario A (Approved Mining) which includes all approved and foreseeable surrounding mines in the Study Area; and
- Scenario B (Cumulative Mining) which includes the surrounding mines from Scenario A, with the addition of the Project.

In both scenarios, the largest inflow contributor to the groundwater system is rainfall recharge. Rainfall recharge contributes on average 7.35 ML/d in Scenario A, and 7.46 ML/d in Scenario B to the model groundwater system. Regional groundwater flow is the next largest contributor in both scenarios. For Scenarios A and B, regional groundwater flow provides a net model inflow contribution of 2.51 ML/d and 2.49 ML/d, respectively. Net inflow of leakage from the Isaac River to the groundwater system is consistent between the scenarios, at 1.89 ML/d.

Groundwater outflow from the model mostly occurs via drain cells, used to simulate open cut and underground mining activity in the model. Drain cell outflow is equal to 9.28 ML/d in Scenario B and 8.94 ML/d in Scenario A. See **Section 3.5** for a summary of the predicted inflows to the proposed open cut pits for the Project. In both scenarios, evapotranspiration and baseflow to minor river systems are responsible for average outflow rates of 0.60 ML/d and 0.61 ML/d, respectively.

Both scenarios maintained mass balance errors below 1% for all time steps as well as cumulatively throughout the simulations. The low error achieved indicates that the predictive model is stable, and the solution achieved is accurate (Barnett *et al.*, 2012).

Table 3-2 Average Simulated Water Balance over the Prediction Period

Component	Scenario A (Approved Mining)		Scenario B (Cumulative Mining)	
	Inflow (ML/d)	Outflow (ML/d)	Inflow (ML/d)	Outflow (ML/d)
Recharge (direct rainfall)	7.35	-	7.46	-
Evapotranspiration (ET)	-	0.59	-	0.59
SW/GW Interaction Isaac River (STR)	14.69	12.80	14.55	12.66
SW/GW Interaction Minor Rivers (RIV)	-	0.61	-	0.60
Regional GW flow (GHB)	3.91	1.40	3.88	1.39
Drains (Mine water removal)	-	8.94	-	9.28
Storage	14.68	16.29	15.15	16.52
Total	40.63	40.63	41.04	41.04

3.3 Predicted Groundwater Levels

Predicted groundwater levels at the end of mining operations for the two scenarios are provided in **Figure 3-2** through **Figure 3-7**. No data regions in the water level grids represent unsaturated areas, i.e. where the simulated water level elevation is below the base of cell.

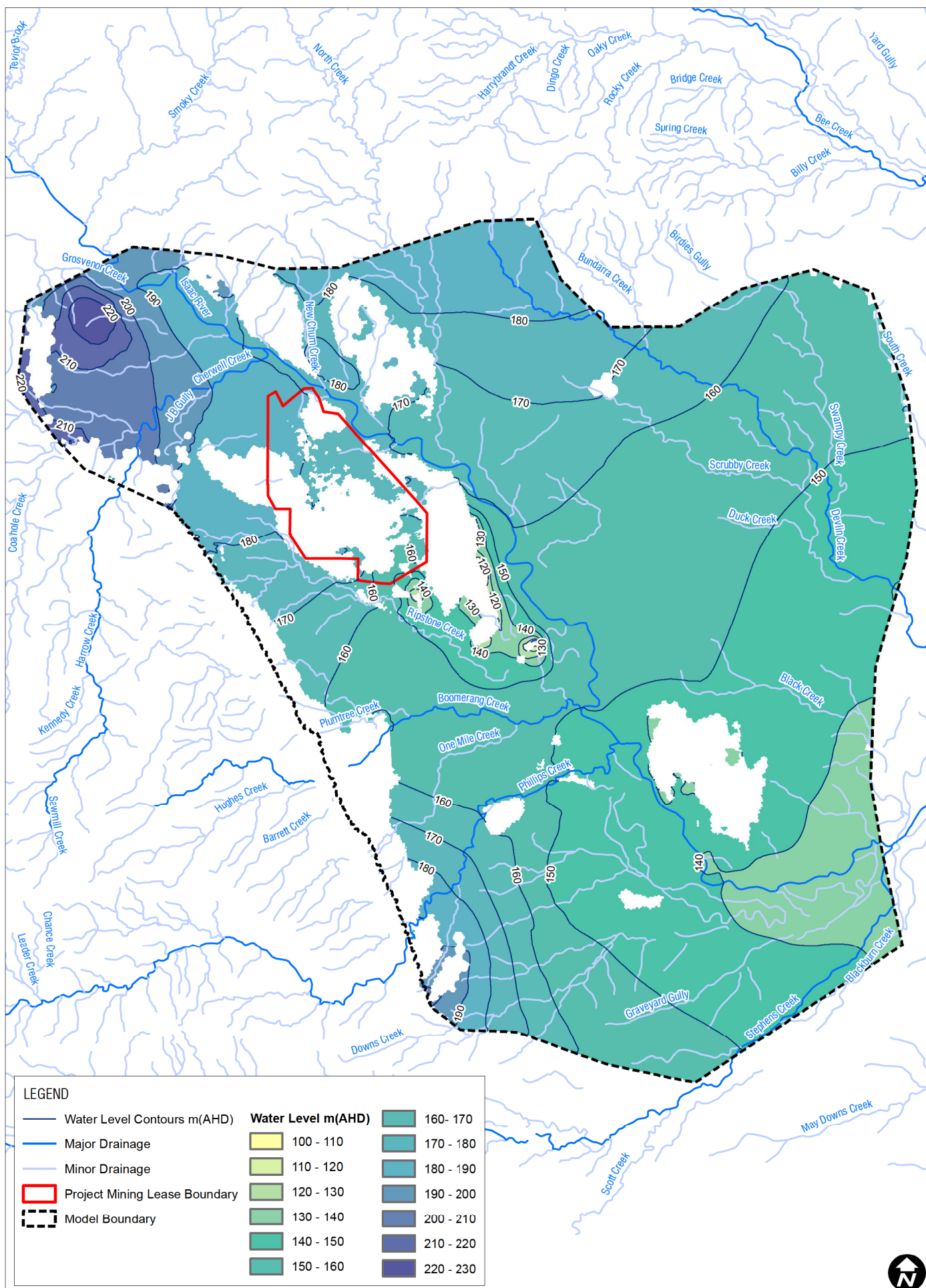
Minimal changes to alluvial groundwater levels are observed between the Approved and Cumulative mining scenarios (**Figure 3-2** and **Figure 3-5**). **Figure 3-3** and **Figure 3-6** show predicted groundwater levels in the regolith at the end of mining for the two scenarios. Dewatering of the regolith caused by the proposed mining at the Project is evident by the larger desaturated zone within the Project Area for the Cumulative mining scenario (**Figure 3-6**), relative to the Approved mining scenario (**Figure 3-3**).

Figure 3-4 and **Figure 3-7** show the predicted water levels in the Fort Cooper Coal Measures overburden (Layer 9) at the end of mining for the Approved and Cumulative mining scenarios. This unit has been chosen to represent head levels in the Permian Coal Measures due to its regional extent. A regional south-easterly hydraulic gradient can be observed, reflecting the downstream flow gradient of the Isaac River. Zones of depressurisation at the Project and surrounding mines are shown to cause localised interruptions to the regional flow gradient. Discussion on groundwater drawdown within the Permian Coal Measures is included in **Section 3.4**.

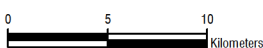


FIGURE 3-2

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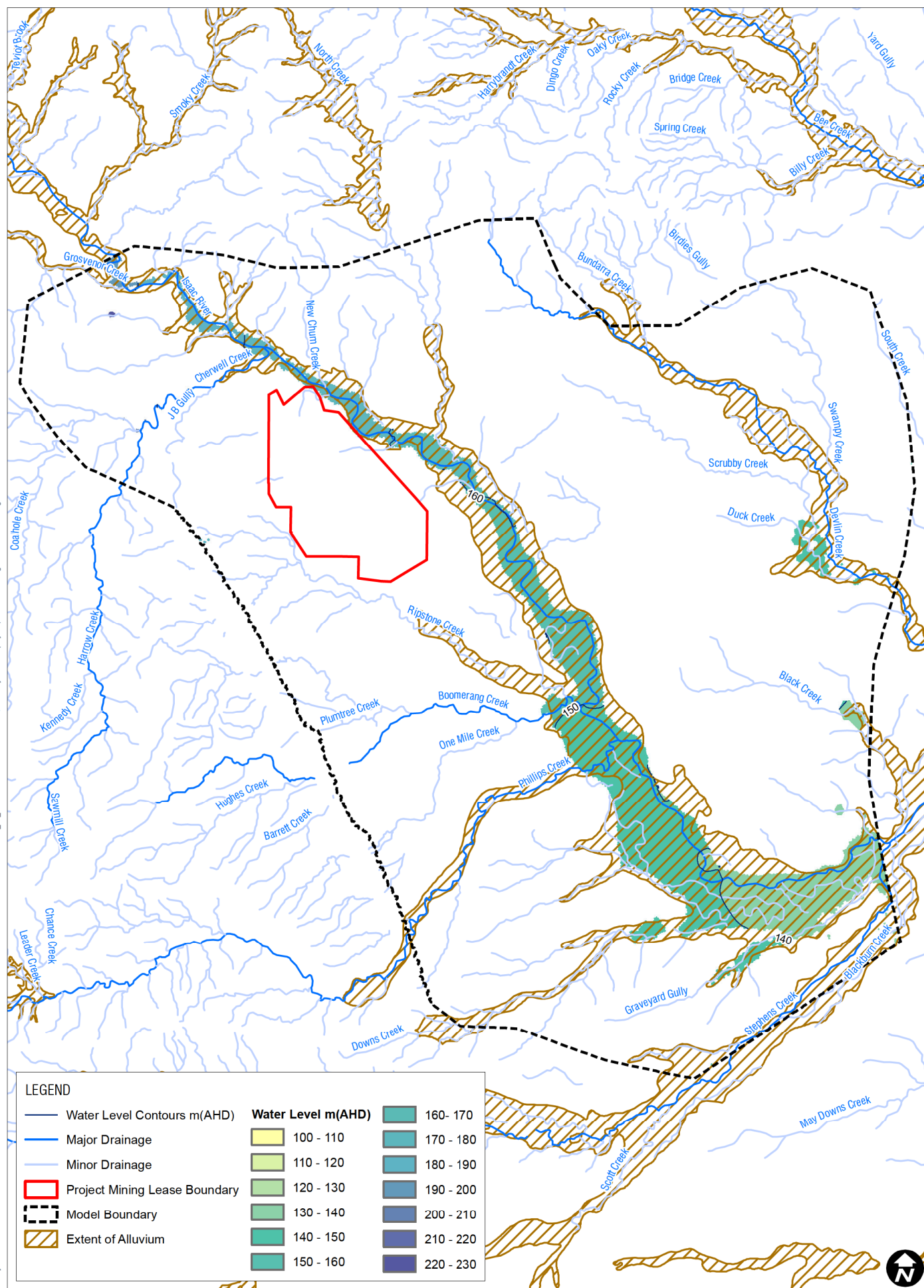
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**Predicted Water Level within Regolith
(Layer 2) at End of Mining - Approved Operations Only**

FIGURE 3-3

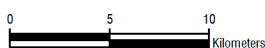
H:\Projects\SLR\020-BNE\020-BNE\020-13245-Winchester South Groundwater\03 SLR Desktop\CAD\GIS\03\0305 Revised Mine Plan\02013245_P3_5 Predicted Water Level within Quaternary Alluvium (Layer 1) at End of Mining - Cumulative Mining Scenario.mxd



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**Predicted Water Level within Quaternary Alluvium
(Layer 1) at End of Mining – Cumulative Mining Scenario**

FIGURE 3-5





3.4 Maximum Predicted Drawdowns

The process of mining directly removes water from the groundwater system and reduces water levels in surrounding groundwater units. The extent of the zone affected is dependent on the properties of the aquifers/aquitards and is referred to as the zone of drawdown. Aquifer drawdown is greatest at the working coal-face and decreases with distance from the mine.

Maximum incremental drawdown refers to the drawdown impact associated with the Project and is obtained by comparing the difference in predicted aquifer groundwater levels for the Approved model scenario and the Cumulative model scenario at matching times. The maximum drawdown represents the maximum drawdown values recorded at each model cell at any time over the predictive model duration. Predicted drawdown figures (**Figure 3-8** through **Figure 3-14**) show where maximum drawdown impacts are predicted to exceed 1 m. In areas within the 1 m drawdown contour, the unit is considered impacted by drawdown. Figures include the locations of known private bores intercepting the relevant layers. Note that no private bores are predicted to be impacted as a result of mining activities at the Project.

There is no incremental drawdown predicted for the Quaternary alluvium as a result of mining at the Project. For a discussion on the potential incidental water impacts on the Quaternary alluvium, see **Section 3.6.1**.

The maximum predicted incremental drawdowns associated with the Project within the regolith is shown in **Figure 3-8**. Lateral incremental drawdown extents within the regolith (Layer 2) is largely confined to the Project Area, and is influenced by the distribution of predicted saturated zones in the regolith. Predicted effects of drawdown in the regolith are largely constrained to the Project Area, extending only up to approximately 1.8 km to the north-west and 1.6 km to the south-east away from the Project Area. Drawdown in the south of the Project is predicted to reach Pit 9 of the Olive Downs Project. Incremental drawdown for the Project within regolith is not predicted to exceed 15 m.

The Leichhardt and Vermont coal seams of the Rangal Coal Measures are the primary aquifers targeted by the Project, and are predicted to experience drawdowns as a direct result of mining at the Project. Groundwater level drawdown within the mined coal seams is influenced by unit structure and is confined to unit extents. The direction of drawdown propagation in the coal seam aquifers is shown to align with the geologic strike of the Winchester South Syncline on which the Project is located (northwest – southeast). Drawdown in this layer is restricted in the east-west direction by the unit structure and are largely contained within the Project Area (**Figure 3-9** and **Figure 3-10**).

Figure 3-9 shows the maximum predicted incremental drawdown for the Leichhardt seam (Layer 5). This unit is predicted to experience a maximum of 88 m drawdown at the working coal face. The maximum predicted Project-only related drawdown within the Leichhardt seam is largely limited to the Project Area, only extending 1.7 km north-west and 1.6 km south-east of the Project.

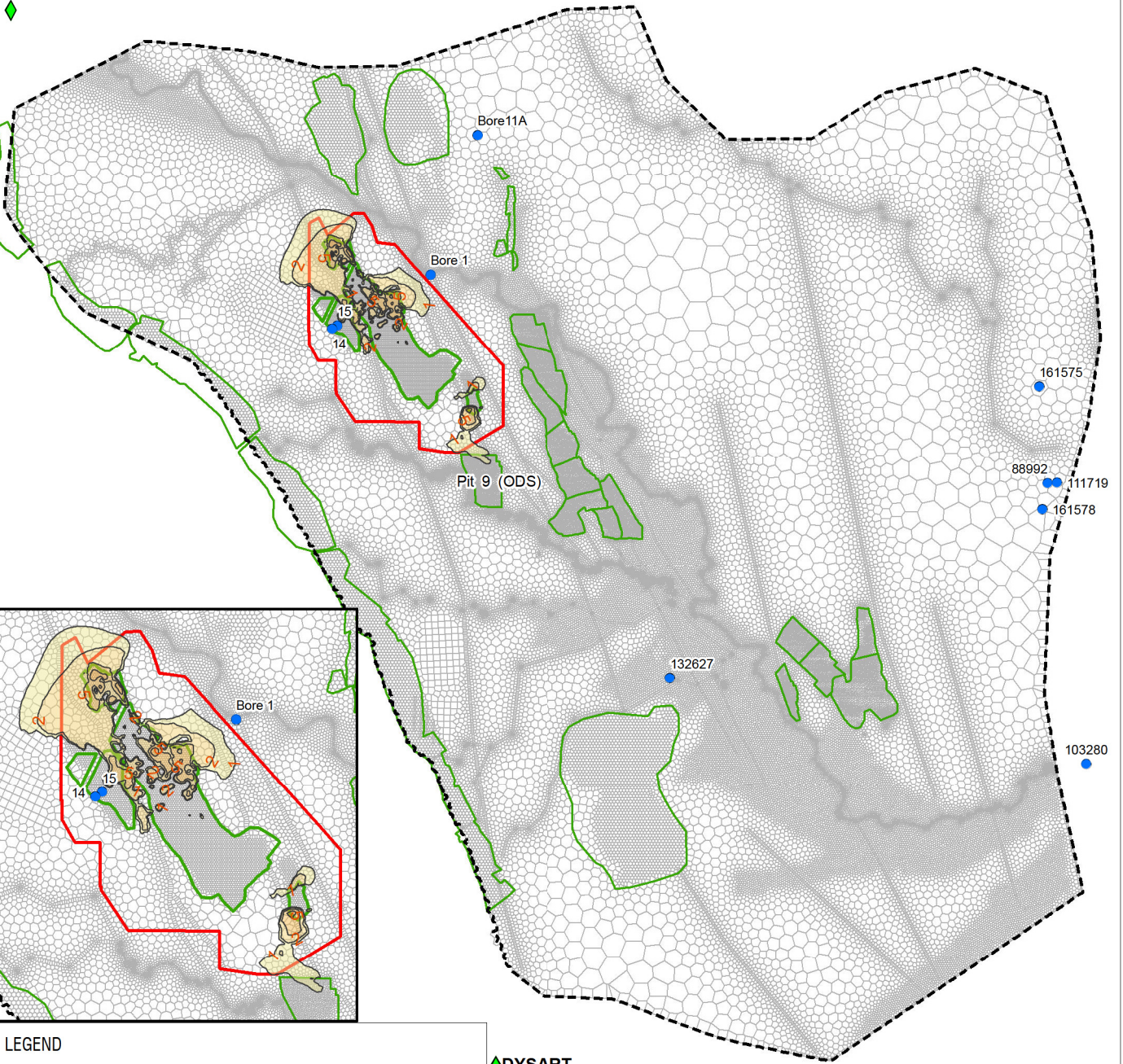
Figure 3-10 shows the maximum predicted incremental drawdown for the Vermont seam (Layer 7). This unit is predicted to experience a maximum of 133 m drawdown at the working coal face. Drawdown is predicted up to 1.6 km west of the Project Area, and up to 1.2 km south-east of the Project. Drawdown in both the Leichhardt and Vermont seams are predicted to reach mining at Pit 9 of the Olive Downs Project.

Cumulative drawdown impacts are shown in **Figure 3-11** through **Figure 3-14**. These drawdowns represent the total impact of mining to model groundwater levels by comparing the maximum difference in aquifer groundwater levels for the Cumulative model scenario with those in a theoretical “No Mining” scenario, for all times during the predictive model period.

Cumulative drawdown impacts are predicted within the extents of the Isaac River alluvium and occur north and east of the Project Area (**Figure 3-11**). Cumulative drawdown within the regolith is predicted to interact with Project-related drawdown at the Olive Downs Project, south of the Project, as well as to Eagle Downs Mine and Peak Downs Mine impacts to the west (**Figure 3-12**). For the Leichhardt and Vermont coal seams, drawdown interaction is predicted between the Project and Olive Downs Project Pit 9 (**Figure 3-13** and **Figure 3-14**). For drawdowns at specific times over the life of mining, see **Appendix D**.



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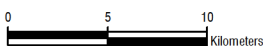
- Privately-owned Bores
 - Major Drainage System
 - Minor Drainage System
 - Drawdown Contours (m)
 - ▭ Project Mining Lease Boundary
 - ▭ Mine Outline
 - ▭ Model Boundary
 - ▭ Model Mesh
- | Maximum Drawdown (m) |
|----------------------|
| 0 - 1 |
| 1 - 2 |
| 2 - 5 |
| 5 - 10 |
| 10 - 20 |
| 20 - 50 |
| 50 - 100 |
| 100 - 200 |
| 200 - 400 |

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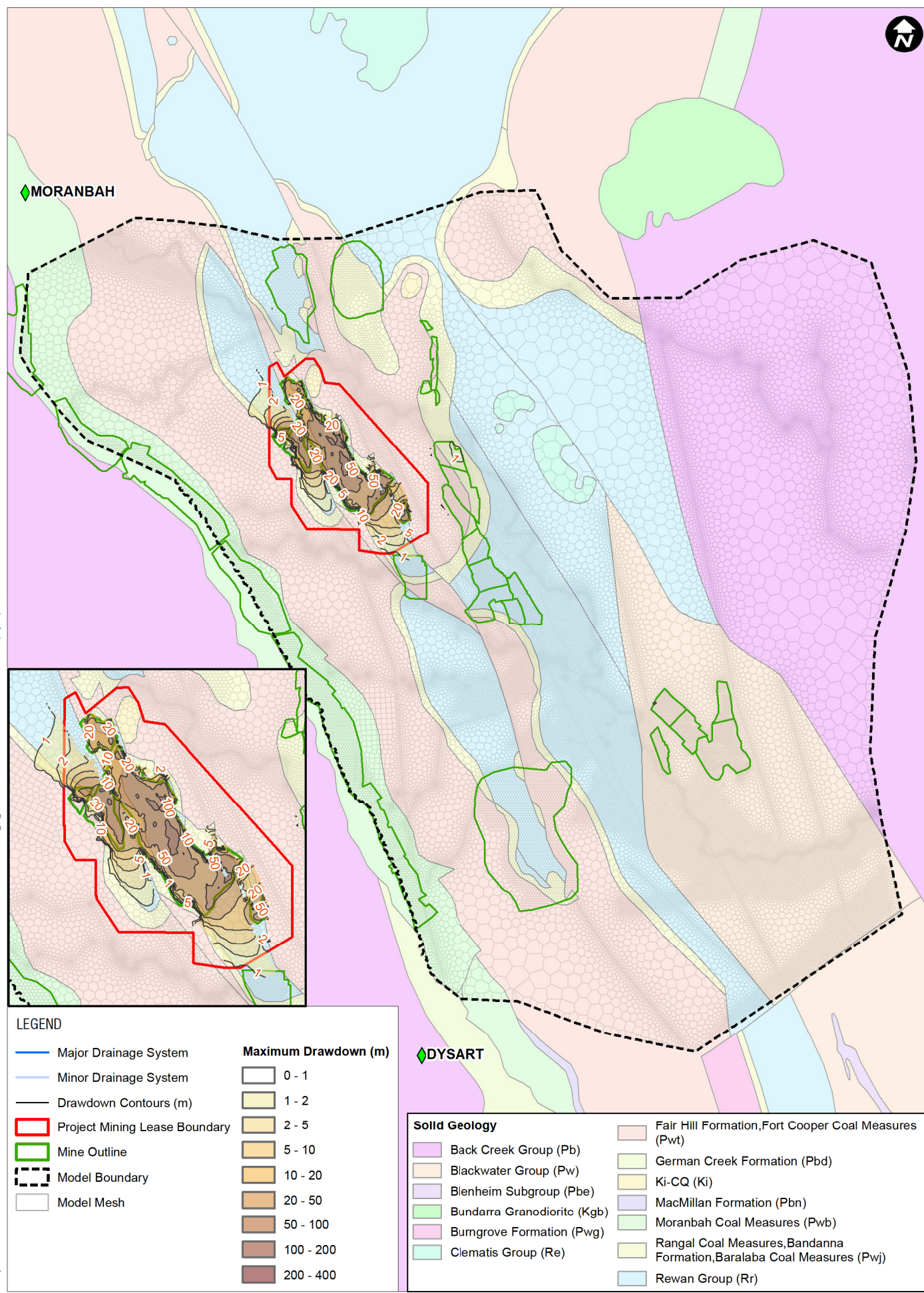
Maximum Incremental Drawdown in
Regolith (Layer 2)

FIGURE 3-8



FIGURE 3-9

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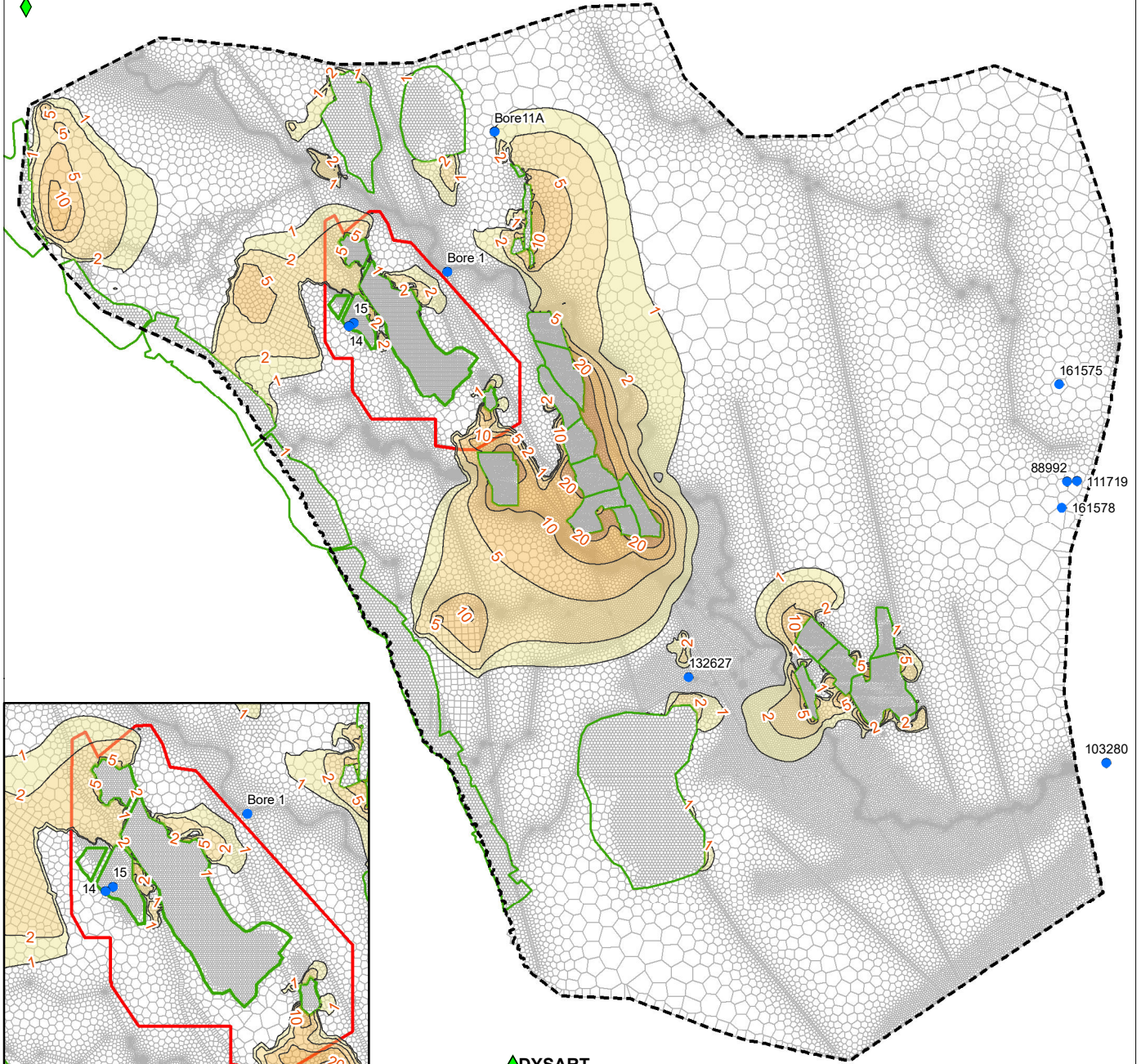


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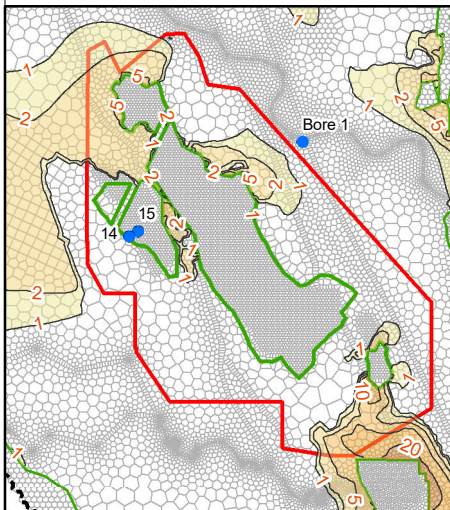




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● Privately-owned Bores	Maximum Drawdown (m)
— Major Drainage System	 0 - 1
— Minor Drainage System	 1 - 2
— Drawdown Contours (m)	 2 - 5
 Project Mining Lease Boundary	 5 - 10
 Mine Outline	 10 - 20
 Model Boundary	 20 - 50
 Model Mesh	 50 - 100
 Extent of alluvium	 100 - 200
	 200 - 400

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GDA 1994 MGA Zone 55

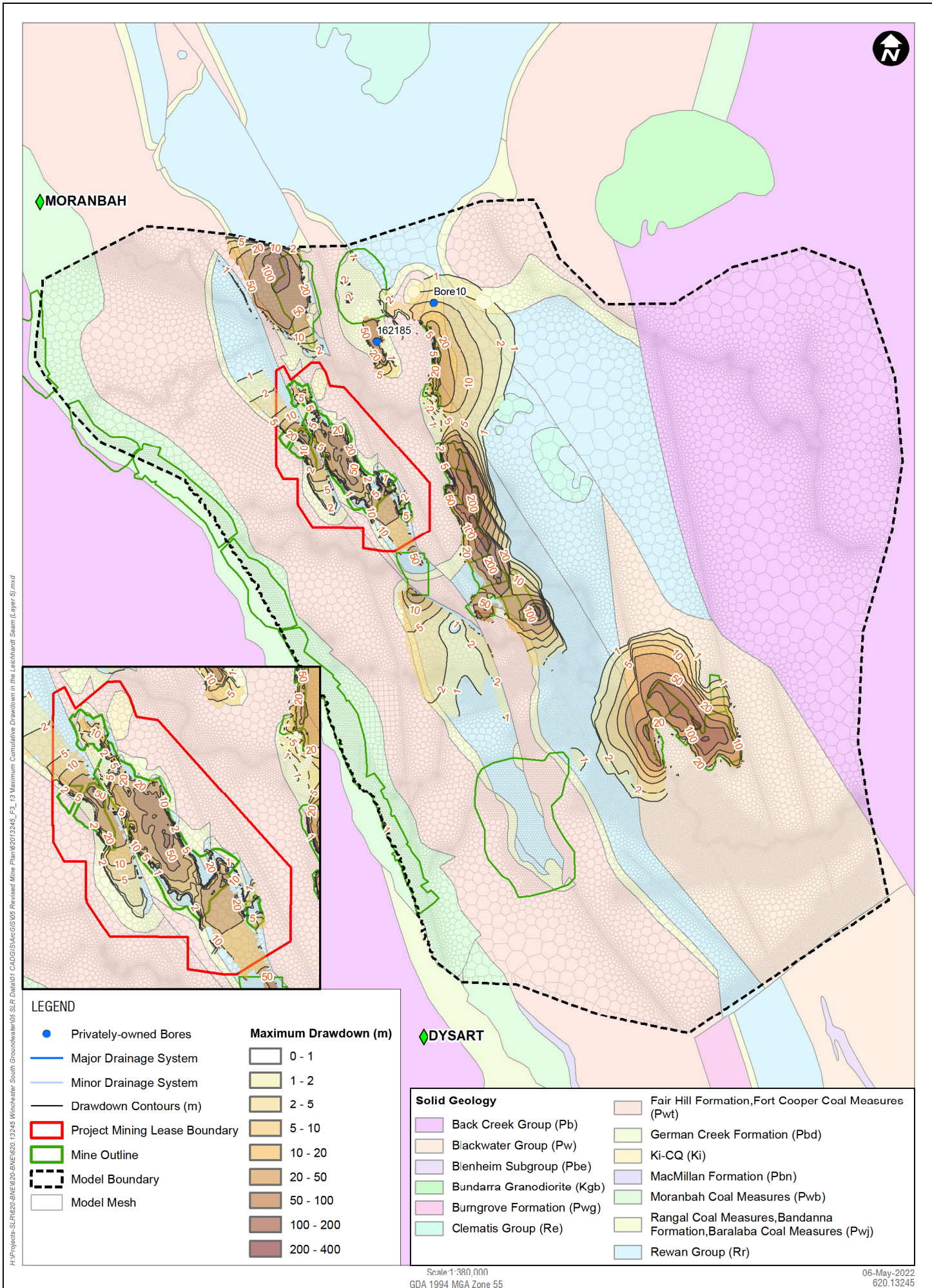
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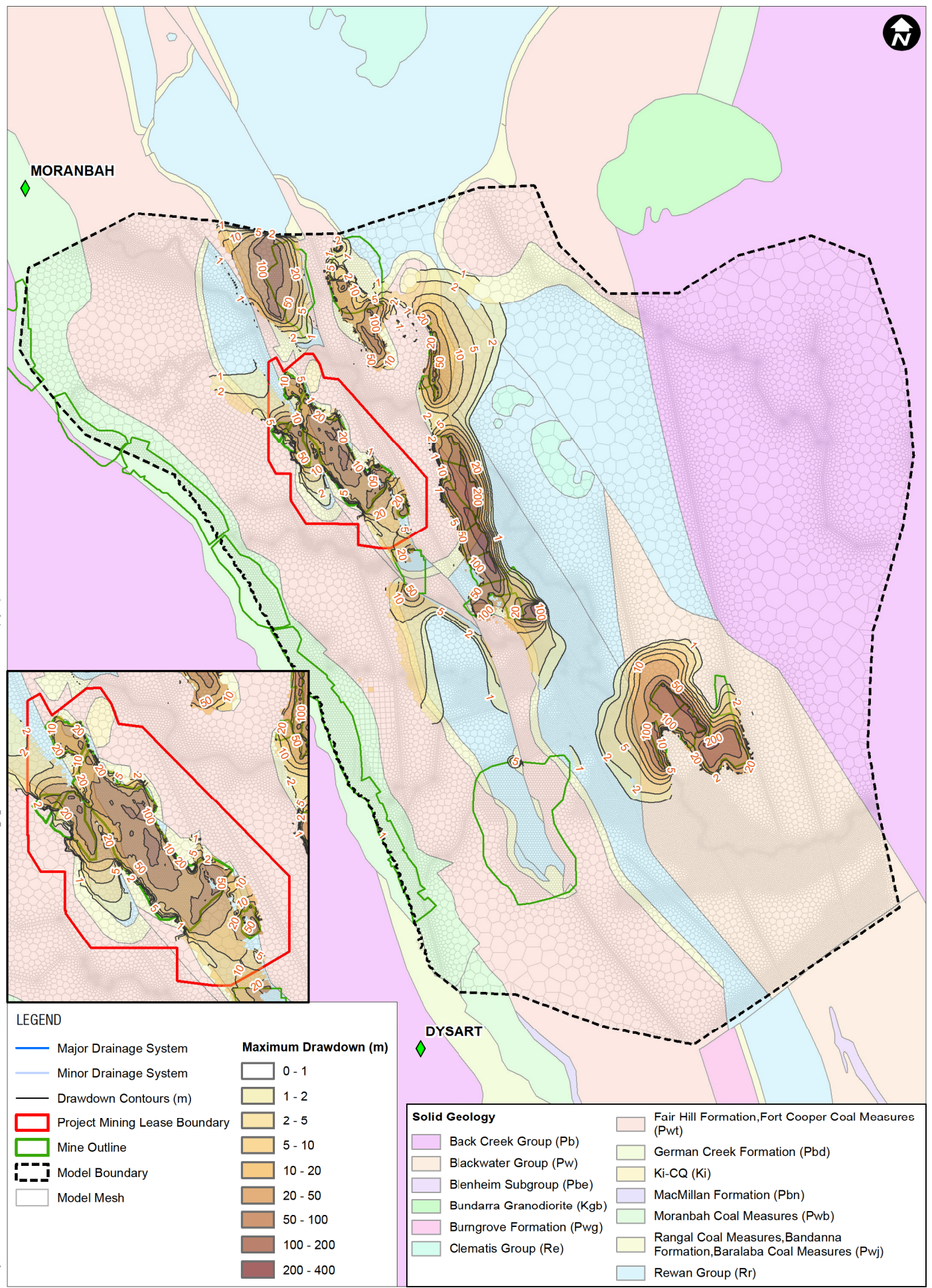


Maximum Cumulative Drawdown in Regolith (Layer 2)

FIGURE 3-12



H:\Projects-SLR\620-BNE\620-BNE\620-13245 Winchester South Groundwater\SLR Data\OT CAD\GIS\620-BNE\620-13245 Maximum Cumulative Drawdown in the Vermont Seam (Layer 7).msd



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3.5 Predicted Groundwater Interception

Project mine pit inflow volumes have been calculated as time weighted averages of the outflow reported by Zone Budget software for Project drain cells. Results are presented in **Figure 3-15**. As shown, inflow to the open cut operations is predicted to reach a maximum peak in year 2032, with 280 ML total inflow predicted for the 2032. Inflow rates decline before rising again from 2047, with the planned commencement of mining at South Pit, West Pit and North-west Pit. The later peak in 2051 is predicted to reach approximately 201 ML/year. The average inflow rate over the total duration of mining is calculated at 155 ML/year.

The Water Plan (Fitzroy Basin) 2011 groundwater area consists of the following:

- Groundwater Unit 1 (containing aquifers of the Quaternary alluvium); and
- Groundwater Unit 2 (sub-artesian aquifers).

Planned mining operations at the Project will not intercept Quaternary alluvium at any of the proposed pits. As such, all direct groundwater take predicted by the model is from Groundwater Unit 2.

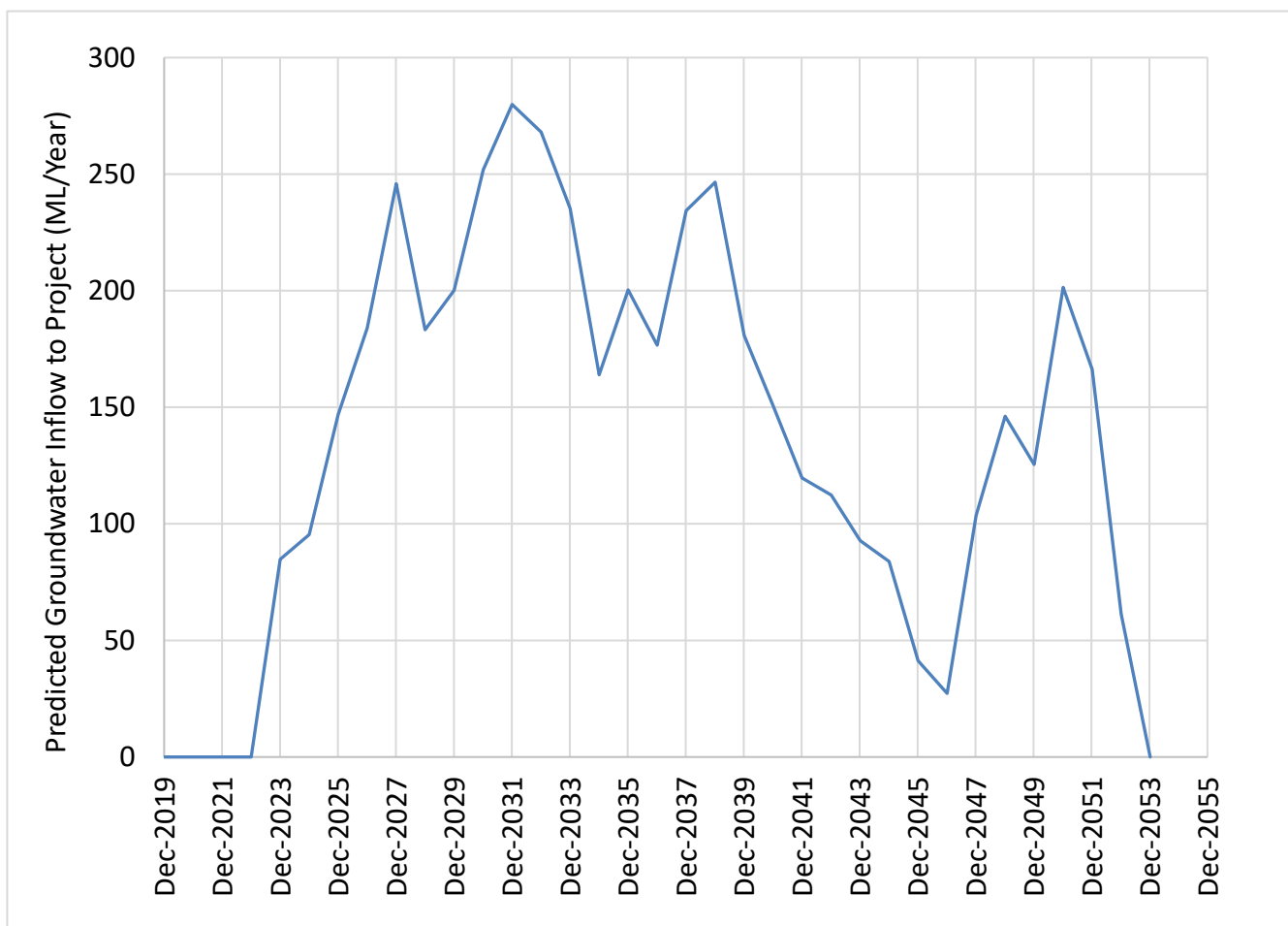


Figure 3-15 Predicted Project Mine Inflows

3.6 Incidental Water Impacts

3.6.1 Influence on Alluvium

Interference of the alluvial groundwater can occur due to increased leakage to the underlying Permian coal measures that are predicted to be depressurised as a result of mining activities. Over the extent of Quaternary alluvium, there is no predicted loss of water from the alluvium as a result of exercising the underground water rights for the Project. Uncertainty analysis was performed on this metric and it was shown that 58% of the realisations run showed zero (0.0 ML) take from the Isaac River alluvium over the life of the Project. Outcomes of the uncertainty analysis show only the 95th percentile indicated any take from alluvium (0.74 ML total over the life of the Project). See **Section 5.4.2** for further uncertainty analysis surrounding this metric.

3.6.2 Groundwater – Surface Water Interaction

The predicted change in water levels induced by mining could increase the hydraulic gradient between the Isaac River and the alluvium. The model predicts that over the life of mine, the change in the average rate of seepage from the river to the alluvium is insignificant and considered within the error threshold of predictions (less than 3.58 ML/year)¹. On average, when the Isaac River flows, 161,863 ML/year of surface water is discharged downstream. An estimate of less than 0.01% increased seepage from the Isaac River to the alluvium as a result of mining at the Project, therefore, represents an insignificant potential for flow rate reduction. The number of days that the Isaac River runs dry is not predicted to increase with the addition of the Project.

¹ Note that the incidental water impacts, reported above, have been obtained using a model version that does not simulate mining at Poitrel.

4 Recovery Model

4.1 Water Level Simulation

The potential post-mining impacts of the Project were investigated with a recovery model for the optimised final landform (e.g. three residual voids), commencing at the end of mining at the Project and run for 2,000 years, then followed by a final steady-state stress period. All drain cells in the Study Area were removed at the start of the recovery period to allow groundwater levels to equilibrate. At the end of mining at the Project, the properties of the residual void cells were converted to values representative of residual void value. The void cells were assigned high horizontal and vertical hydraulic conductivities (1,000 m/day) and storage parameters based on the compressibility of water (specific yield of 1.0, storage coefficient of $5.0 \times 10^{-6} \text{ m}^{-1}$), to simulate free water movement within the residual void. This approach is often referred to as a 'high-K' lake. The location of residual voids at the Project is provided in **Figure 4-1**.

Predicted groundwater inflows were incorporated in the site water balance model for the Surface Water Assessment (WRM Water & Environment [WRM], 2022). The pit lake recovery levels and timings were then predicted by the surface water consultants. These elevations and recovery timings derived from the surface water modelling were replicated within the numerical groundwater model (**Figure 4-2**) using the time variant constant head boundary (CHD) condition. This recovery model was then re-run for 2,000 years. A steady state run was then undertaken after 2,000 years to ensure that the simulated groundwater system has reached an equilibrium, and hence the predicted groundwater levels are representative of long-term average conditions.

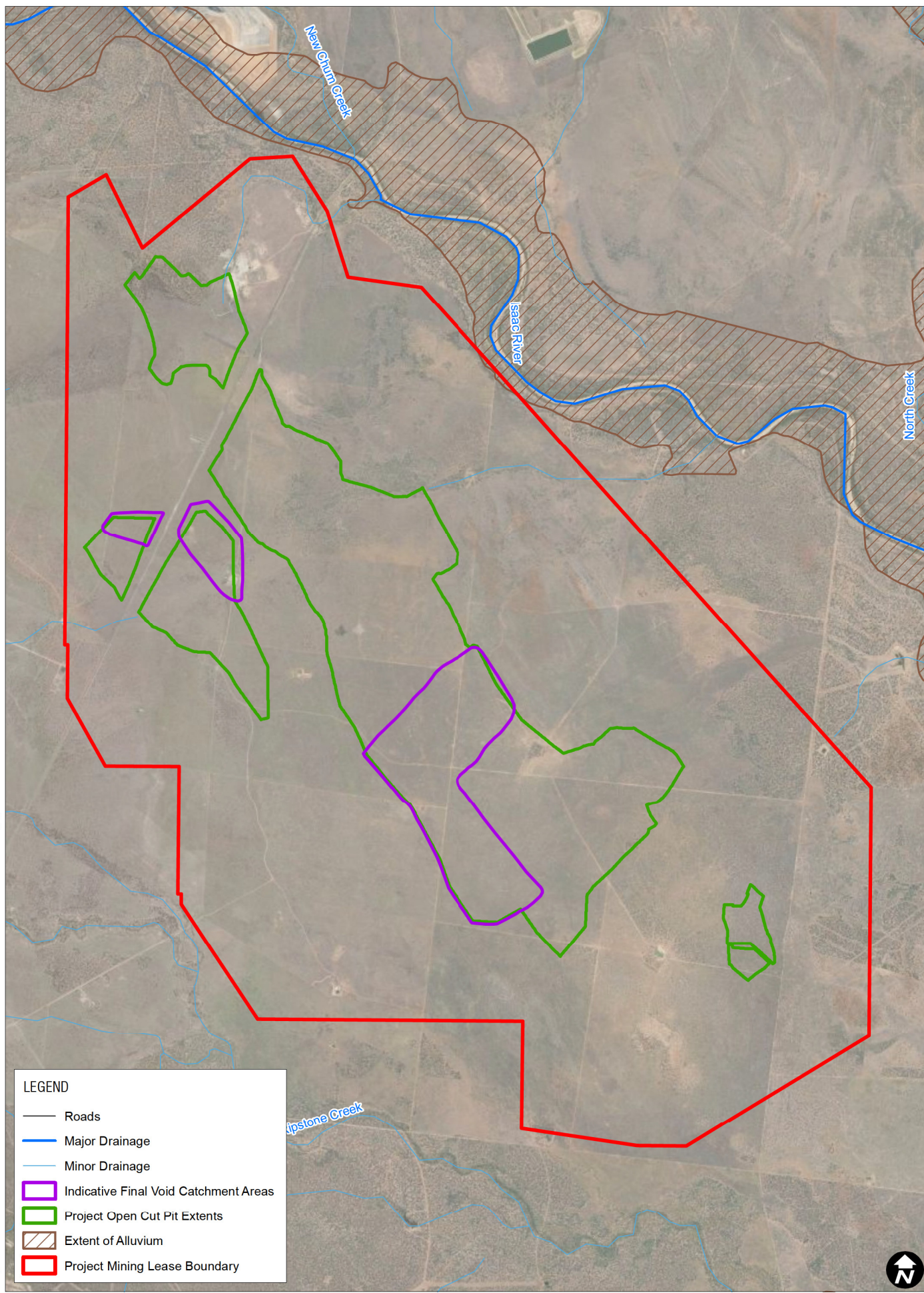
Figure 4-3 shows the final predicted groundwater flows to the three residual voids during the 2,000 years of transient recovery simulation. It should be mentioned that the negative numbers in **Figure 4-3** indicate an outflow from the void and positive numbers indicate an inflow to the void. During the first 50 years of recovery, the flows between the Main pit and spoil are negative indicating that there is an outflow from the void to the spoil. This is expected since the spoil is unsaturated at the start of the recovery and CHD cells within the void inject water to the spoil. The direction of flow reverses after 150 years as the water level stabilises and reaches equilibrium within the void following 150 years. In the latter, the flow will be generally toward the void.

The average predicted equilibrated residual void water levels were:

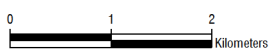
- 128 mAHD within North-west Pit Void;
- 104 mAHD within West Pit Void; and
- 141 mAHD within Main Pit Void.

The peak predicted equilibrated residual void water levels (including the predicted residual void water levels following a Q1000 event) are presented in the Surface Water and Flooding Assessment prepared by WRM (2022).

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Residual Void Locations

FIGURE 4-1

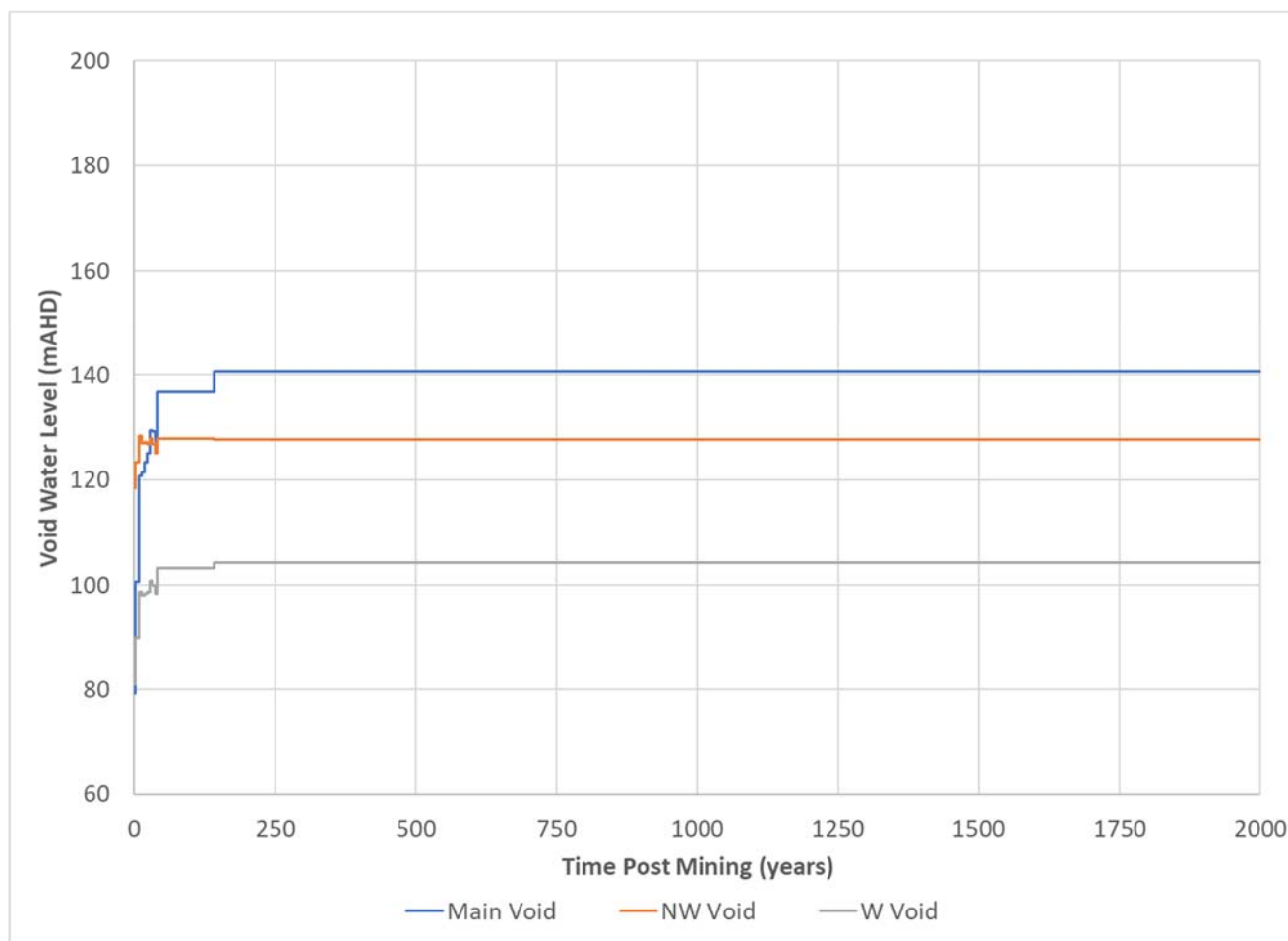


Figure 4-2 Simulated Residual Void recovery

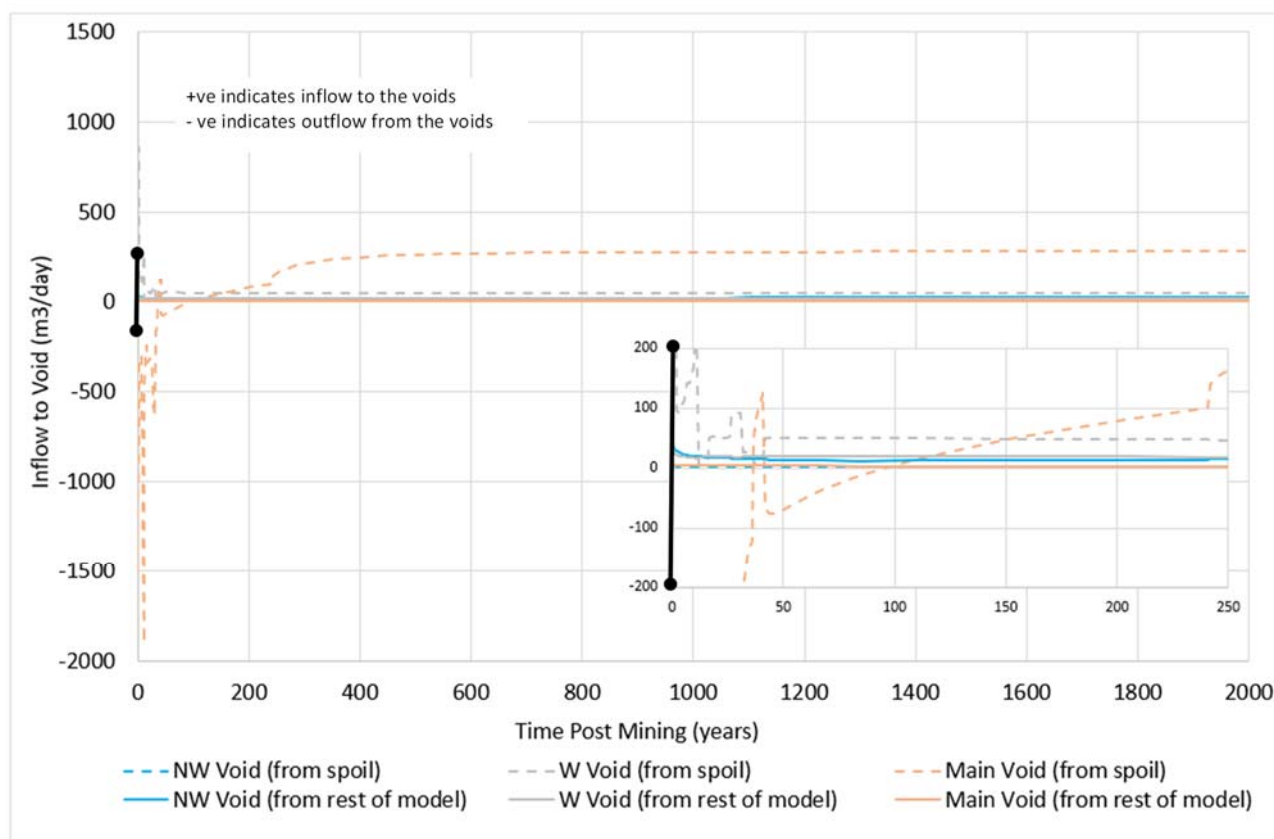


Figure 4-3 Predicted Residual Void inflows

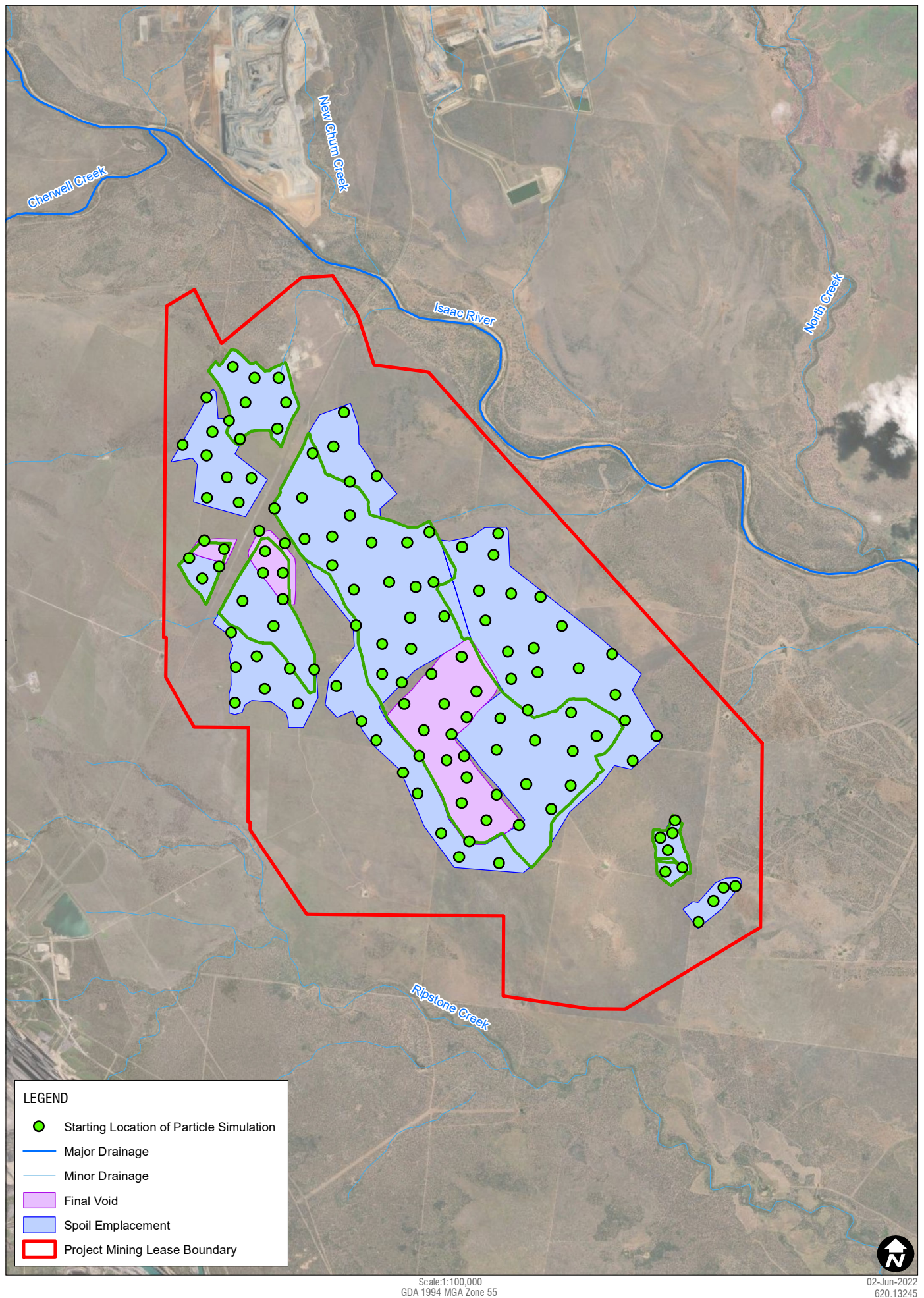
4.2 Flow Path Simulation

To investigate the water movement within the voids and spoil within the backfilled open cut pits of the optimised final landform during the recovery, an assessment was undertaken to simulate the movement and fate of water particles through the groundwater system. To achieve this, a number of particles were placed within the voids in the model and the Mod-PATH3DU code (S.S. Papadopoulos & Associates, Inc., 2018) was used to simulate particle pathways along the groundwater flow field during recovery (i.e., 2,000 years). To run the Mod-Path3DU code, the groundwater flow model was first simulated, and the transient head outputs from the groundwater flow model were used by Mod-PATH3DU to simulate particle flowpath lines. **Figure 4-4** shows the location of particles placed within voids and out-of-pit dumps. The particles were released from the start of recovery and the movement of particles was recorded during the recovery simulation.

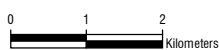
Figure 4-5 shows the predicted movement of water particles in the recovery simulation. The colours along the flow path range from red to green, representing layers 2 (Regolith) through to 9 (Fort Cooper Overburden). Flow simulated within the Vermont Seam is shown in pale green. The blue arrows in the figure show the general direction of particle movements. The flow path analysis indicates the particles generally move toward the residual void lakes, indicating that the simulation predicts the residual voids are acting as sinks. The colour changes along the paths indicate that particles situated within the shallower layers at the beginning of the recovery model move progressively toward layers 6 and 7 (Vermont Seam and Overburden).

The head contours for the recovered groundwater levels at the end of the recovery model (i.e. steady state heads following at least 2,000 years of recovery) are shown in **Figure 4-5**. Given that the particle pathlines are simulated from the start of recovery, they generally follow head gradients, and any changes that occur to the gradients during the transient simulations.

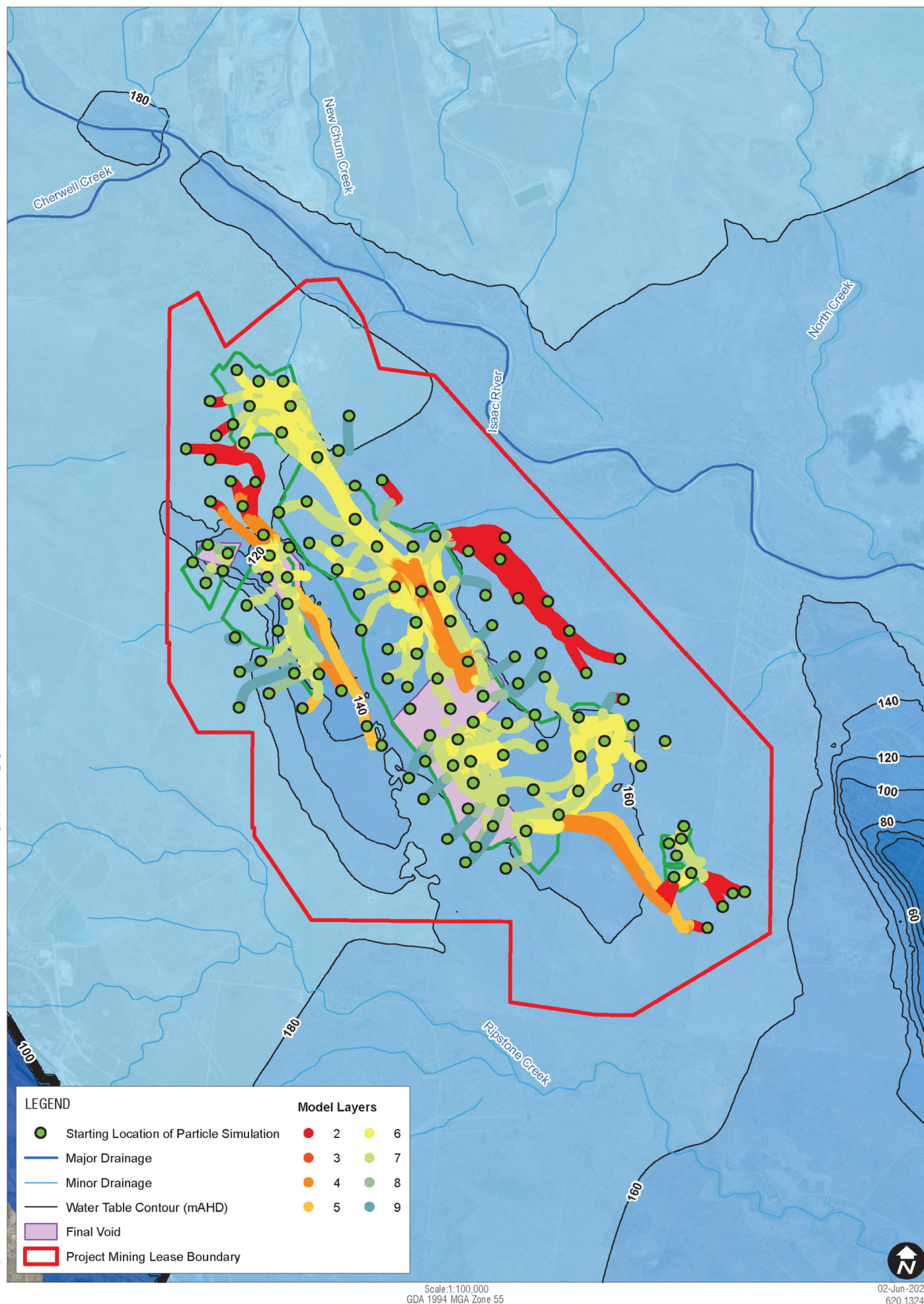
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5 Sensitivity and Uncertainty Analysis

A Type 3 Monte Carlo uncertainty analysis (Independent Expert Scientific Committee on Coal Seam Gas and Large Coal Mining Development [IESC], 2018) was undertaken to estimate the uncertainty in the future impacts predicted by the model. This method operates by generating numerous alternative sets of input parameters to the deterministic groundwater flow model (realisations), executing the model independently for each realisation, and then aggregating the results for statistical analysis.

The first step in Monte Carlo analysis is to define the parameter distribution and range. For the Project, the parameters are assumed to be log-normally distributed around the mean derived value with assumed standard deviations variable for different parameters (0.5 or 1 order of magnitude). The distributions for each parameter were checked and constrained such that upper or lower ranges do not go beyond ranges in literature for physical constraints. 1400 model realisations were generated, each having differing values of key parameters. The realisations were run and calibration quality was assessed. In this case, models were considered to have an acceptable calibration if they achieved an SRMS value less than 6%. The calibration cut-off criterion of 6% SRMS is 15% higher than the achieved SRMS of the best calibrated model (i.e. the base case model), while being 24% below the SRMS of the foundational ODP model and 29% below that of the MVS model. Of the 1400 model runs, 229 model runs were found to be sufficiently calibrated. These were used in all model scenarios (calibration, cumulative mining, approved mining and no mining) and statistically analysed for uncertainty analysis.

5.1 Parameter Distribution

Table 5-1 through **Table 5-4** show the parameter ranges explored during the sensitivity and uncertainty analysis simulation. Parameters were assumed to possess a log-Normal distribution. The parameter distribution for the converged and calibrated model runs are provided as **Appendix E**.

Table 5-1 Uncertainty Parameter Range for Horizontal Hydraulic Conductivity.

Zone	Layer - Unit	Horizontal Hydraulic Conductivity (m/day)	
		Mean (Log10)	Constraint
1	Layer 1 - Alluvium	1.0	No constraint
2	Layer 1 - Colluvium	0.0	< Kh_Alluvium
3	Layer 2 - Regolith (< 65 mbgl)	0.3	< Kh_Alluvium
4	Layer 2 - Regolith (> 65 mbgl)	-0.1	< Kh_Alluvium
5	Layer 1 & 2 - Tertiary basalt	-0.8	No constraint
6	Layer 3 - Rewan Group (< 65 mbgl)	-3.0	< Kh_Alluvium
7	Layer 3 - Rewan Group (> 65 mbgl)	-3.0	< Kh_Alluvium
8	Layer 3 - Rewan Group Fault	-3.0	< Kh_Alluvium
9	Layer 4 - RCM O/B	-0.7	< Kh_Alluvium
10	Layer 4 - RCM O/B Fault	-2.3	< Kh_Alluvium
11	Layer 5 - Leichhardt Seam	-0.2	< Kh_Alluvium
12	Layer 5 - Leichhardt Seam Fault	-2.6	< Kh_Alluvium
13	Layer 6 - RCM I/B	-2.9	< Kh_Alluvium
14	Layer 6 - RCM I/B Fault	-4.6	< Kh_Alluvium

Zone	Layer - Unit	Horizontal Hydraulic Conductivity (m/day)	
		Mean (Log10)	Constraint
15	Layer 7 - Vermont Seam	-1.6	< Kh_Alluvium
16	Layer 7 - Vermont Seam Fault	-2.0	< Kh_Alluvium
17	Layer 8 - RCM U/B	-3.0	< Kh_Alluvium
18	Layer 8 - RCM U/B Fault	-3.4	< Kh_Alluvium
19	Layer 9 - FCCM O/B	-3.0	< Kh_Alluvium
20	Layer 9 - FCCM O/B Fault	-4.4	< Kh_Alluvium
21	Layer 10 - FCCM Seam	-3.3	< Kh_Alluvium
22	Layer 10 - FCCM Seam Fault	-2.3	< Kh_Alluvium
23	Layer 11 - FCCM U/B	-4.0	< Kh_Alluvium
24	Layer 11 - FCCM U/B Fault	-3.5	< Kh_Alluvium
25	Layer 12 - MCM O/B	-4.0	< Kh_Alluvium
26	Layer 12 - MCM O/B Fault	-4.6	< Kh_Alluvium
27	Layer 13 - MCM Seam	-2.4	< Kh_Alluvium
28	Layer 13 - MCM Seam Fault	-2.3	< Kh_Alluvium
29	Layer 14 - MCM U/B	-3.0	< Kh_Alluvium
30	Layer 14 - MCM U/B Fault	-2.3	< Kh_Alluvium
31	All - Intrusives	-3.0	< Kh_Alluvium

Standard deviation = 1 order of magnitude for all units.

O/B = Overburden.

I/B = Interburden.

U/B = Underburden.

RCM = Rangal Coal Measures.

FCCM = Fort Cooper Coal Measures.

MCM = Moranbah Coal Measures.

Table 5-2 Uncertainty Parameter Range for Anisotropy

Layer - Unit	Anisotropy (Kv/Kx) (Log10)	
	Mean	Constraint
Layer 1 - Alluvium	-0.53	< 0.5
Layer 1 - Colluvium	-0.61	< 0.5
Layer 2 - Regolith (< 65 mbgl)	-2.01	< 0.5
Layer 2 - Regolith (> 65 mbgl)	-1.18	< 0.5
Layer 1 & 2 - Tertiary basalt	-0.75	< 0.5
Layer 3 - Rewan Group (< 65 mbgl)	-1.57	< 0.5
Layer 3 - Rewan Group (> 65 mbgl)	-1.0	< 0.5
Layer 3 - Rewan Group Fault	-2.25	< 0.5
Layer 4 - RCM O/B	-2.38	< 0.5
Layer 4 - RCM O/B Fault	-2.13	No constraint
Layer 5 - Leichhardt Seam	-1.42	< 0.5
Layer 5 - Leichhardt Seam Fault	-1.8	No constraint
Layer 6 - RCM I/B	-1.89	< 0.5
Layer 6 - RCM I/B Fault	-3.0	No constraint
Layer 7 - Vermont Seam	-1.04	< 0.5
Layer 7 - Vermont Seam Fault	-2.3	No constraint
Layer 8 - RCM U/B	-1.1	< 0.5
Layer 8 - RCM U/B Fault	-1.97	No constraint
Layer 9 - FCCM O/B	-2.09	< 0.5
Layer 9 - FCCM O/B Fault	-2.64	No constraint
Layer 10 - FCCM Seam	-1.0	< 0.5
Layer 10 - FCCM Seam Fault	-2.2	No constraint
Layer 11 - FCCM U/B	-1.0	< 0.5
Layer 11 - FCCM U/B Fault	-0.83	No constraint
Layer 12 - MCM O/B	-1.98	< 0.5
Layer 12 - MCM O/B Fault	-2.69	No constraint
Layer 13 - MCM Seam	-1.3	< 0.5
Layer 13 - MCM Seam Fault	-1.19	No constraint
Layer 14 - MCM U/B	-0.73	< 0.5
Layer 14 - MCM U/B Fault	-0.97	No constraint
All - Intrusives	-1.86	< 0.5

Standard deviation = 0.5 orders of magnitude for all units.
mbgl = metres below ground level.

Table 5-3 Uncertainty Parameter Range for Specific Yield

Layer - Unit	Specific Yield (Log10)	
	Mean	Constraint
Layer 1 - Alluvium	-1.3	No constraint
Layer 1 - Colluvium	-2.43	< Sy_Alluvium; < 0.05
Layer 2 - Regolith (< 65 mbgl)	-1.72	< Sy_Alluvium; < 0.15
Layer 2 - Regolith (> 65 mbgl)	-3.0	< Sy_Alluvium; < 0.3
Layer 1 & 2 - Tertiary basalt	-1.53	< 0.1
Layer 3 - Rewan Group (< 65 mbgl)	-2.61	< Sy_Alluvium; < 0.1
Layer 3 - Rewan Group (> 65 mbgl)	-2.33	< Sy_Alluvium; < 0.1
Layer 3 - Rewan Group Fault	-2.74	< Sy_Alluvium; < 0.1
Layer 4 - RCM O/B	-2.9	< Sy_Alluvium; < 0.1
Layer 4 - RCM O/B Fault	-2.53	< Sy_Alluvium; < 0.05
Layer 5 - Leichhardt Seam	-2.77	< Sy_Alluvium; < 0.05
Layer 5 - Leichhardt Seam Fault	-2.82	< Sy_Alluvium; < 0.05
Layer 6 - RCM I/B	-2.8	< Sy_Alluvium; < 0.05
Layer 6 - RCM I/B Fault	-2.41	< Sy_Alluvium; < 0.05
Layer 7 - Vermont Seam	-2.94	< Sy_Alluvium; < 0.05
Layer 7 - Vermont Seam Fault	-2.02	< Sy_Alluvium; < 0.05
Layer 8 - RCM U/B	-2.42	< Sy_Alluvium; < 0.05
Layer 8 - RCM U/B Fault	-2.94	< Sy_Alluvium; < 0.05
Layer 9 - FCCM O/B	-2.77	< Sy_Alluvium; < 0.05
Layer 9 - FCCM O/B Fault	-2.86	< Sy_Alluvium; < 0.05
Layer 10 - FCCM Seam	-2.06	< Sy_Alluvium; < 0.05
Layer 10 - FCCM Seam Fault	-2.25	< Sy_Alluvium; < 0.05
Layer 11 - FCCM U/B	-2.64	< Sy_Alluvium; < 0.05
Layer 11 - FCCM U/B Fault	-2.13	< Sy_Alluvium; < 0.05
Layer 12 - MCM O/B	-2.8	< Sy_Alluvium; < 0.05
Layer 12 - MCM O/B Fault	-2.07	< Sy_Alluvium; < 0.05
Layer 13 - MCM Seam	-2.75	< Sy_Alluvium; < 0.05
Layer 13 - MCM Seam Fault	-2.41	< Sy_Alluvium; < 0.05
Layer 14 - MCM U/B	-2.95	< Sy_Alluvium; < 0.05
Layer 14 - MCM U/B Fault	-2.88	< Sy_Alluvium; < 0.05
All - Intrusives	-4.0	< Sy_Alluvium; < 0.001

Standard deviation = 0.5 orders of magnitude for all units.

Table 5-4 Uncertainty Ranges for Recharge Factor

	Mean (Log10)	Constraints
Unit	% of rainfall	
Stream Channel	0.55	>RCH_Rewan_Group; >RCH_Outcropping_Coal_Measures
Flood Plain Alluvium	0.25	>RCH_Rewan_Group; >RCH_Outcropping_Coal_Measures
Other Alluvium	0.25	>RCH_Rewan_Group; >RCH_Outcropping_Coal_Measures
Tertiary Sediments	0.02	No constraint
Tertiary Basalt	5.01	>RCH_Rewan_Group; >RCH_Outcropping_Coal_Measures
Rewan Group	0.01	No constraint
Outcropping Coal Measures	0.01	No constraint

Standard deviation = 0.5 orders of magnitude for all units.
RCH = Recharge.

5.2 Number of realisations

As discussed in **Section 5.1**, 229 realisations met the calibration criteria and were selected as calibrated realisations. The predictive model was run using the 229 parameter sets. The results from the predictive model were used to conduct statistical analyses to assess if additional realisations were likely to provide results that would significantly change the reported predictive results. The 95% confidence interval was calculated for the mine inflows and the maximum drawdown.

Figure 5-1 and **Figure 5-2** show the 95% confidence intervals of the median and maximum drawdown and predicted inflows, as well as the variance of the median and maximum drawdown and predicted inflows as more realisations are added to the uncertainty analysis. For example, the 95% confidence interval for the maximum drawdown is calculated by first estimating the maximum drawdown for each realisation and then calculating the 95% confidence interval of the maximum drawdowns as each realisation is added to the dataset. As shown in **Figure 5-1** and **Figure 5-2**, additional realisations are unlikely to significantly increase or decrease the confidence intervals of predictions of mine inflows and maximum drawdowns. Therefore, the results from the 229 realisations can be considered representative and used for predicted drawdown and indirect water take (alluvium and surface water).

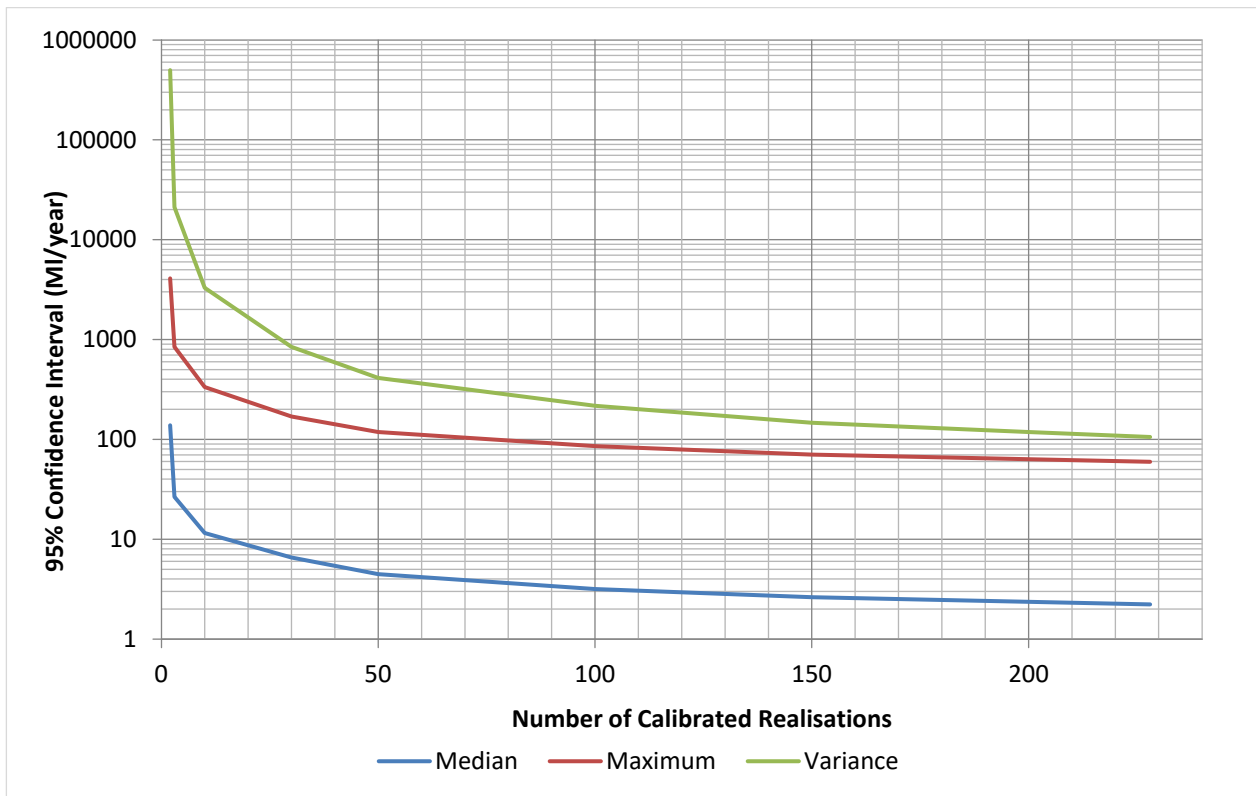


Figure 5-1 95% Confidence Intervals for Mine Pit Inflows

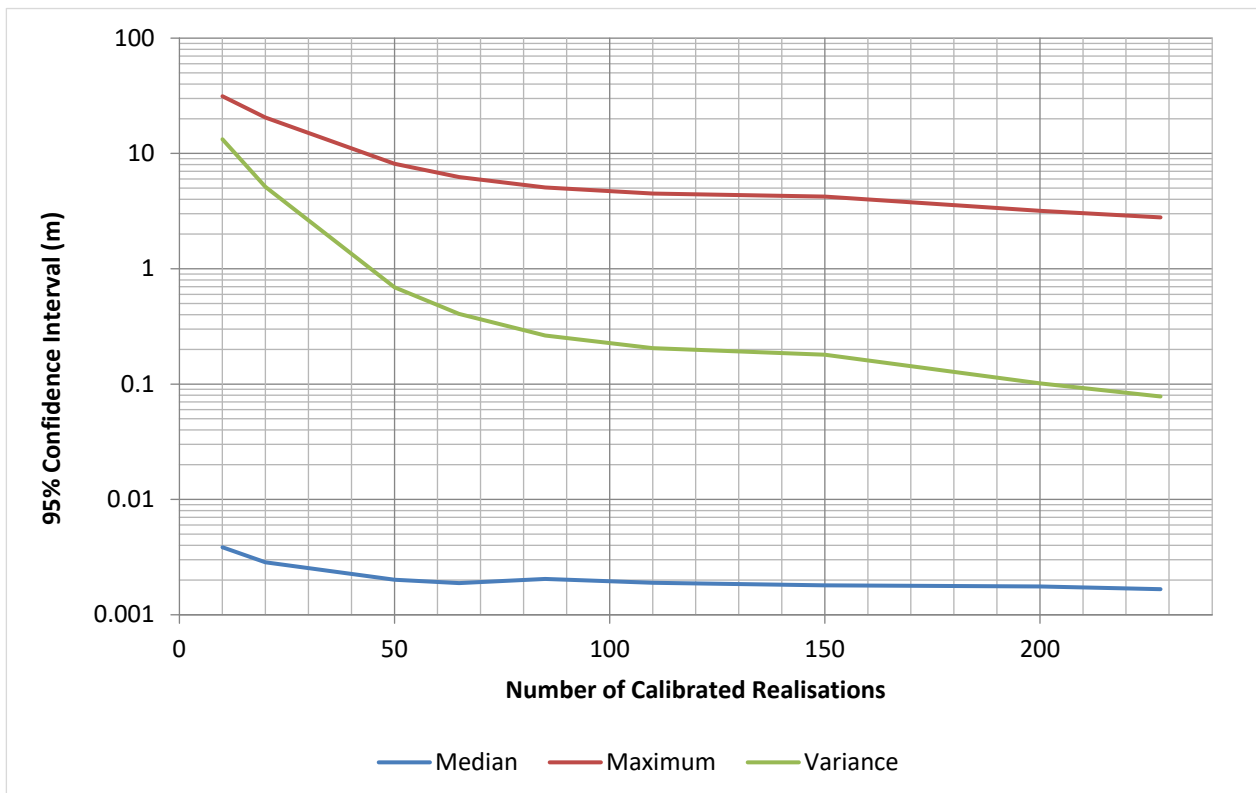


Figure 5-2 95% Confidence Intervals for Maximum Drawdowns

5.3 Sensitivity Analysis

5.3.1 Calibrations Identifiability and Sensitivity Analysis

Identifiability describes a parameter's capability to be constrained by the model calibration. Identifiability values range from zero to one. As identifiability approaches one, the parameter is increasingly able to be constrained. Likewise, as values approach zero the parameter is increasingly unable to be constrained by the calibration and uncertainty of model results is not reduced through calibration.

The PEST utility GENLINPRED was used to provide an estimate of parameter identifiability for each of the model parameters. Estimated identifiability values for the calibrated parameters horizontal hydraulic conductivity, Anisotropy, Specific yield and recharge are summarised in **Figure 5-3** through **Figure 5-6**.

Figure 5-3 indicates that the horizontal hydraulic conductivity of faults generally has not been able to be constrained well during calibration, relative to their surrounding unit. The exception to this is the Fort Cooper Coal Measures coal seam fault zone, which has been constrained much better than the seam in which it is located. Notably, the colluvium, Rewan Group, Vermont Seam and Fort Cooper Coal Measures units are less constrained by calibration. While all other units have high identifiability values (equal to or above 0.90, with the exception of Moranbah Coal Measures underburden).

Identifiability of hydraulic conductivity anisotropy for model zones is presented in **Figure 5-4**. Anisotropy in the Rangal Coal Measures interburden, Fort Cooper Coal Measures overburden and Moranbah Coal Measures overburden have high identifiability values indicating these are able to be constrained, and contribute to reducing model uncertainty. All other zones feature low values (equal to and below 0.40) and are less constrained by calibration.

Specific yield shows high identifiability for the alluvium (**Figure 5-5**). Alluvium is a sensitive receptor within the model domain, and so, the high value is desirable, as it indicates calibration has constrained this variable and gives confidence to model predictions for how mining impacts the unit (i.e. drawdown, baseflow changes and indirect take predictions). Specific yield of other zones in the model domain has low identifiability.

The recharge zones for Tertiary sediments, alluvium (excluding stream channel alluvium) and Tertiary basalt are prevalent across the model domain and are highly constrained by the calibration. The other zones have low identifiability (**Figure 5-6**). Note that the stream channel alluvium represents a narrow zone along the Isaac River, with a small area relative to the other recharge zones. It is, therefore, considered less impactful to model predictions.

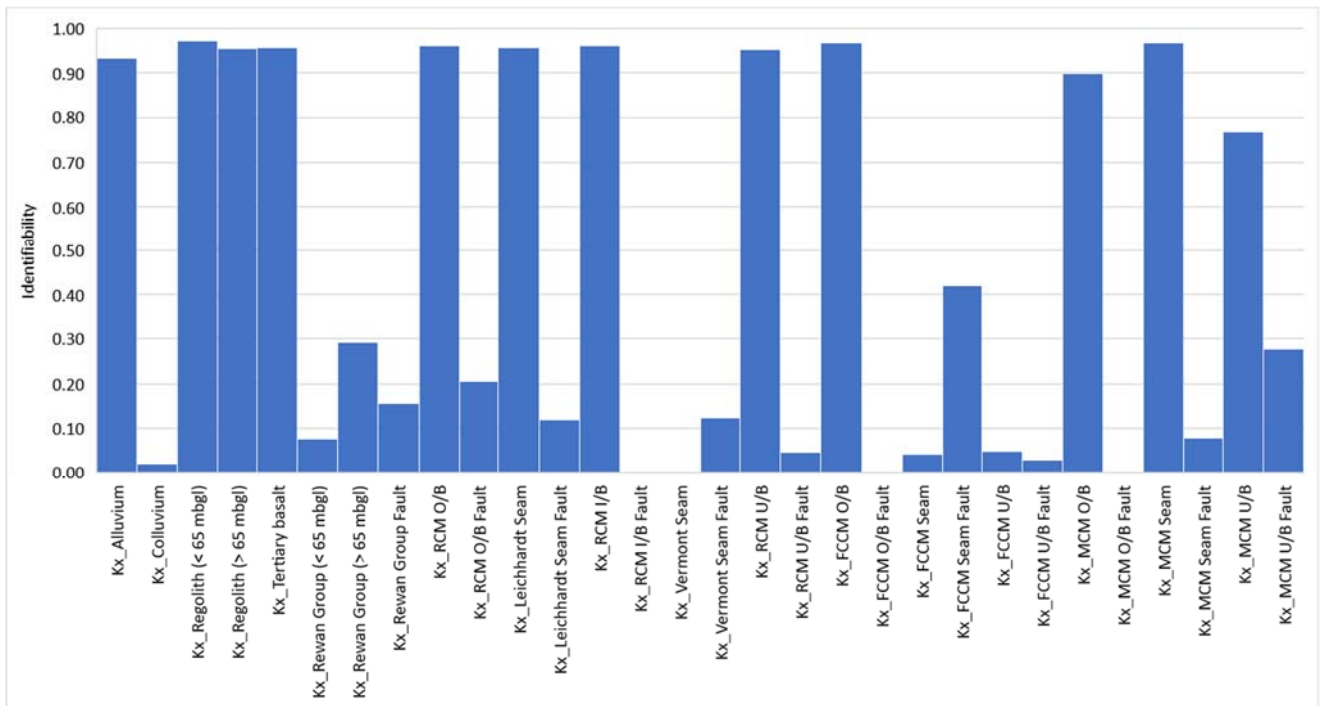


Figure 5-3 Identifiability – Horizontal Hydraulic Conductivity (Kh)

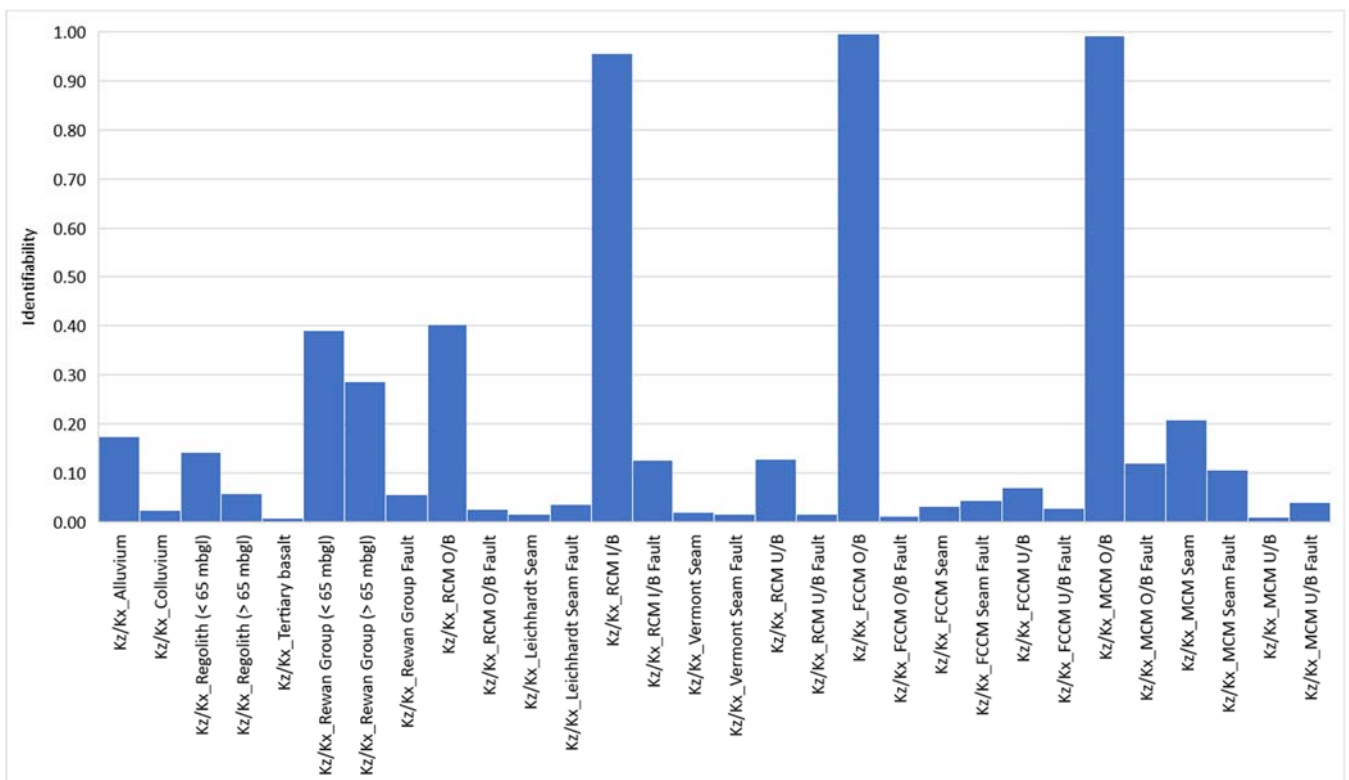


Figure 5-4 Identifiability – Anisotropy (Kv/Kx)

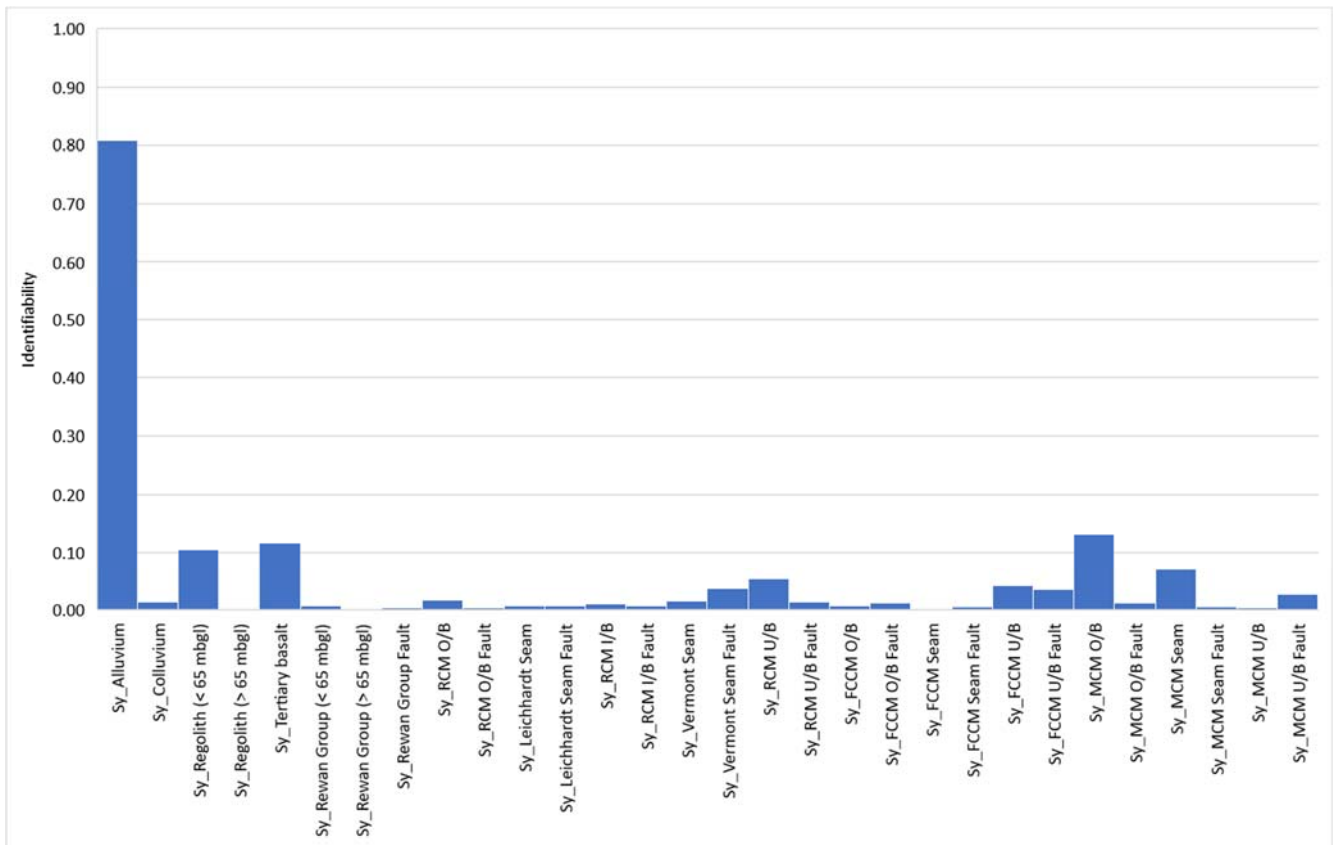


Figure 5-5 Identifiability – Specific Yield (Sy)

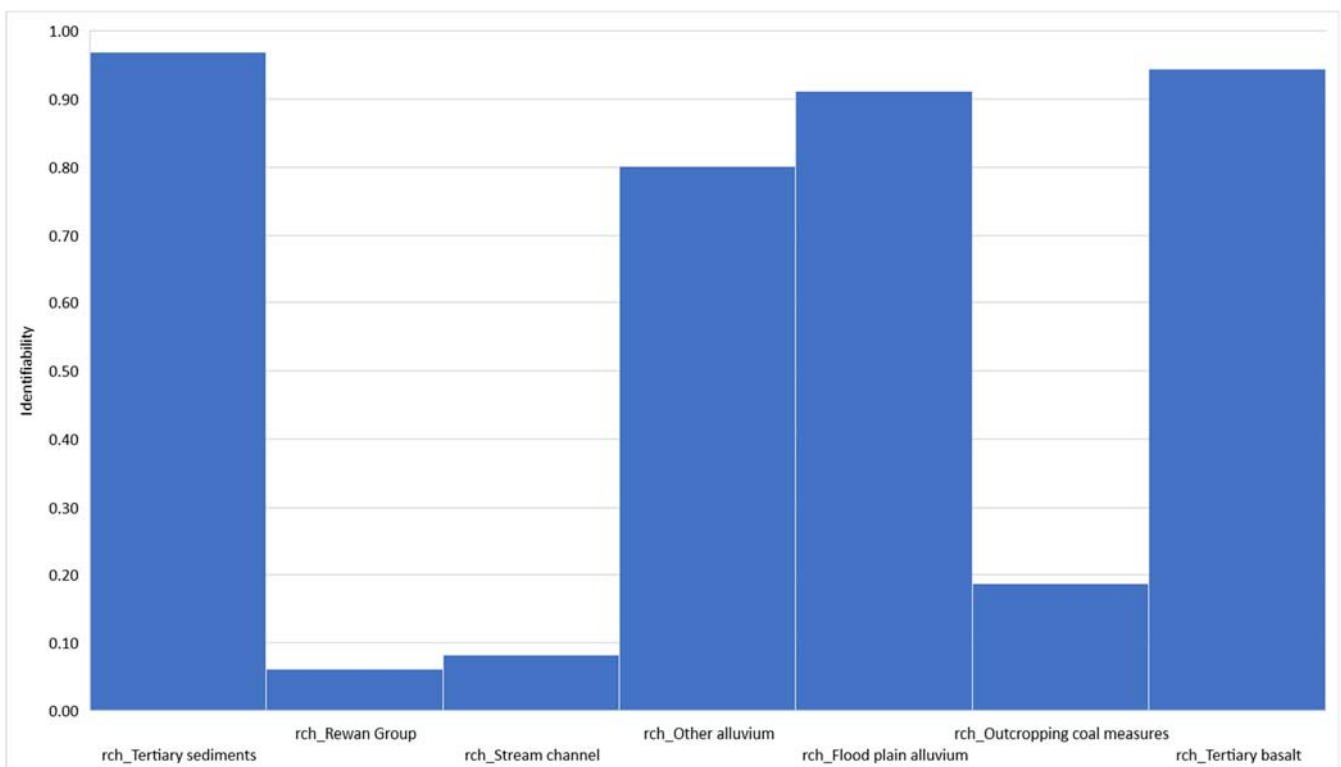


Figure 5-6 Identifiability – Recharge

5.3.2 Predictive Sensitivity Analysis

Graphs showing sensitivity derivatives for model parameters are included in **Appendix F**. “In strict mathematical terms, a sensitivity measures how fast one quantity changes when another changes. A sensitivity is the derivative, or slope, of a function” (Barnett et al, 2012). For the purposes of assessing parameter predictive sensitivity, SLR has calculated the sensitivity derivative of each parameter to mine inflows, alluvial drawdown magnitude and extent, and coal seam drawdown extent. Parameter and predicted values were standardized as standard deviation from the mean prior to calculating the derivatives. The derivative was calculated as the slope of the linear regression line through the predicted standardized values (y values) and the parameter standardized values (x values). The sensitivity to model parameters for the following has been investigated:

- maximum drawdown impacted area extents for alluvium/colluvium;
- maximum drawdown impacted area extents for the Vermont Seam;
- Project open cut pit inflows; and
- maximum drawdown to the alluvium has been investigated.

Drawdown impact area of the alluvium/colluvium is shown to be most sensitive to the horizontal hydraulic conductivity of the colluvium. For the Vermont Seam, impact area is most sensitive to horizontal hydraulic conductivity of the Vermont Seam. The Project open cut pit inflows are most sensitive to the specific yield of the Rangal Coal Measures interburden. Maximum drawdown in the alluvium appears to be most sensitive to the recharge to Tertiary sediments.

5.4 Uncertainty Results

5.4.1 Uncertainty of Mine Inflows

Figure 5-7 presents the uncertainty of groundwater inflow rates to the Project from 2020 to 2055. The figure presents the inflow to the proposed open pit mine for the reported base case model, along with the 5th, 50th and 95th inflow percentiles. The base case model falls below the 50th percentile inflow value for most the Project duration, i.e. the majority of the 229 model realisations had inflows consistently above those reported in **Section 3.5**. The maximum mine inflow rate predicted by the uncertainty analysis is 381 ML/year (1.04 ML/day) and occurs in 2032. Total inflows for the base case model and the different percentiles are provided in **Table 5-5**. The uncertainty analysis indicates a 95% confidence that the total inflow to the Project over the life of the Project will be within 59.5 ML of the mean total inflow (4,880 ML) calculated across the 229 realisations.

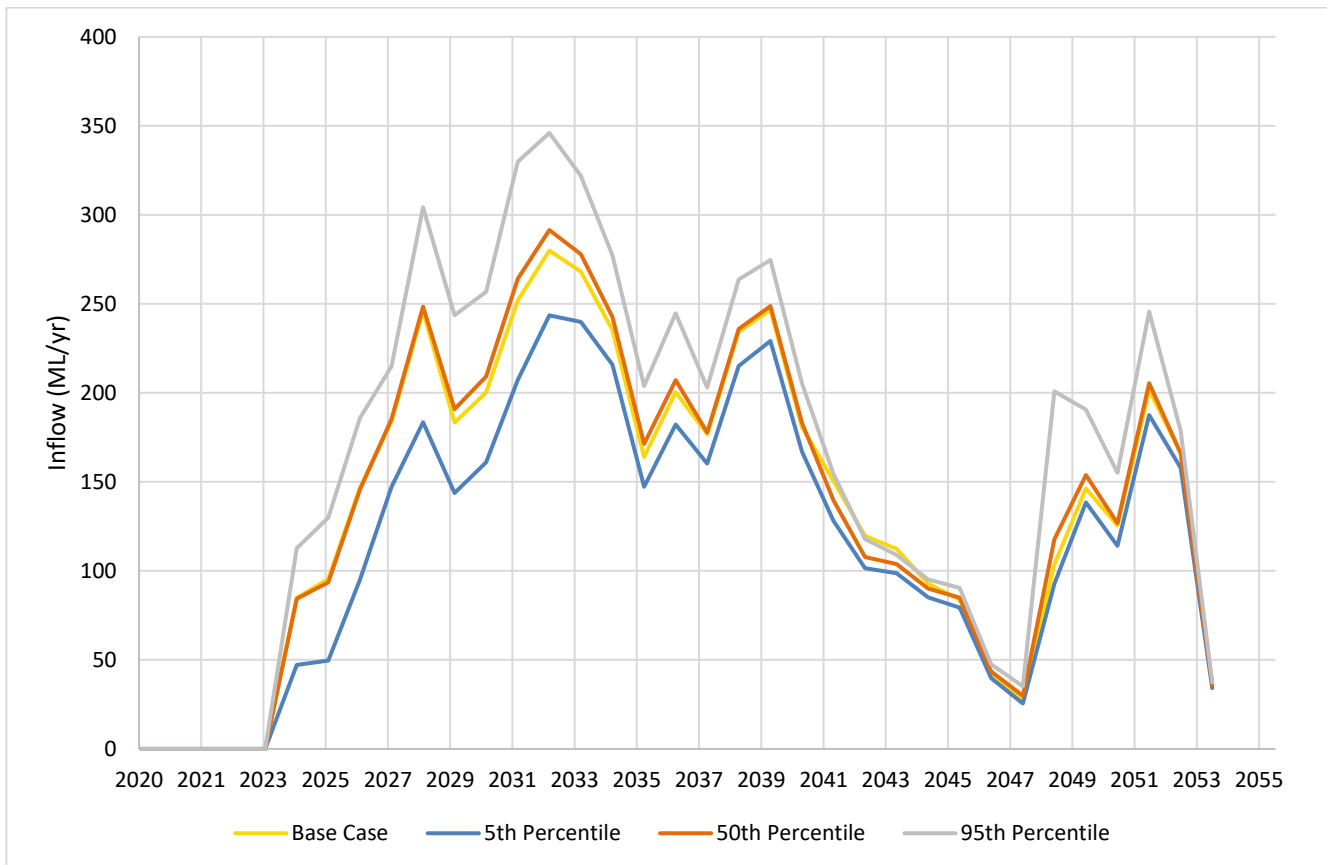


Figure 5-7 Uncertainty Analysis – Predicted Project Mine Inflows

Table 5-5 Total Predicted Inflows Over Life of the Project

	Base Case	5 th Percentile	50 th Percentile	95 th Percentile
Total Inflow (ML)	4,784 ML	4,116 ML	4,862 ML	5,774 ML

5.4.2 Uncertainty of Influence on Alluvium

Uncertainty analysis surrounding the Project's influence on alluvium via take from 2020 to 2055 was carried out using the 229 available model realisations. Note that any predicted take is indirect, as mined areas are outside the Pit extents of the Project. 58% of realisations (132 of 229 realisations) showed zero loss from alluvium. The 5th, 50th and 95th percentiles were calculated for total alluvial take volume over the life of the project. Of these, only the 95th percentile indicated any take from alluvium over the life of the Project (0.74 ML).

5.4.3 Groundwater Drawdowns

5.4.3.1 Groundwater Probability Drawdown Extents

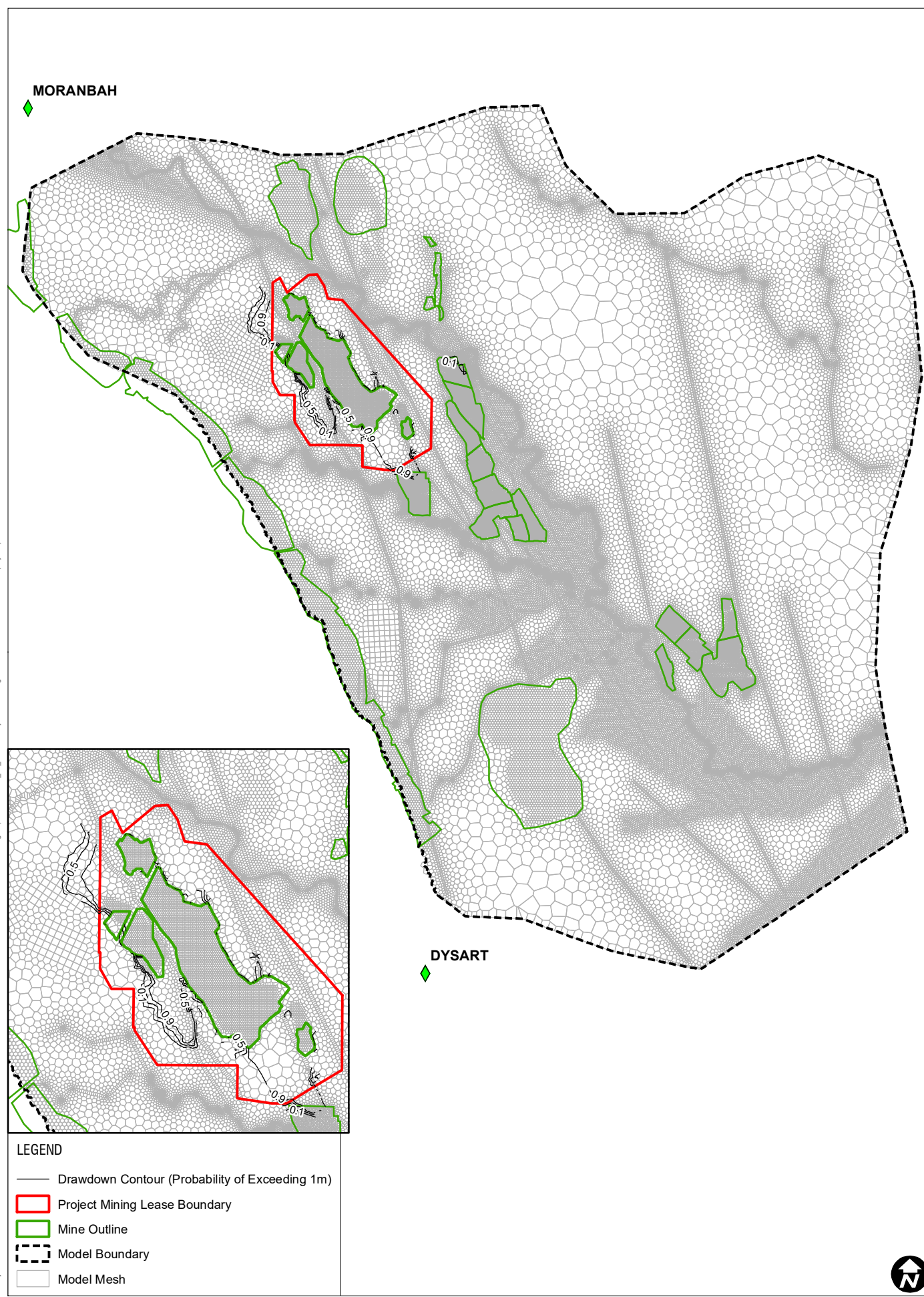
To illustrate the level of uncertainty in the extent of predicted incremental drawdown, maximum drawdown probability contour maps were generated for incremental drawdown to Layer 1 (colluvium and Quaternary alluvium) and Layer 7 (Vermont seam). These contours represent the probability of maximum aquifer drawdown at any location exceeding 1 m, as a result of mining at the Project. Probability maps are based on the 229 available model realisations.

Layer 1 drawdown probability contours are shown in **Figure 5-8**. This shows a small zone of potential drawdown to the Quaternary alluvium, near the northern boundary of MDL183. This zone represents a 10 % drawdown probability and is 390 m across, at its widest extent. It is located approximately 70 m from the edge of the mapped Quaternary alluvium, along the Isaac River. No private landholder bores, intersecting the alluvium or colluvium, are known to occur within this zone.

Vermont Seam drawdown probability contours are shown in **Figure 5-9** for the 10%, 50% and 90% probability drawdown impact zones. Model structure influences probability contour distribution, with greater distance between contours occurring to the north, where drawdown is able to propagate. The general contour distribution is similar to what was observed in **Figure 3-10** for the base case maximum incremental drawdown map. No private landholder bores, intersecting the Vermont seam, are known to occur within the zone covered by the drawdown probability map.



H:\Projects\SLR\620-BNE\620-13-245 Winchester South Groundwater\SLR Data\OT CAD\GIS\ArcGIS\Revised Mine Plan Modelling Report\62013245_P5_9 Probability of Exceeding 1m Drawdown to Vermont Seam (Layer 7).mxd



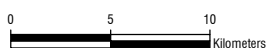
LEGEND

- Drawdown Contour (Probability of Exceeding 1m)
- ▭ Project Mining Lease Boundary
- ▭ Mine Outline
- - - Model Boundary
- ▭ Model Mesh

Scale: 1:380,000
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6 Limitations and Recommendations

Site geological models were available for the Project Area as well as for Moorvale South and Olive Downs Project sites. Model geology at these locations is, therefore, reliable. However, elsewhere within the model domain, geology has been interpolated and estimated from publicly available data; including regional scale mapping (e.g. Qld Government mapping and EIS documentation [including the BGP]). Consequently, the depths, thickness and extents of the model layers away from the mentioned site geological models (the Project, Moorvale South Project and the Olive Downs Project) may not closely replicate reality. This limitation is important to note as inaccurate geology at surrounding mines may result in over or under prediction of impacts when considering the cumulative impacts of mining in the Study Area.

Additionally, the timings and active extents of surrounding mines in the model (excluding Moorvale South and Olive Downs Projects where mine plans were available) have been largely inferred from publicly available data. Therefore, the simulation of these mines is unlikely to be entirely accurate, and potential over- or under-estimation of impacts, or timing of impacts may result due to this. It is recommended that the timings and extents of surrounding mines simulated in the model be updated as new information on these sites becomes available.

The inaccuracies involved in the modelling of surrounding mines, as noted above, combined with the large scale and complexity of the groundwater model has resulted in some model inaccuracies, which manifested as isolated targeted drawdowns at Poitrel seemingly caused by the incremental impacts at the Project. However, the lateral separation of the isolated drawdowns at Poitrel from the drawdowns at the Project indicated that these were not true impacts. The model was subjected to thorough quality control processing and the conclusion was made that the drawdowns at Poitrel likely resulted from inaccuracies surrounding how the mining and geology at Poitrel has been simulated. The decision was, therefore, to exclude Poitrel mining activities from the calibration and predictive simulation periods, for the impact assessment results relating to the direct impacts of the Project (i.e. incremental drawdowns, pit inflows, indirect alluvial take and changes in baseflow).

The coal seams of the Fort Cooper Coal Measures and Moranbah Coal Measures are simplified to single seams with aggregated seam thickness; as mining is applied conservatively to the base of this simplified seam, the simulated depths of the surrounding mines targeting these units may not be accurate, and the model stresses exaggerated. As these seams are not intercepted by planned mining operations at the Project, this simplification of the geology is considered appropriate for the purpose of assessing potential impacts caused by the Project.

Limited site-specific information on hydraulic conductivities and storage parameters were available during calibration. As more site-specific hydraulic data becomes available, new data should be compared with the calibrated parameters achieved and the validity of the model calibration should be assessed. Additional site specific data is expected to “tighten” uncertainty bounds for model prediction results.

Future revisions of the model should feature simplified hydraulic zone distributions. Currently, multiple zones are used to simulate Isaac River alluvium. This is a redundancy carried forward from the foundational model. Reducing the number of zones will improve the efficiency of stochastic runs during sensitivity and uncertainty analysis.

Predictive sensitivity indicates that mine inflows are most sensitive to the specific yield values of the Rewan Group, Rangal Coal Measures interburden and the Vermont Seam. However, calibration sensitivity to these parameters is relatively low. Future work should consider opportunities to further constrain values of these parameters.

Mine cells at the Project assume a two-year operational window and have been based on annual stage plans provided to SLR by Whitehaven Coal in February 2022. Any variation to the simulated mine plan should be addressed in coming model iterations, to ensure mine impacts are captured to the best approximation.

7 Conclusions

The numerical groundwater model developed for the Project successfully achieved the modelling objectives, as outlined in **Section 1**. Model calibration statistics are within suggested guidelines (Middlemis *et al.*, 2001) and mass balance errors remain low, through the model calibration and predictive modelling. Model construction considers all available data, including the current site mine plan and site geological model for the Project Area. Mine inflows are expected to be average 155 ML/year over the life of Project, with negligible alluvial or river take. Uncertainty analysis has demonstrated a low likelihood for the Project to impact on alluvial water levels, with drawdown to layers mostly contained within the Project Area. The model serves as a suitable representation of possible transient groundwater conditions within the Study Area, over the life of the Project, however, the uncertainty in predictions should be acknowledged.

8 References

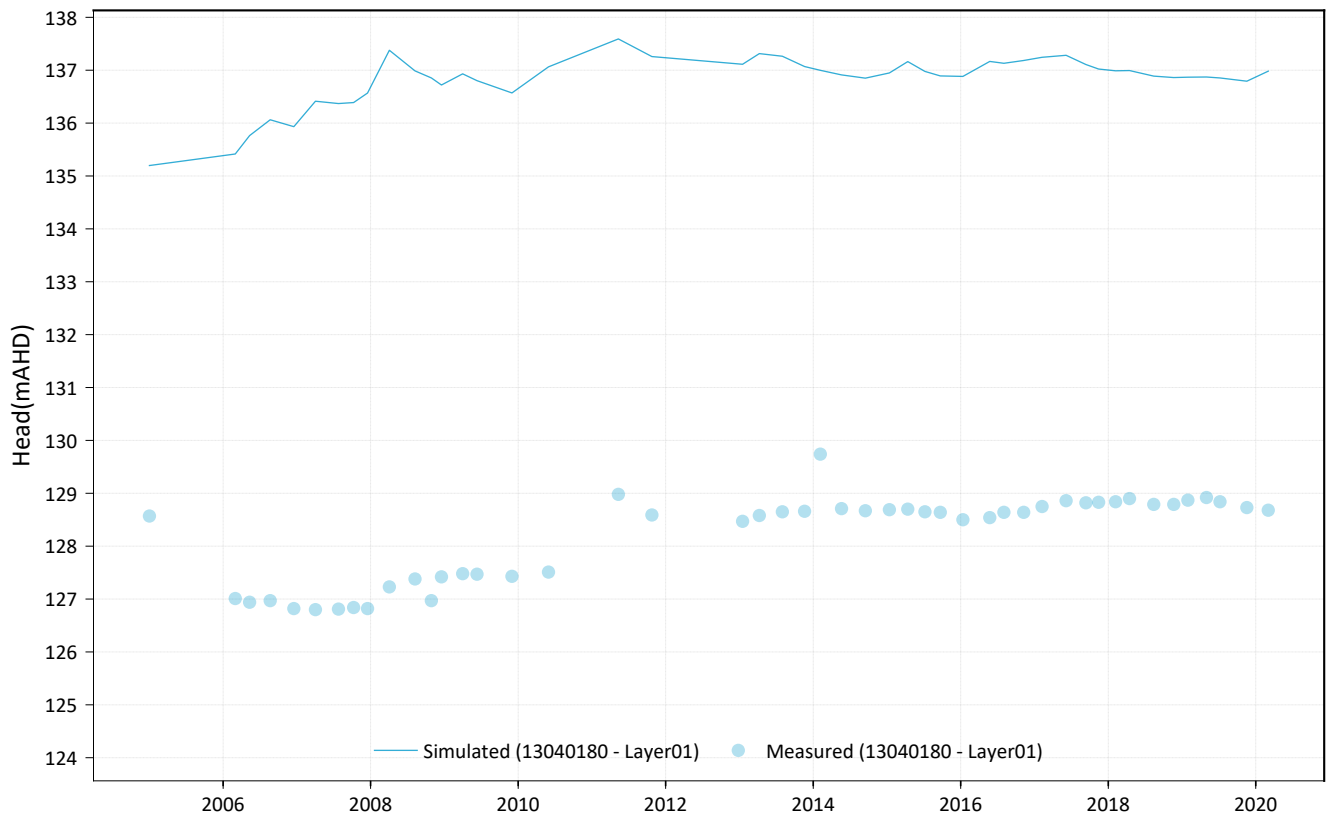
- Barnett, B., Townley, L.R., Post, V., Evans, R.E., Hunt, R.J., Peeters, L., Richardson, S., Werner, A.D., Knapton, A. and Boronkay, A., 2012. Australian Groundwater Modelling Guidelines. Waterlines Report Series No. 82, June 2012.
- Coffey 2014, Supplementary Groundwater Assessment - Arrow Energy Bowen Gas Project. Supplementary Report to the EIS, Document ENAUBRIS107043AC, April 2014.
- Doherty, J., 2010. PEST - Model Independent Parameter Estimation. User Manual 5th Edition. Watermark Numerical Computing.
- Groundwater Imaging Pty Ltd (Allen, D), 2018. Moorvale South AgTEM survey for Groundwater Investigation, near Coppabella, Bowen Basin, QLD. Report for Peabody Energy. 2018.
- Hansen Bailey, 2016. *Isaac Plains East Project – Appendix IV Numerical Groundwater Modelling Report*. Prepared by Klohn Crippen Berger, October 2016.
- Hawkins, J.W., 1998. Hydrogeologic characteristics of surface-mine spoil. In Coal Mine Drainage Prediction and Pollution Prevention in Pennsylvania, ed. Brady, Smithy and Schueck. Harrisburg, Pennsylvania. Available at: <http://www.techtransfer.osmre.gov/nttmainsite/Library/pub/cmdpppp/chapter3.pdf>.
- HydroAlgorithmics, 2018. AlgoCompute web site, <https://www.algocompute.com/>. Accessed 8 May, 2018.
- Hydrogeologist.com.au, 2019. *Hydrogeological Drilling Report Winchester South Project*. Report for Whitehaven Coal Ltd, November 2019.
- HydroSimulations, 2018. Olive Downs South and Willunga. Appendix B. Modelling Appendix. Report for Pembroke Resources Pty Ltd. 2018.
- Independent Expert Scientific Committee on Coal Seam Gas and Large Coal Mining Development, 2018. Information guidelines for proponents preparing coal seam gas and large coal mining development proposals. May 2018.
- Mackie, C.D., 2009. Hydrogeological characterisation of Coal Measures and overview of impacts of coal mining on groundwater systems in the upper Hunter Valley of NSW. PhD thesis at University of Technology, Sydney.
- Merrick, D. and Merrick, N., 2015. AlgoMesh: A new software tool for building unstructured grid models. In Proc. MODFLOW and More, Golden, Colorado.
- Merrick, D., 2017. AlgoCompute: Large-scale calibration and uncertainty analysis made easy in the cloud. In Proc. MODFLOW and More, Golden, Colorado.
- Middlemis, H., Merrick, N., and Ross, J., 2001. Murray-Darling Basin Commission – Groundwater Flow Modelling Guideline. Report for Murray-Darling Basin Commission. January 2001.

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- USEPA, 2009. Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities: Unified Guidance, EPA 530-R-09-007, Washington, DC: Office of Resource Conservation and Recovery, Program Implementation and Information Division, United States Environmental Protection Agency.
- Panday, S., Langevin, C.D., Niswonger, R.G., Ibaraki, M., and Hughes, J.D., 2013., MODFLOW–USG version 1: An unstructured grid version of MODFLOW for simulating groundwater flow and tightly coupled processes using a control volume finite-difference formulation: USGS Techniques and Methods 6-A45.
- S. S. Papadopoulos & Associates, Inc., 2018. mod-PATH3DU, version 2.1.2. Rockville, Maryland.
- SLR, 2019. Moorvale South Groundwater Assessment. Appendix B1 Groundwater Modelling Technical Appendix. Report for Peabody Energy. 2019.
- SLR, 2022. Winchester South Project EIS Groundwater Impact Assessment. Report for Whitehaven Coal.
- WRM Water & Environment, 2022. Winchester South Project - Surface Water and Flooding Assessment. Report for Whitehaven Coal Limited.

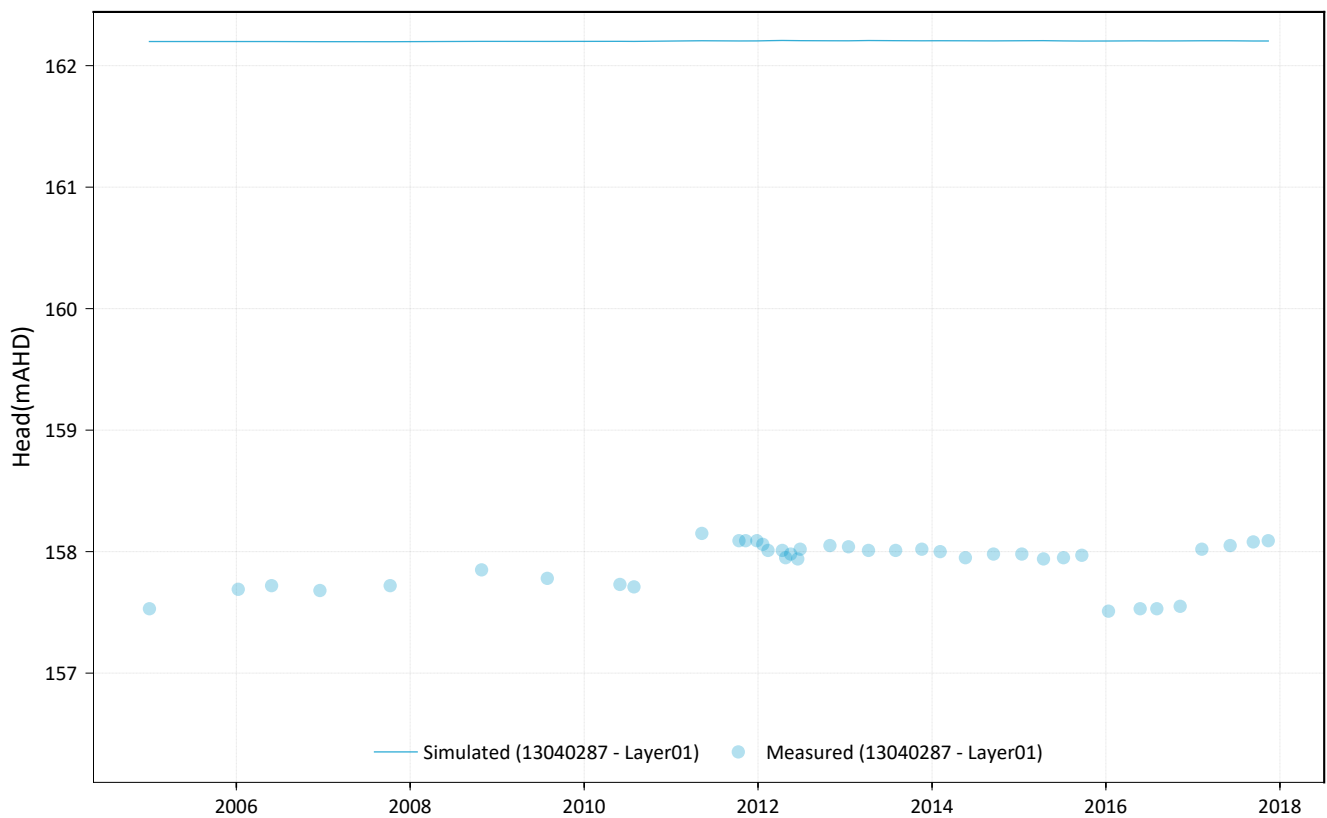
APPENDIX A

Calibration Bore Hydrographs

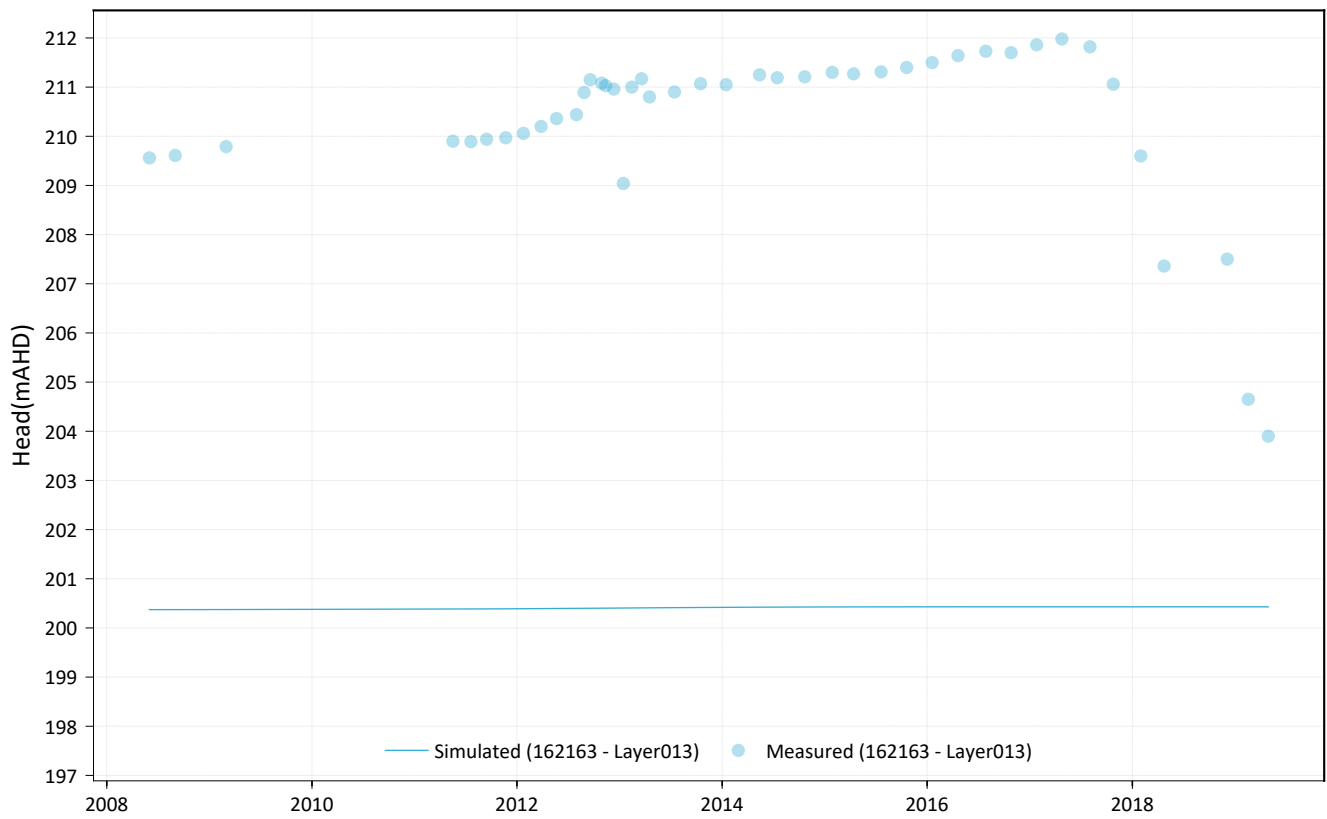
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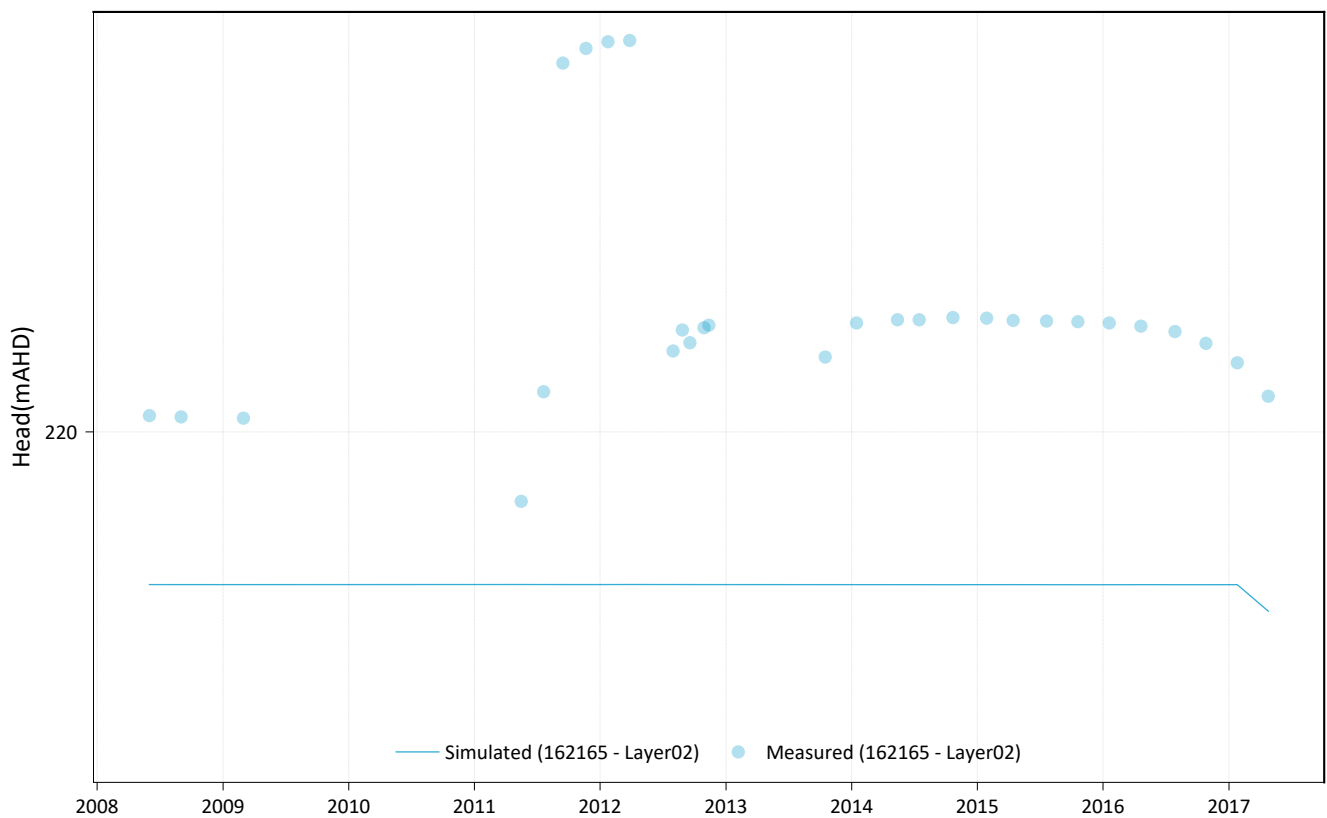
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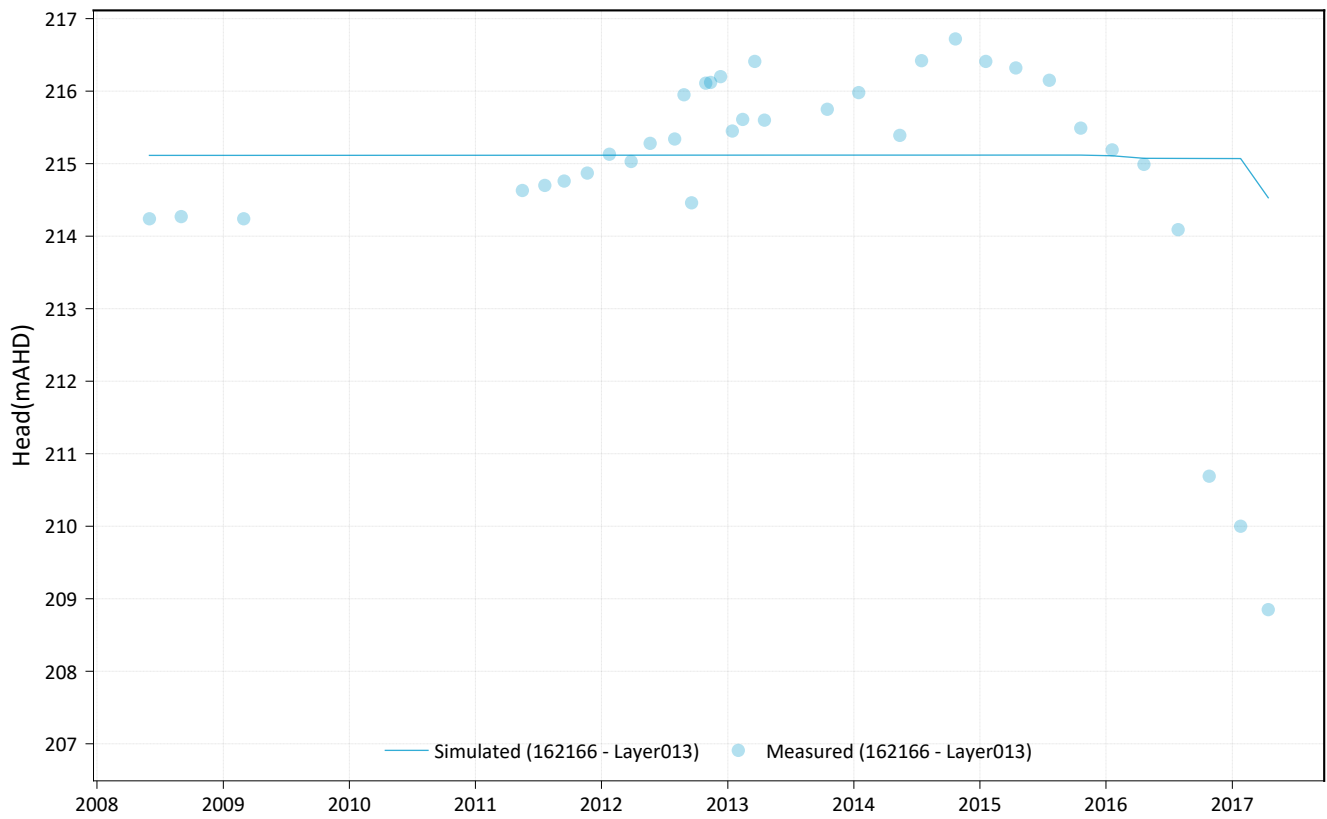
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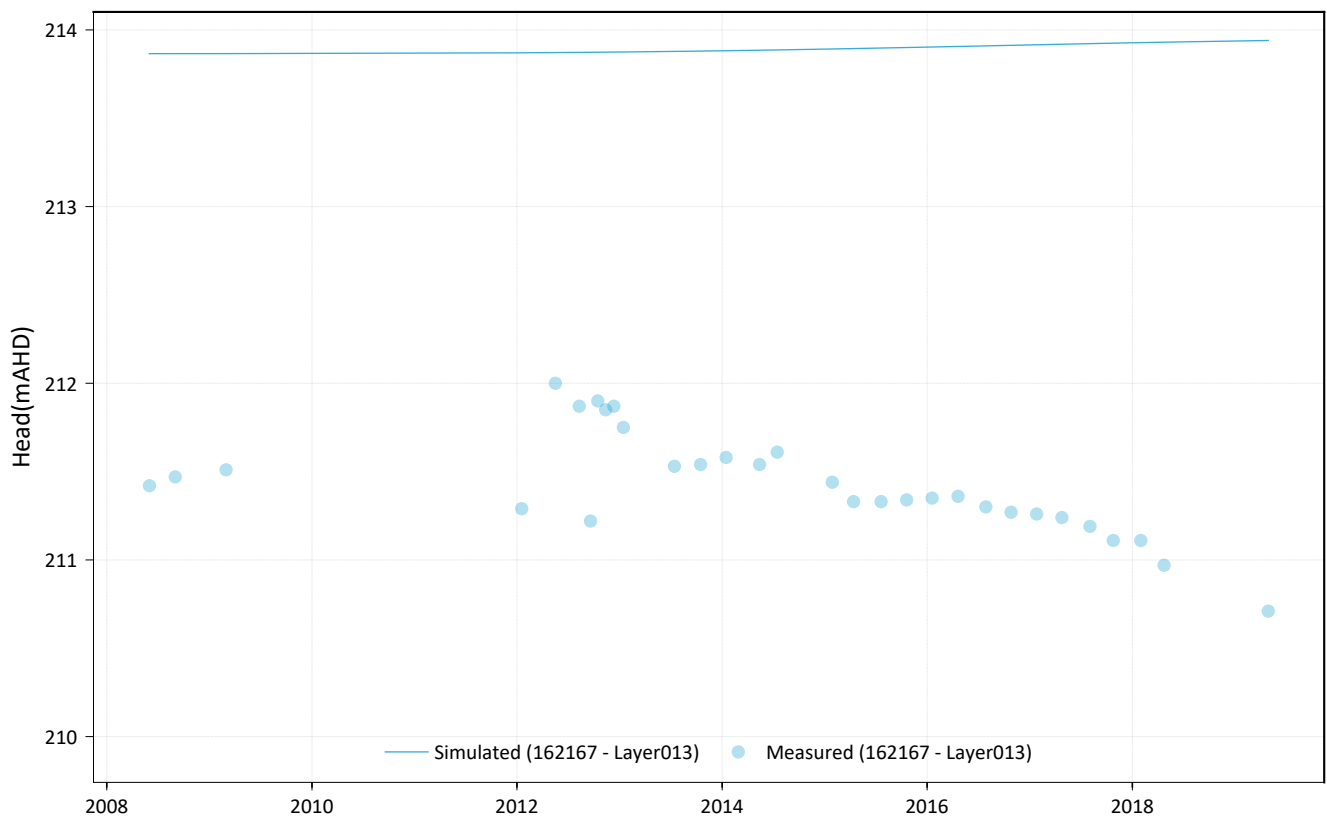
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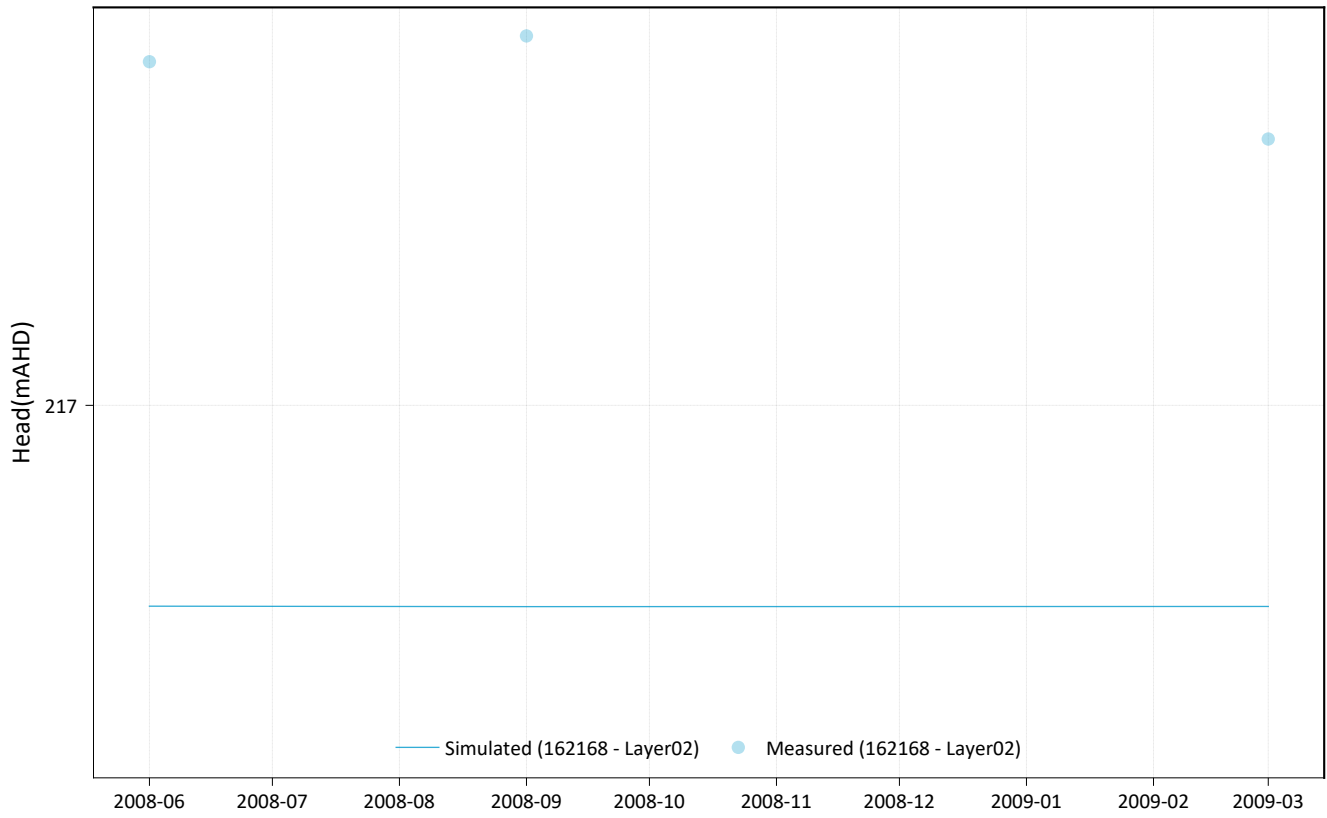
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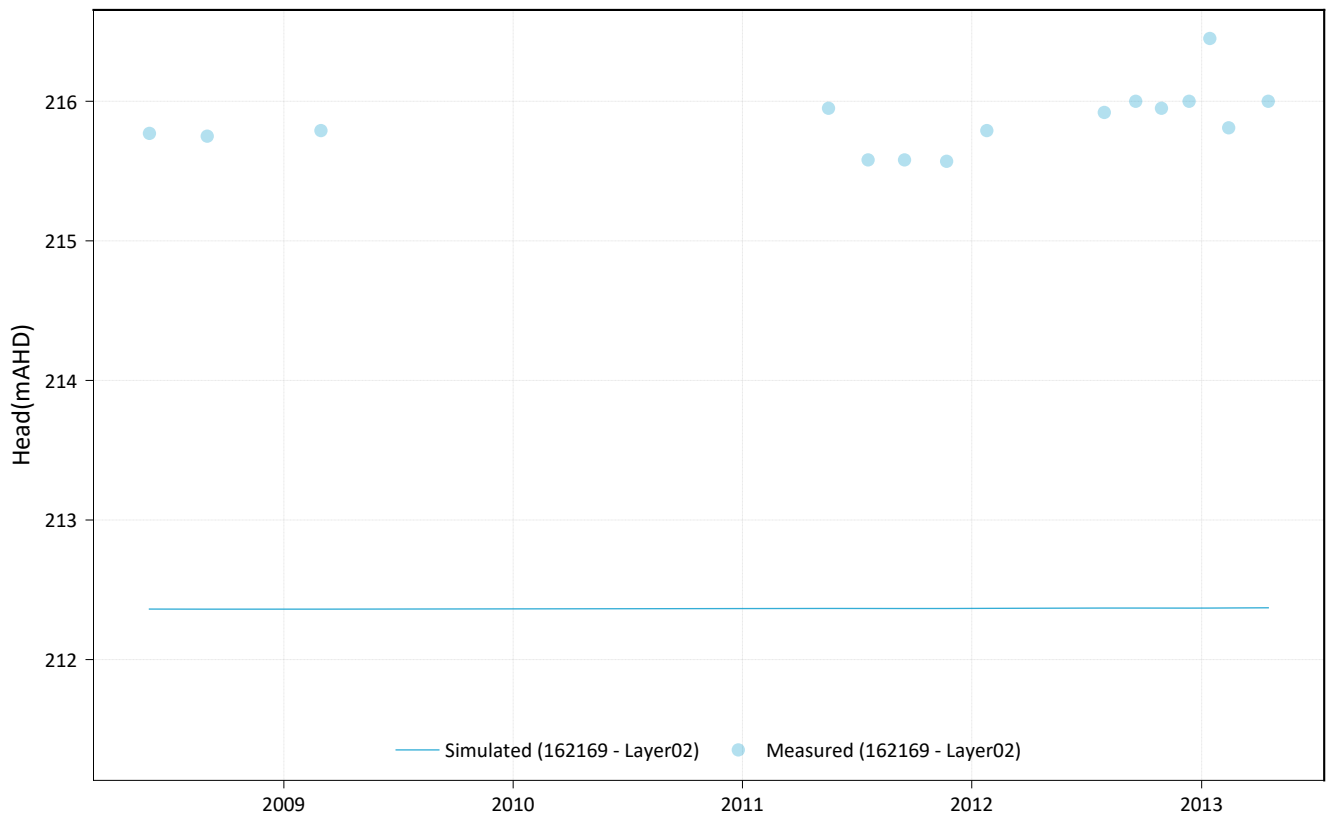
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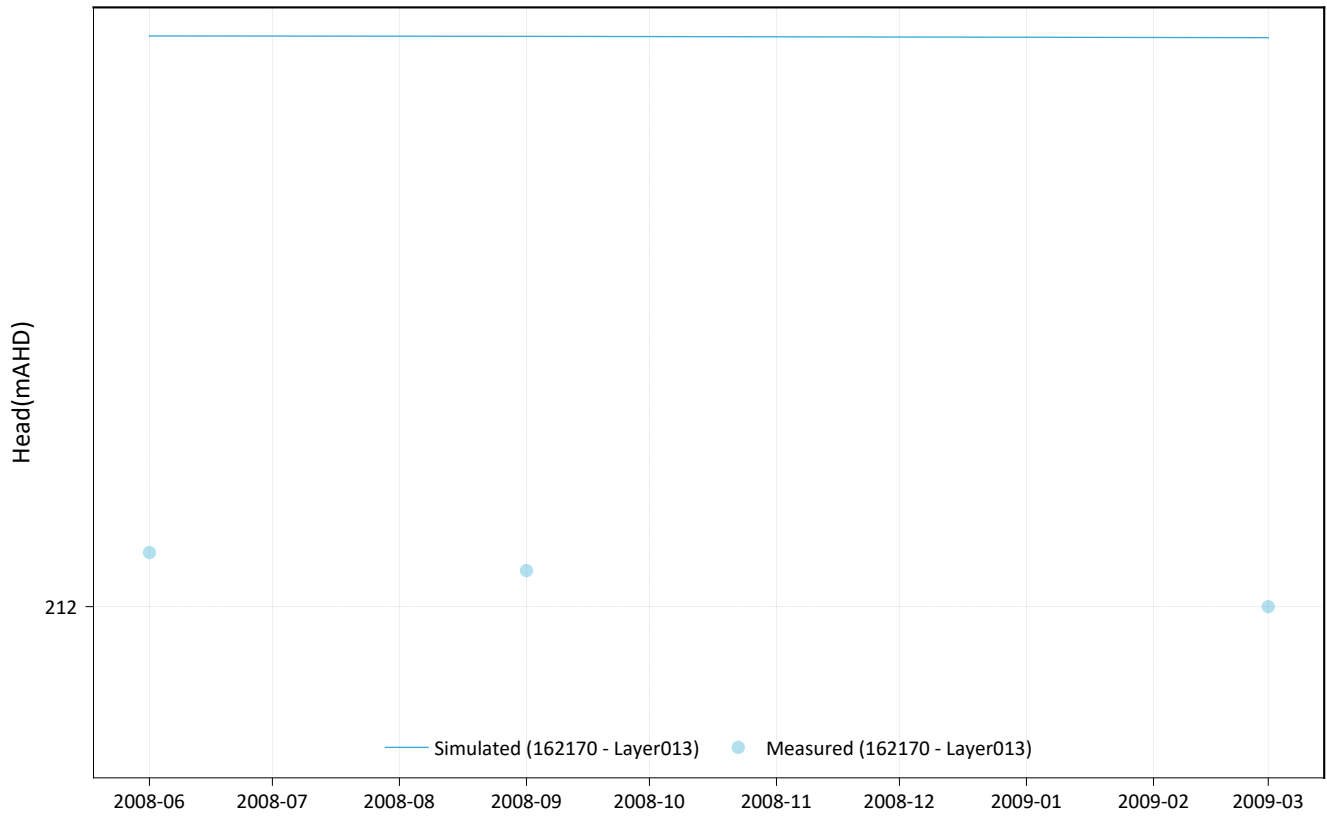
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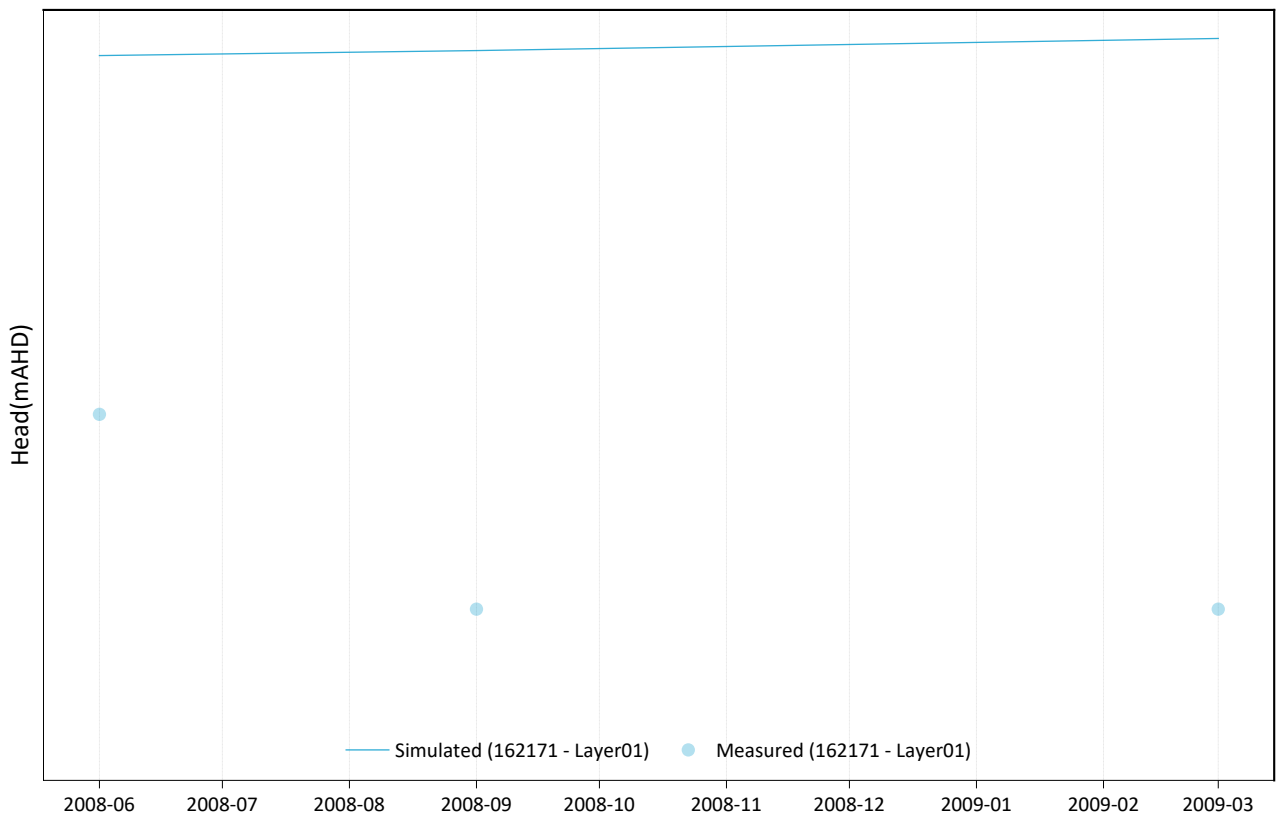
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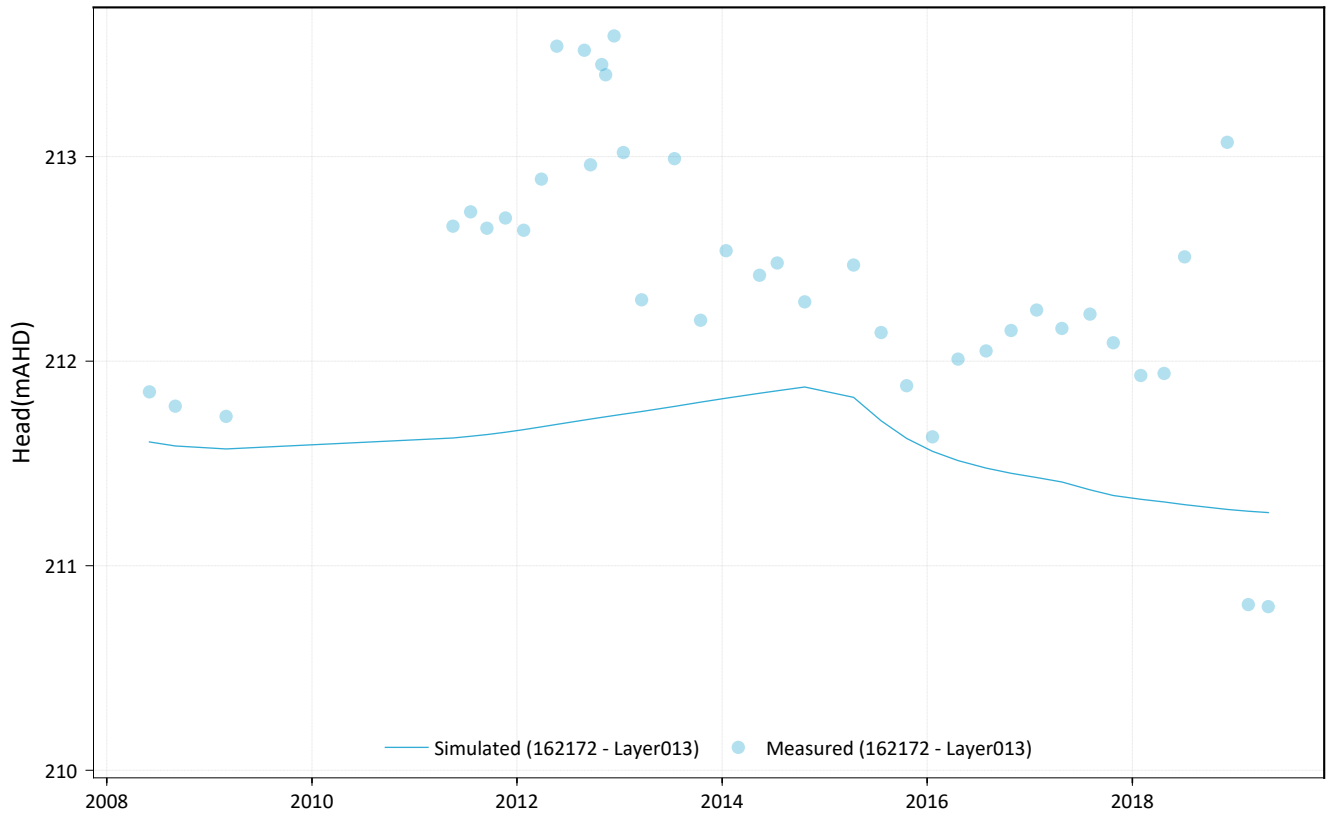
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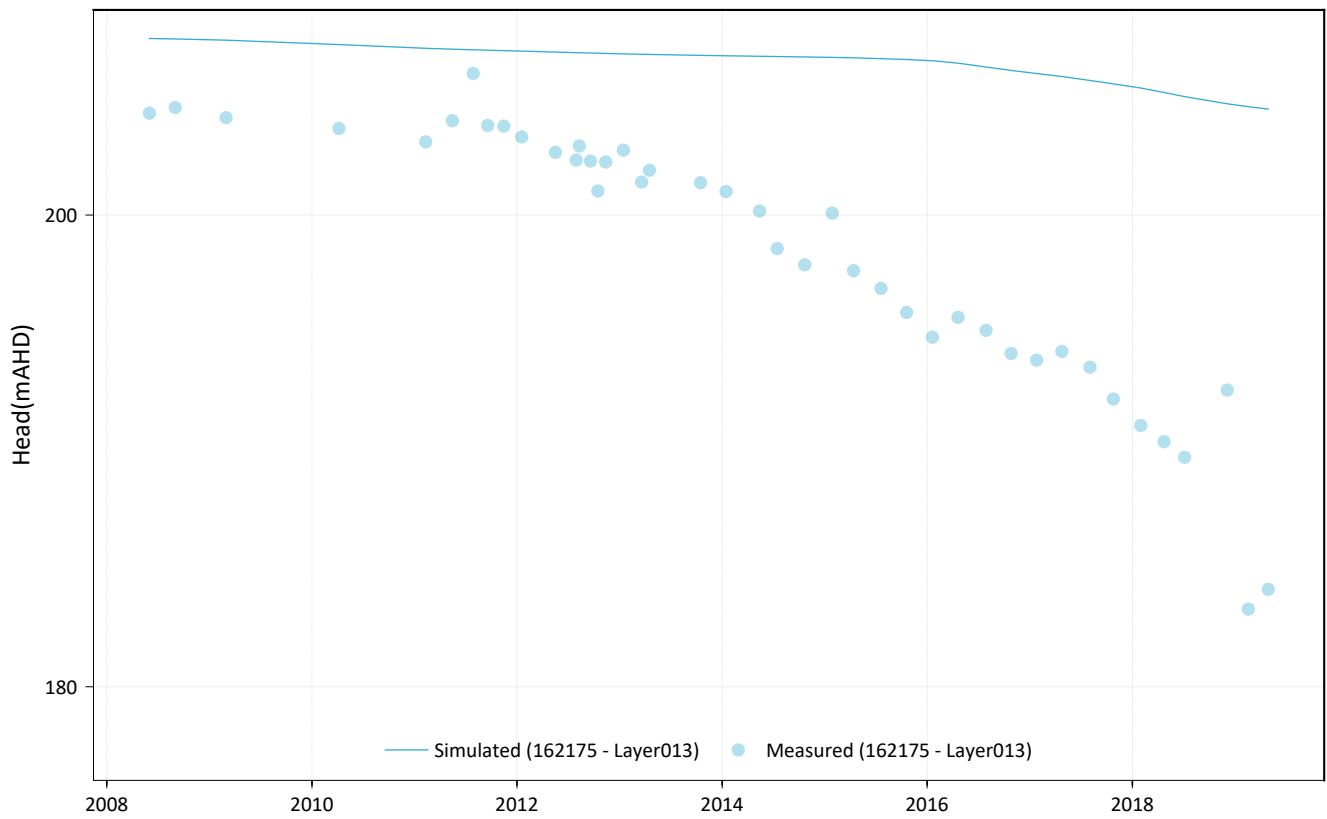
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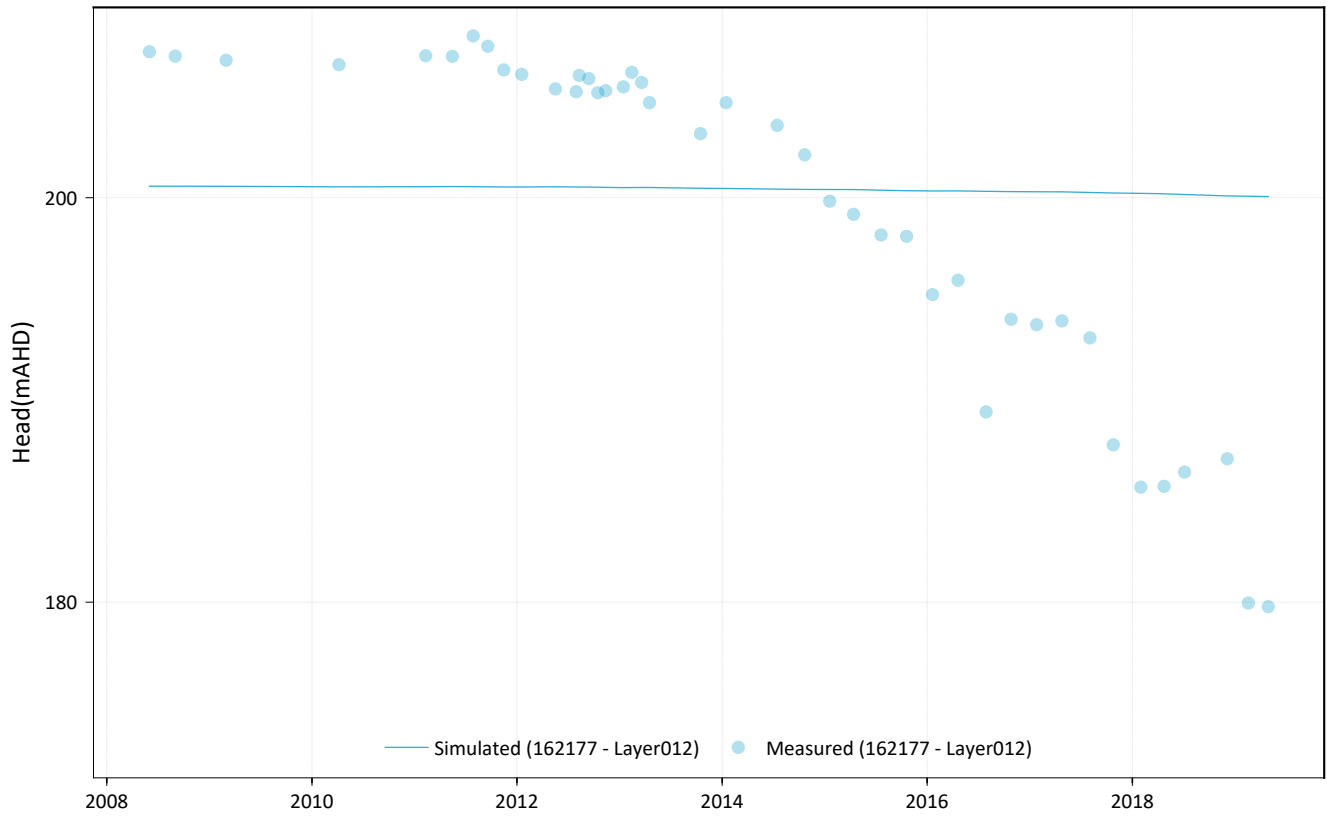
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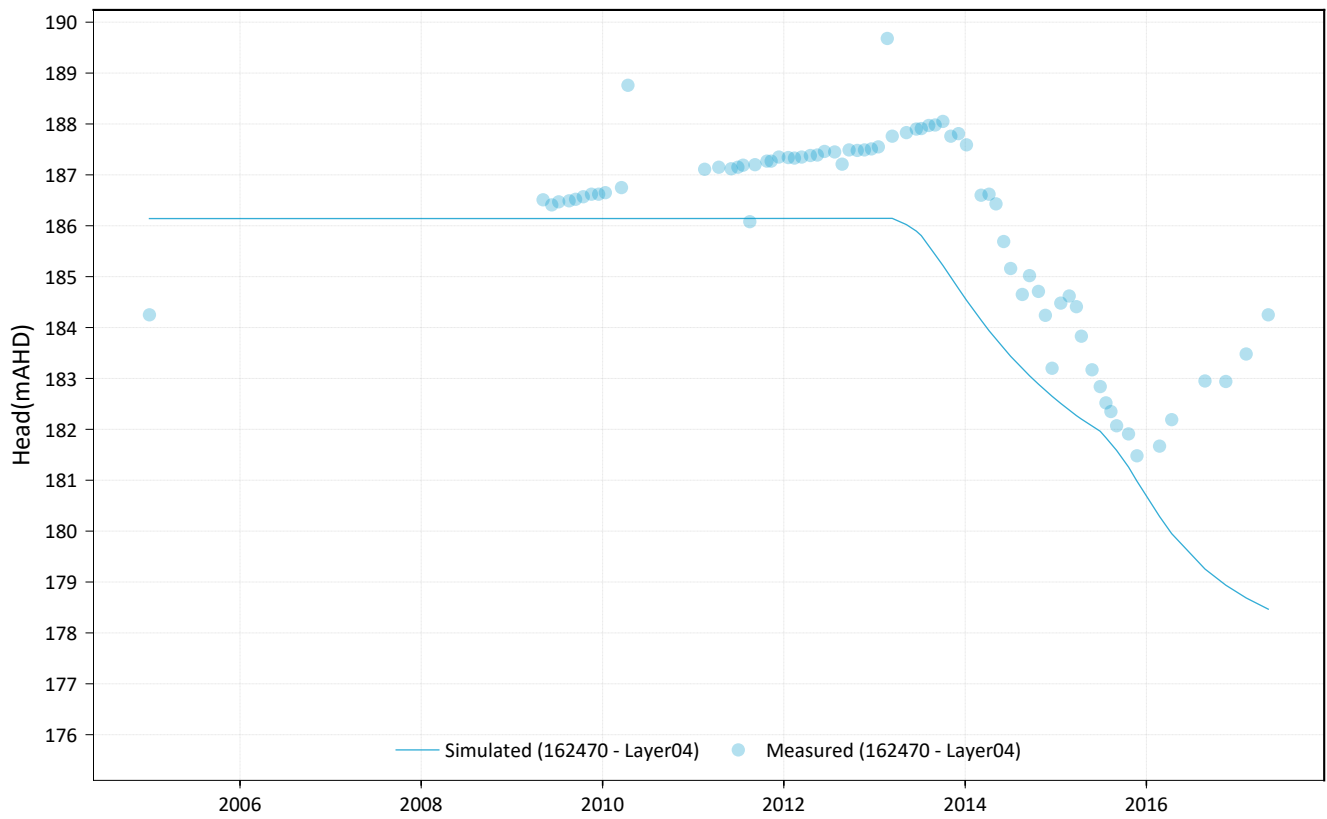
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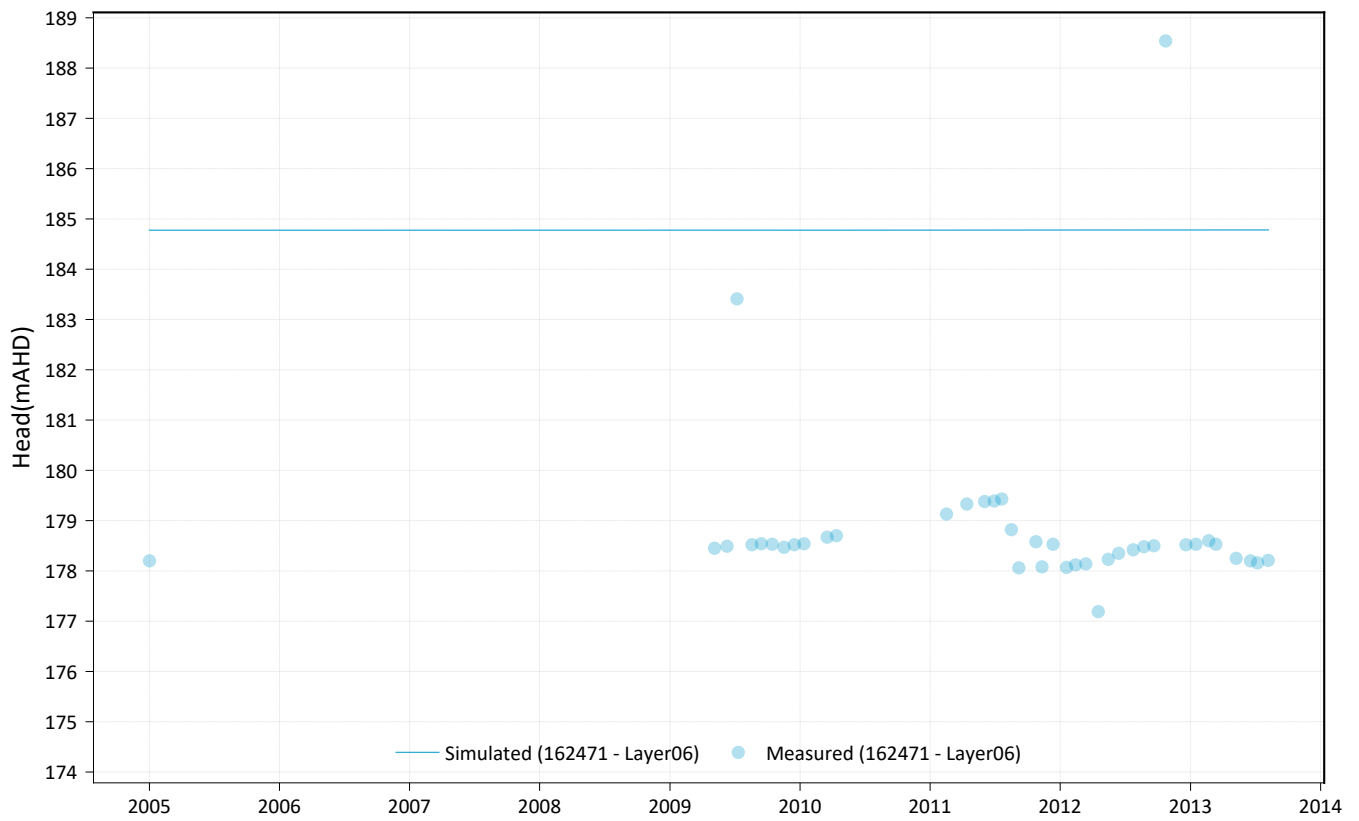
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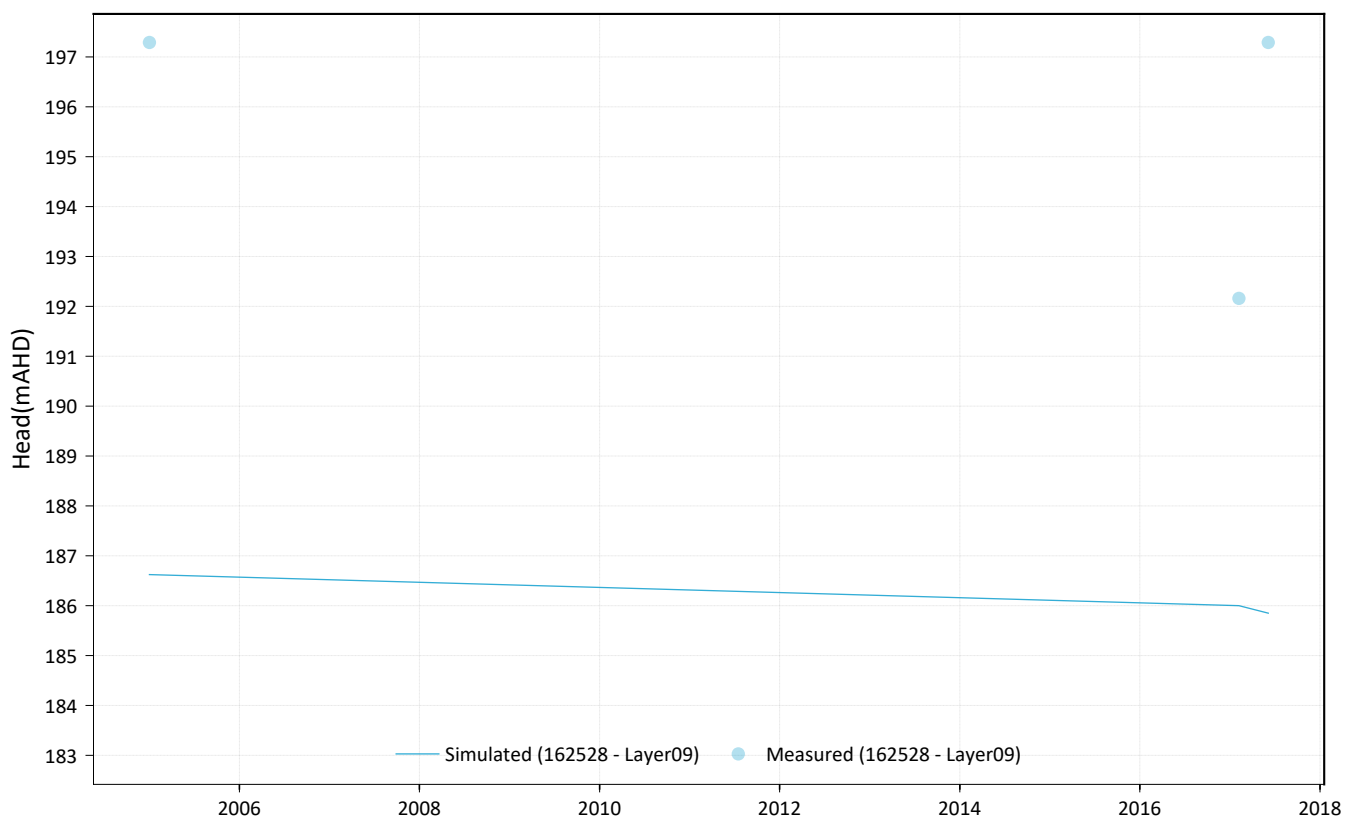
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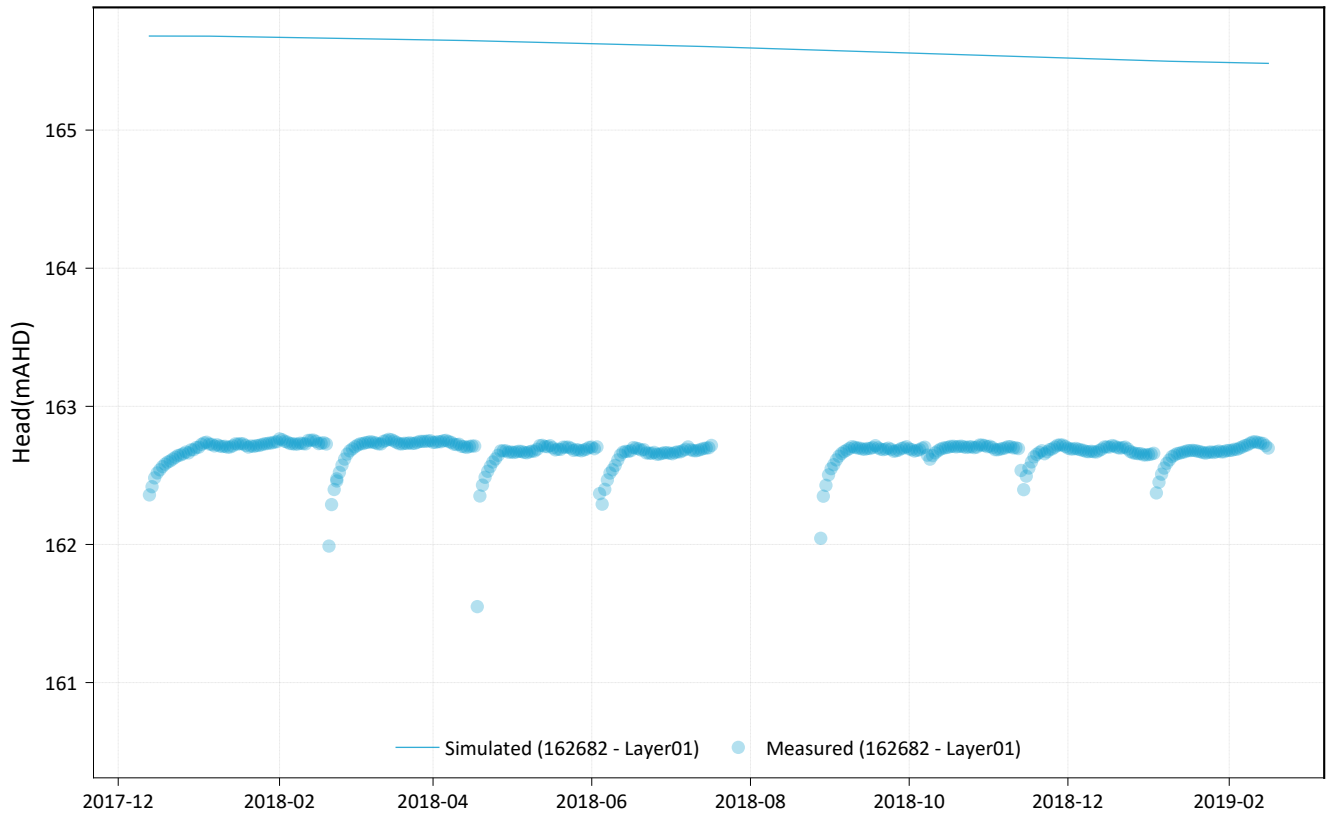
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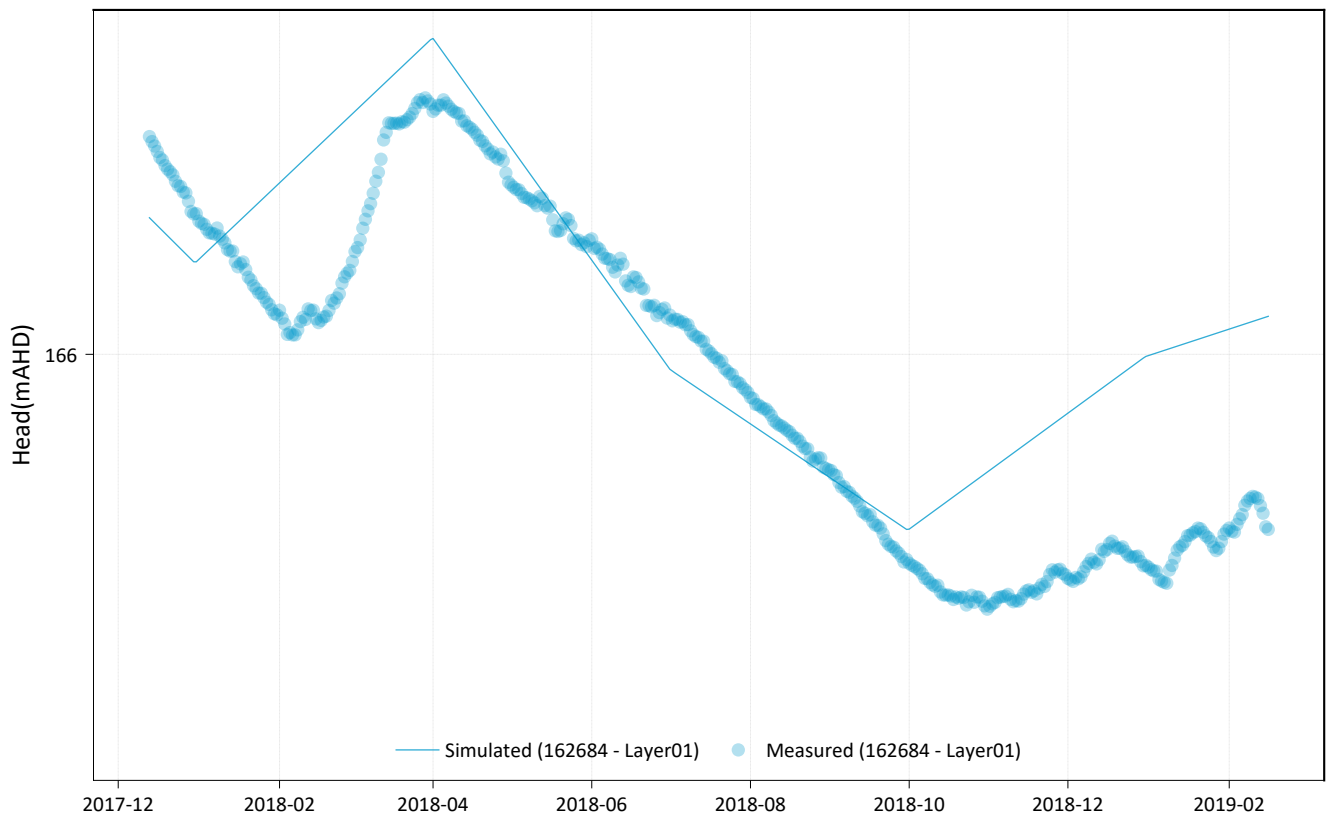
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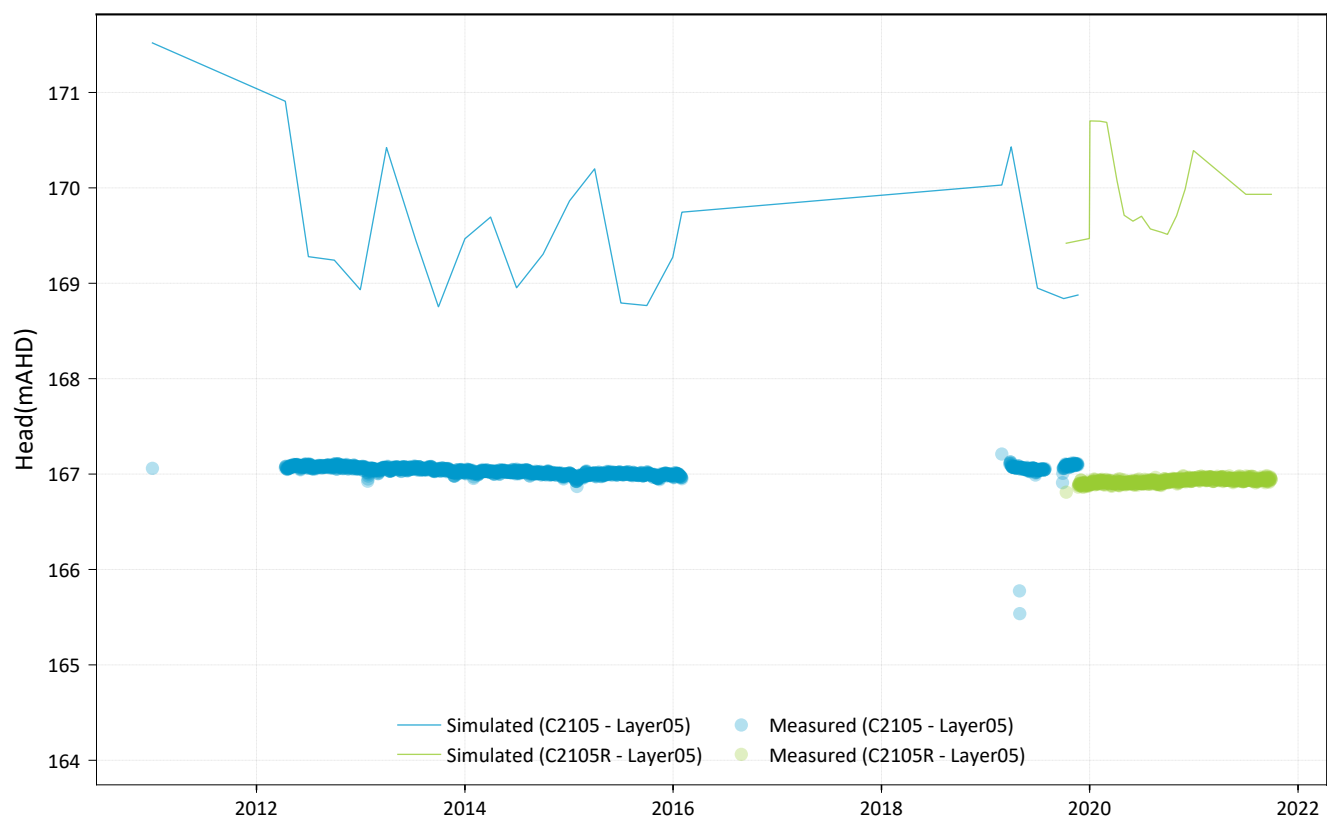
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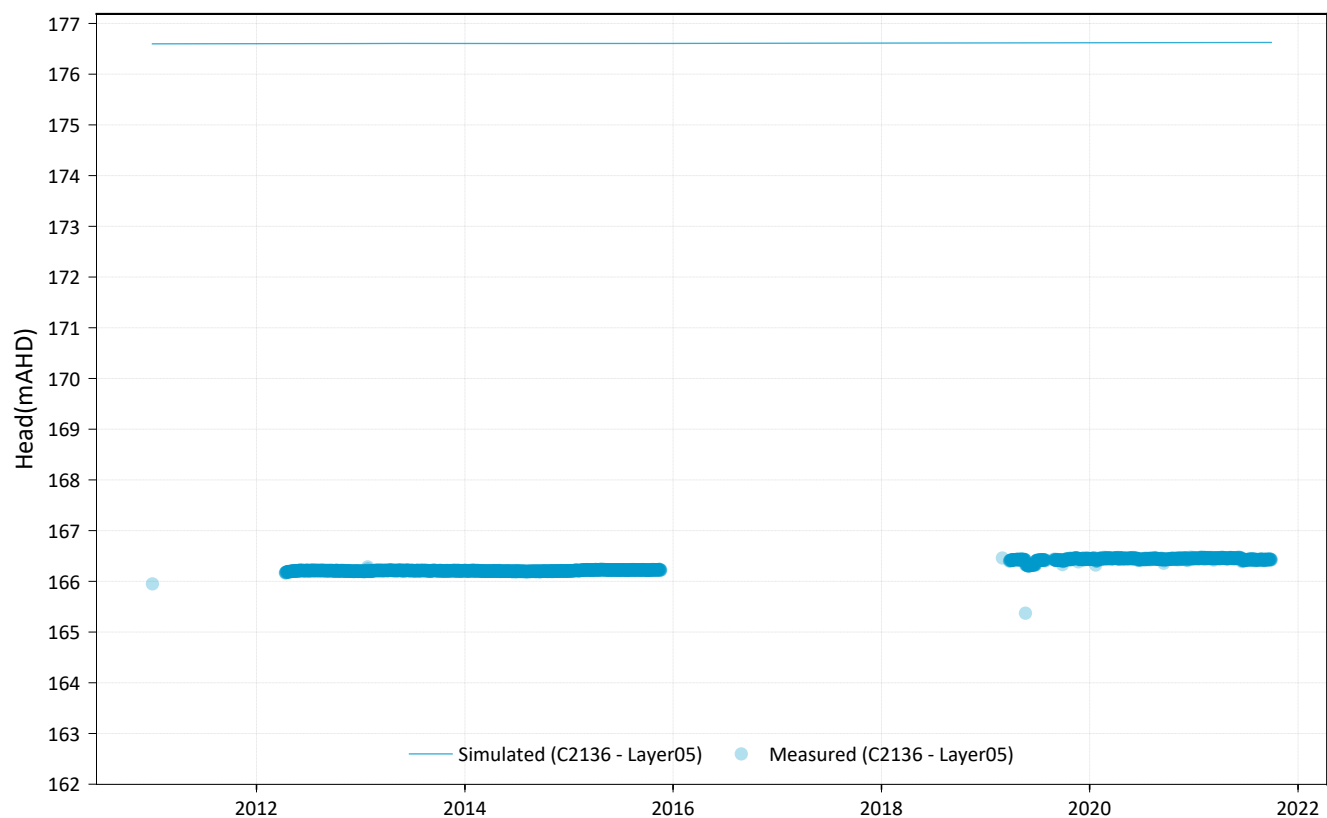
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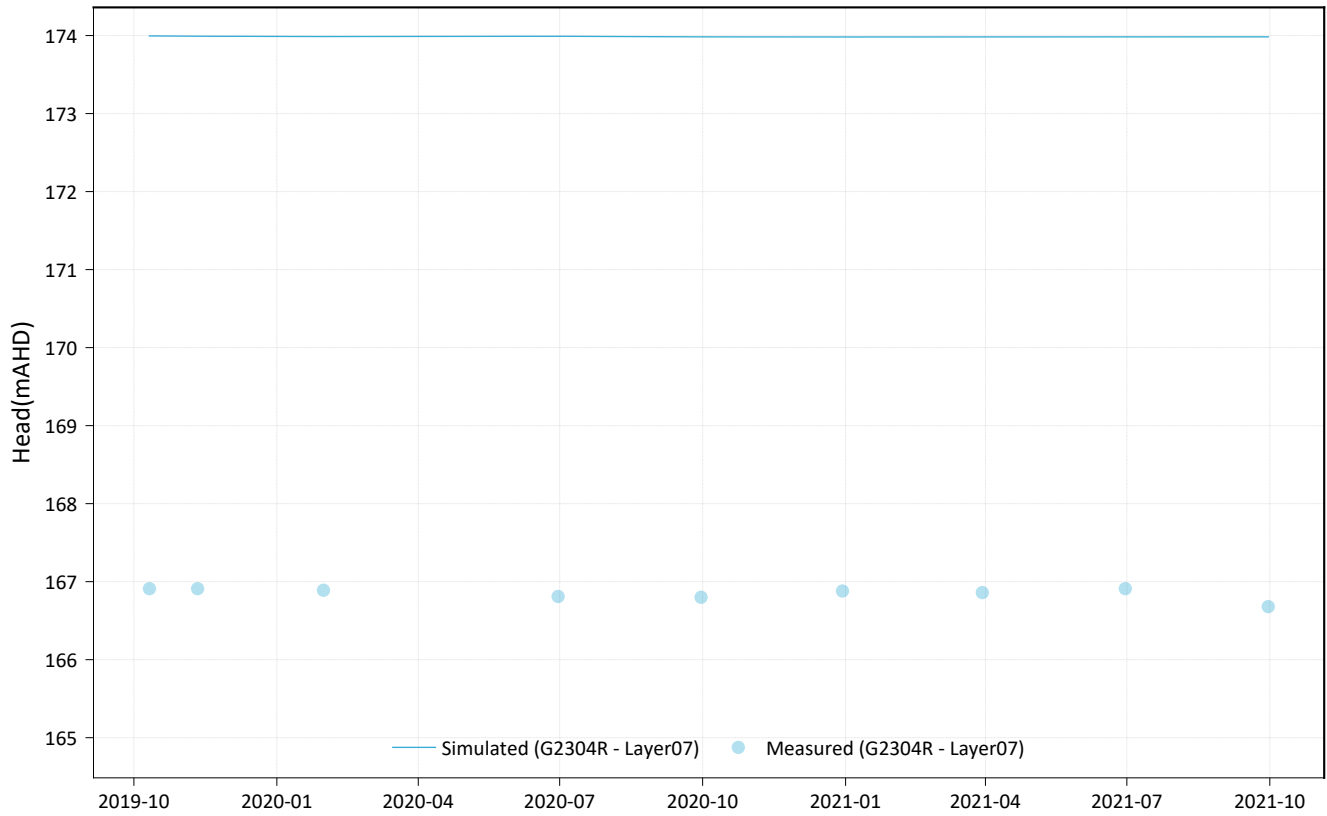
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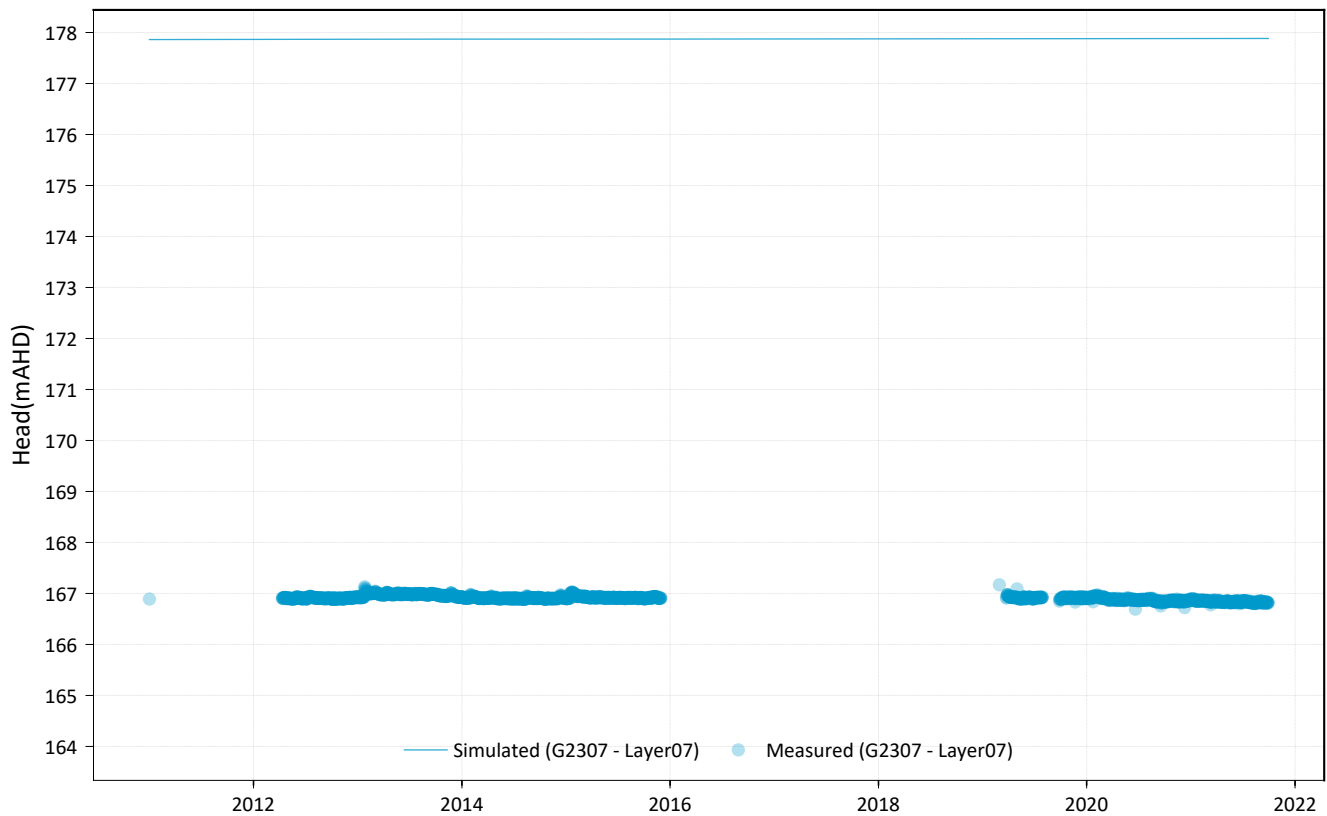
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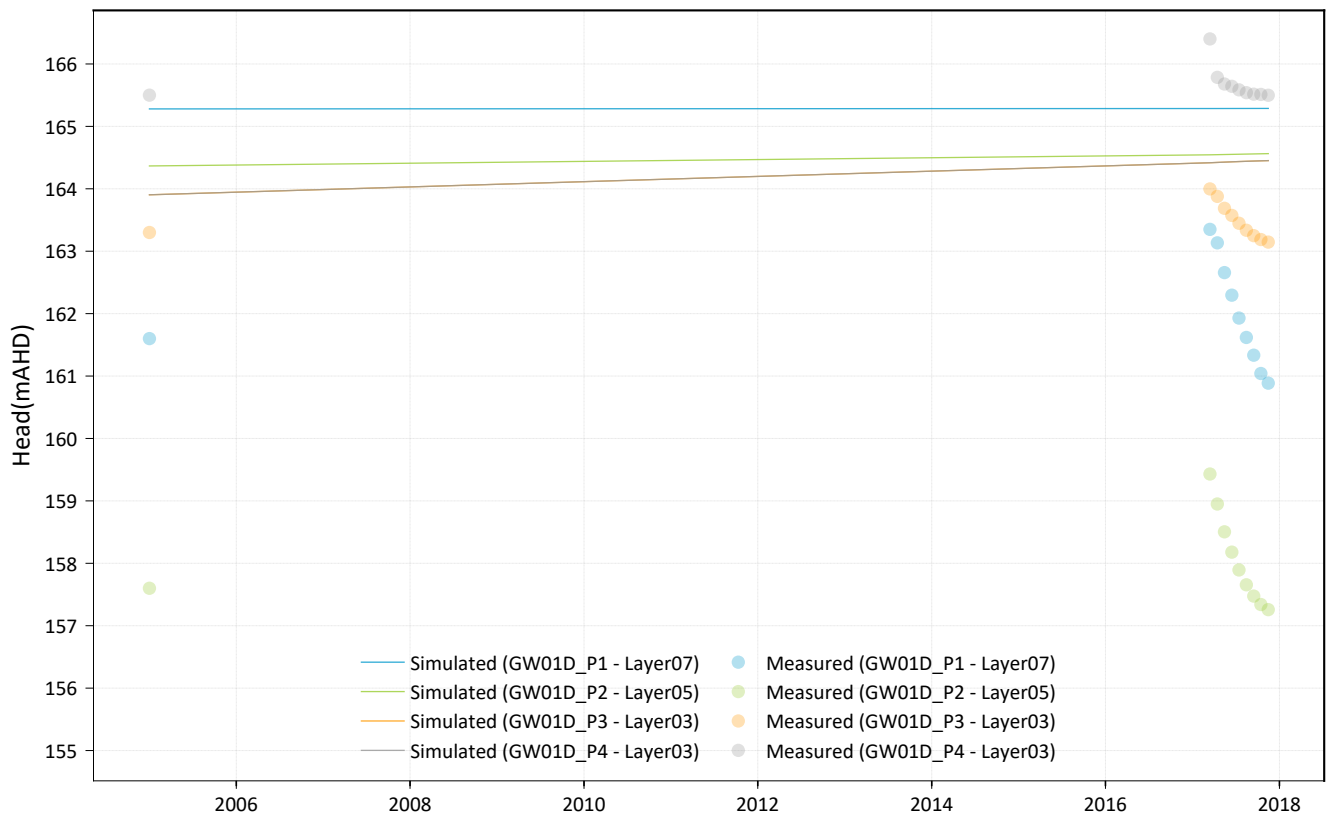
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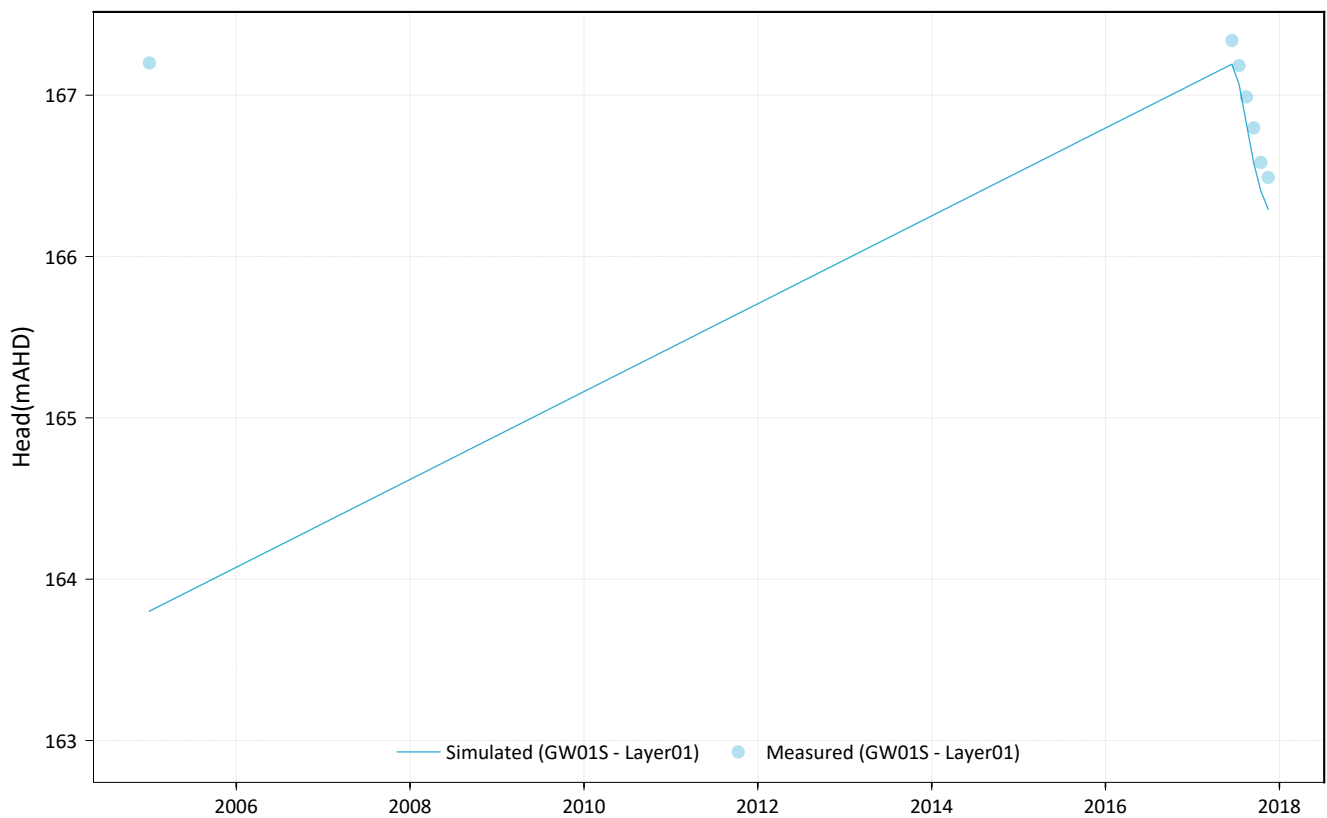
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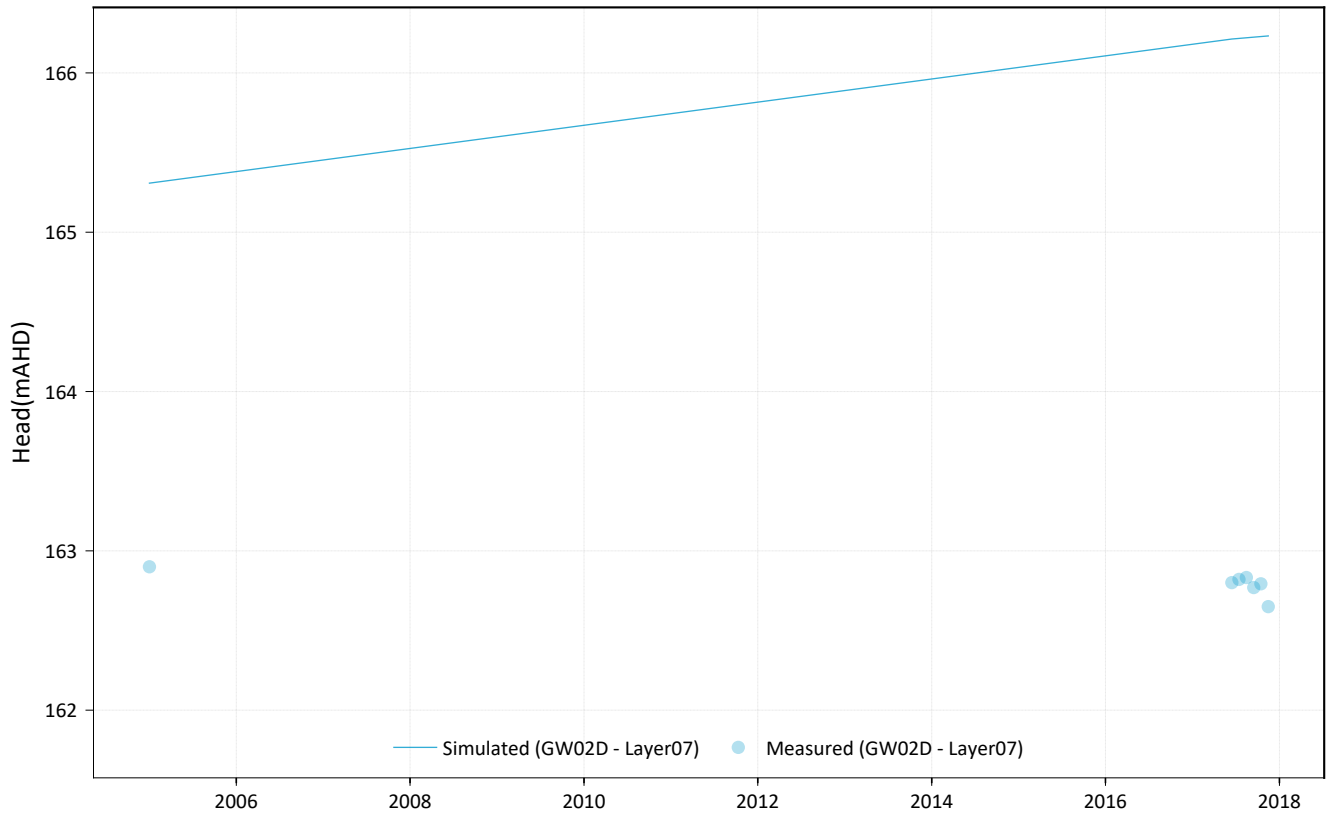
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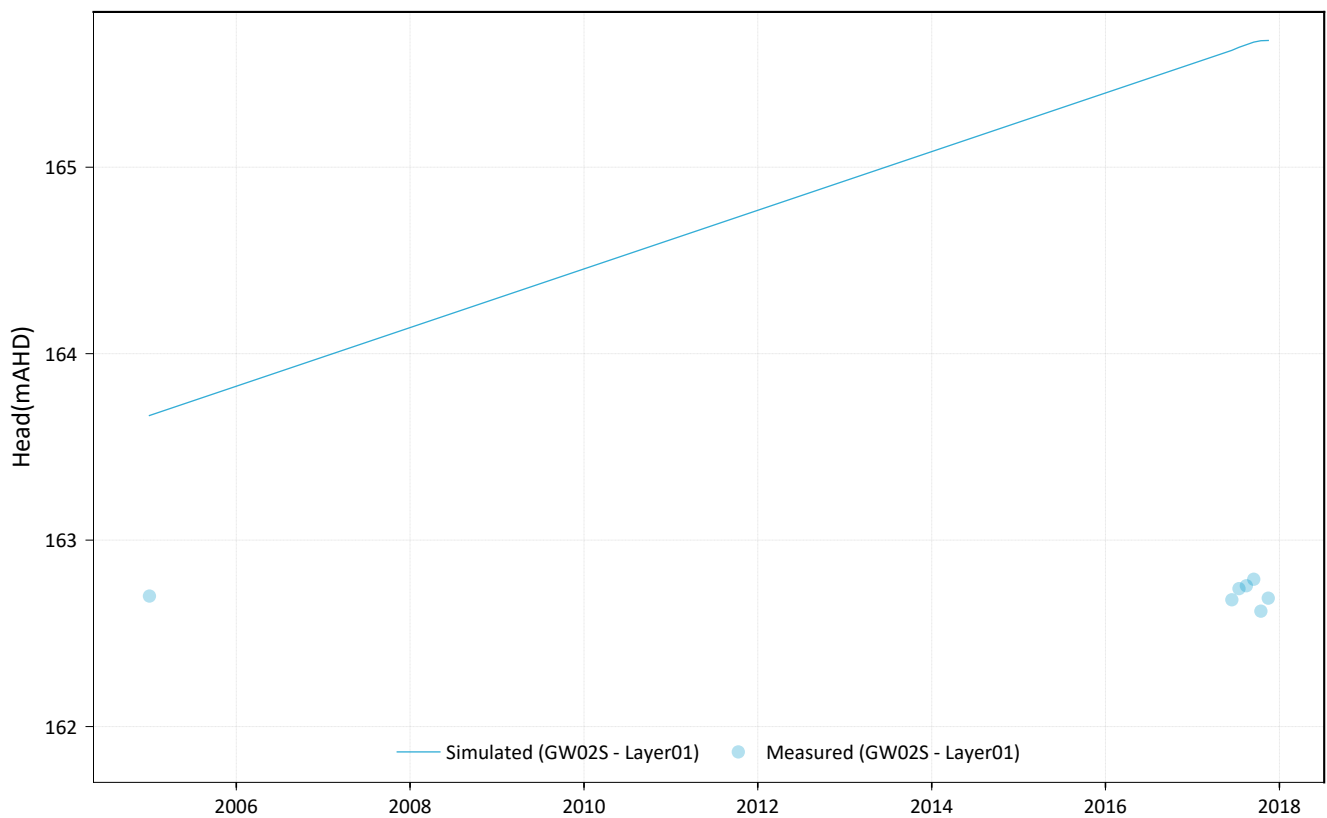
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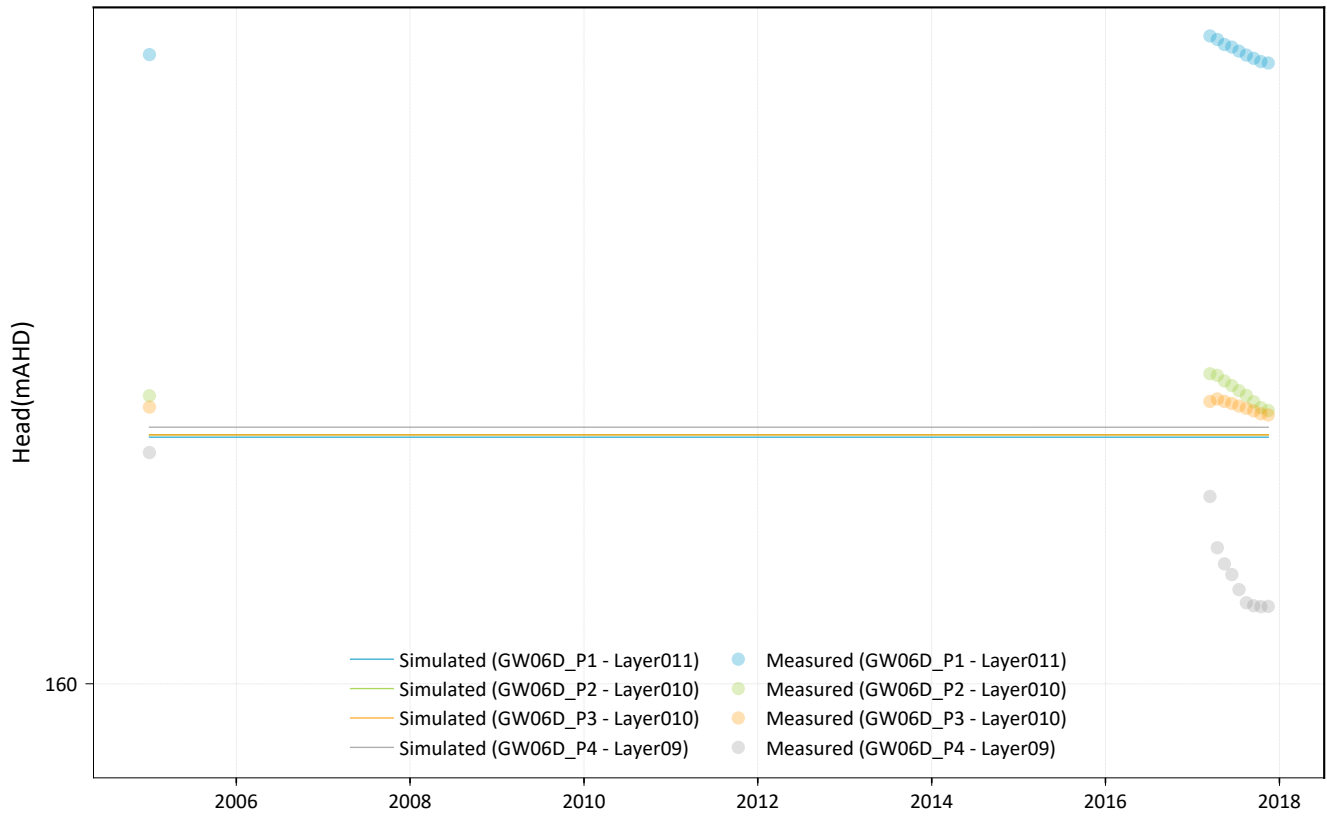
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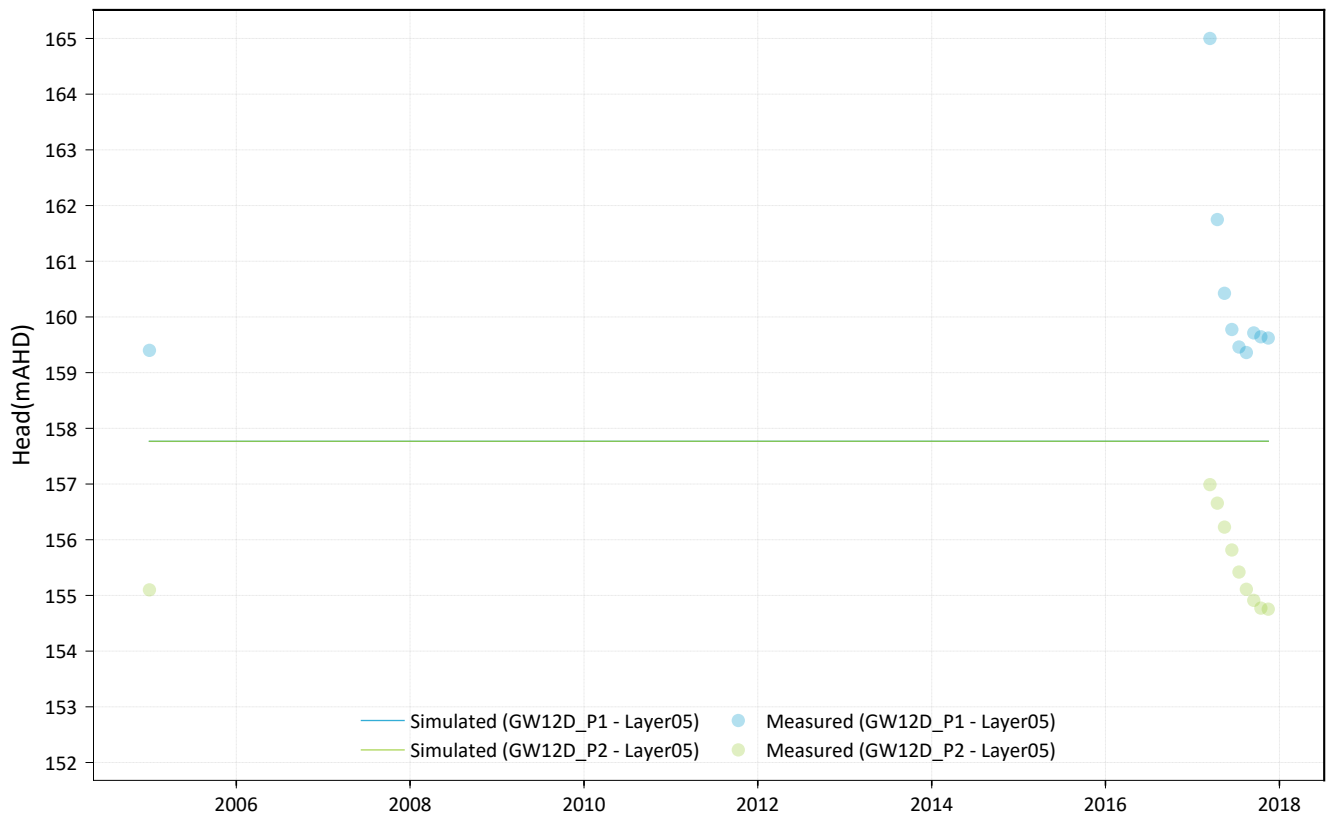
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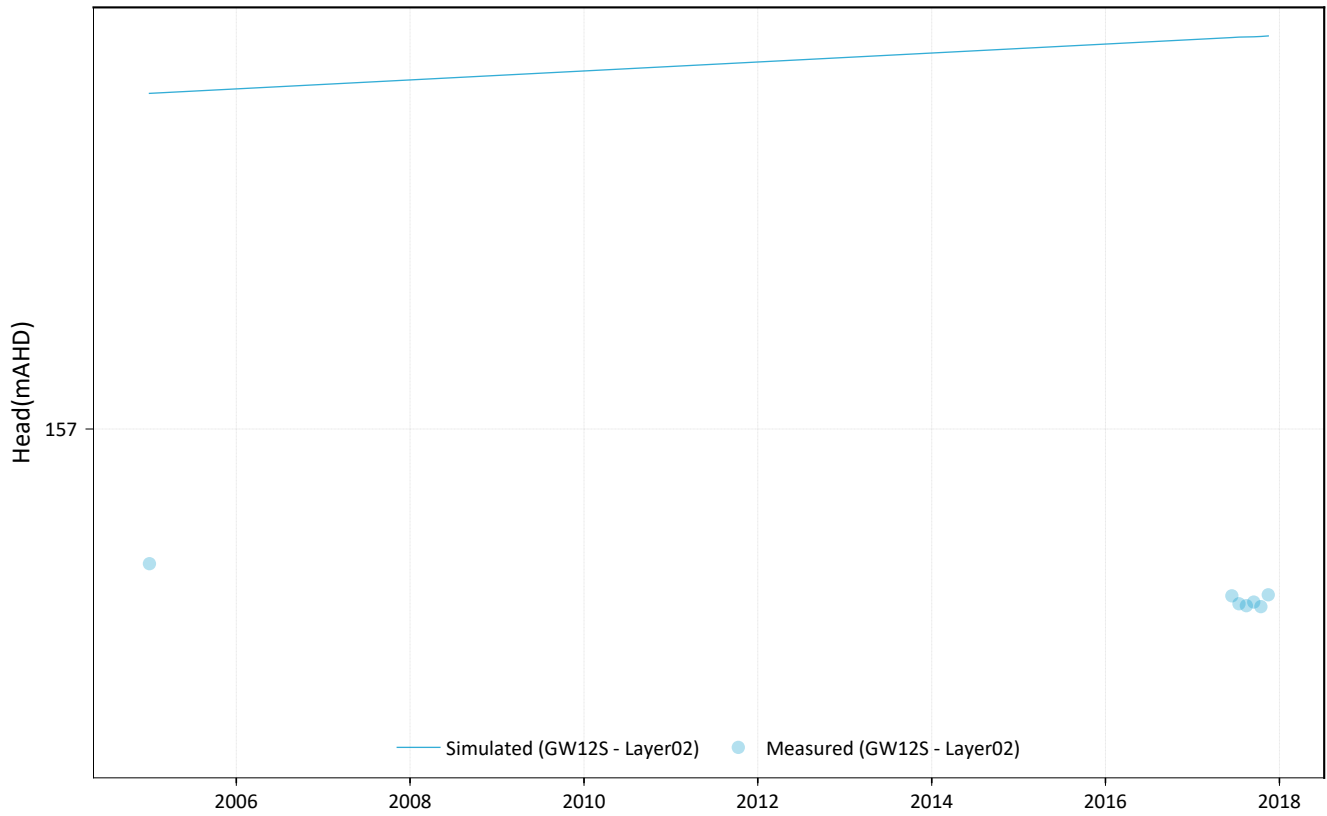
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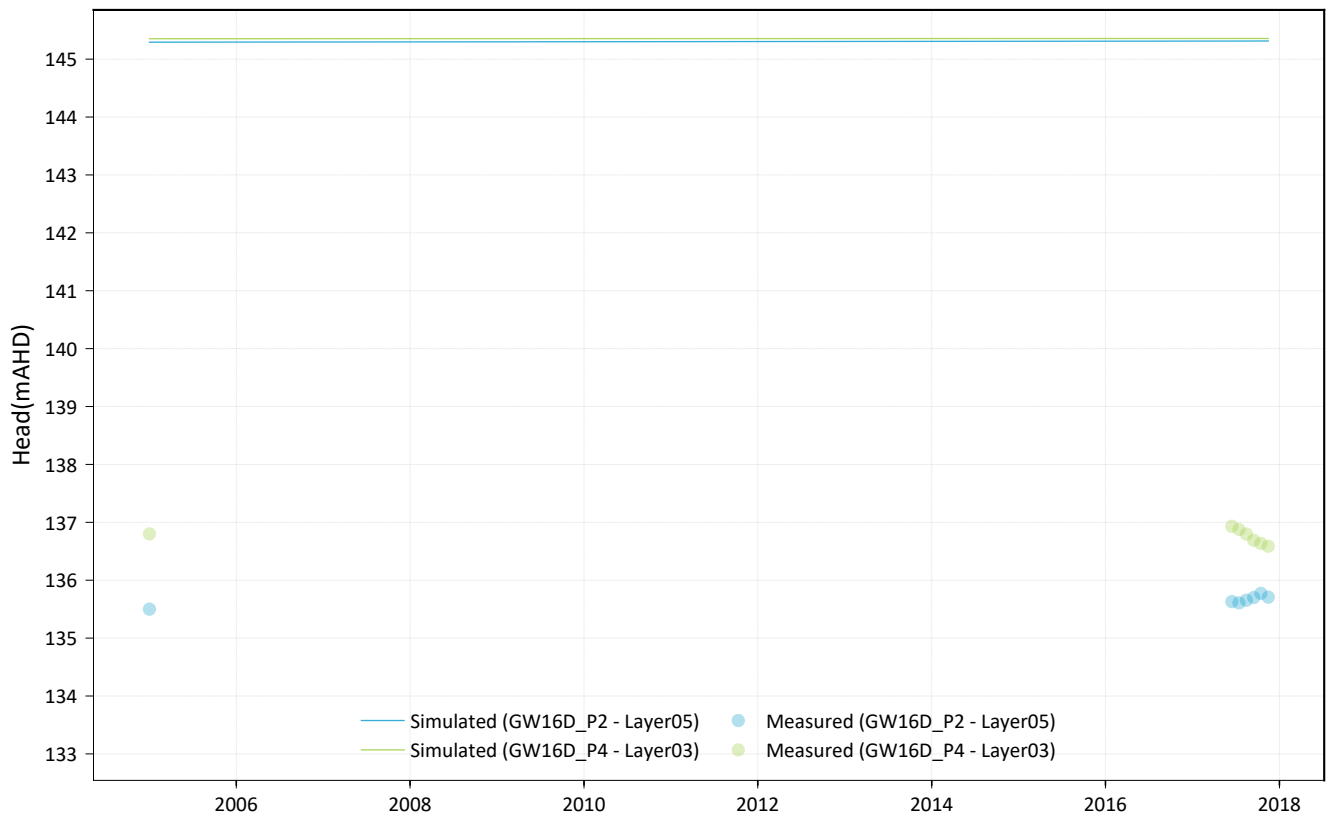
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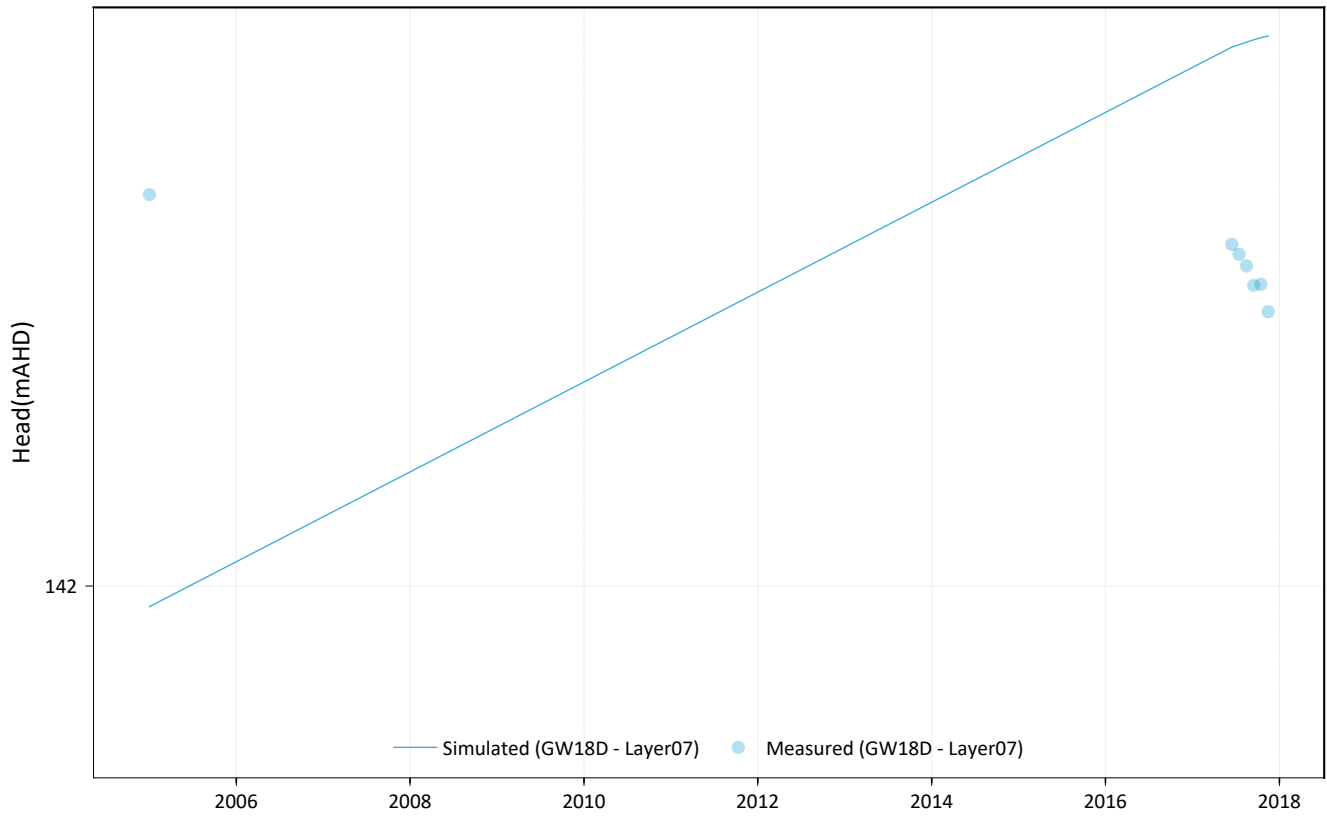
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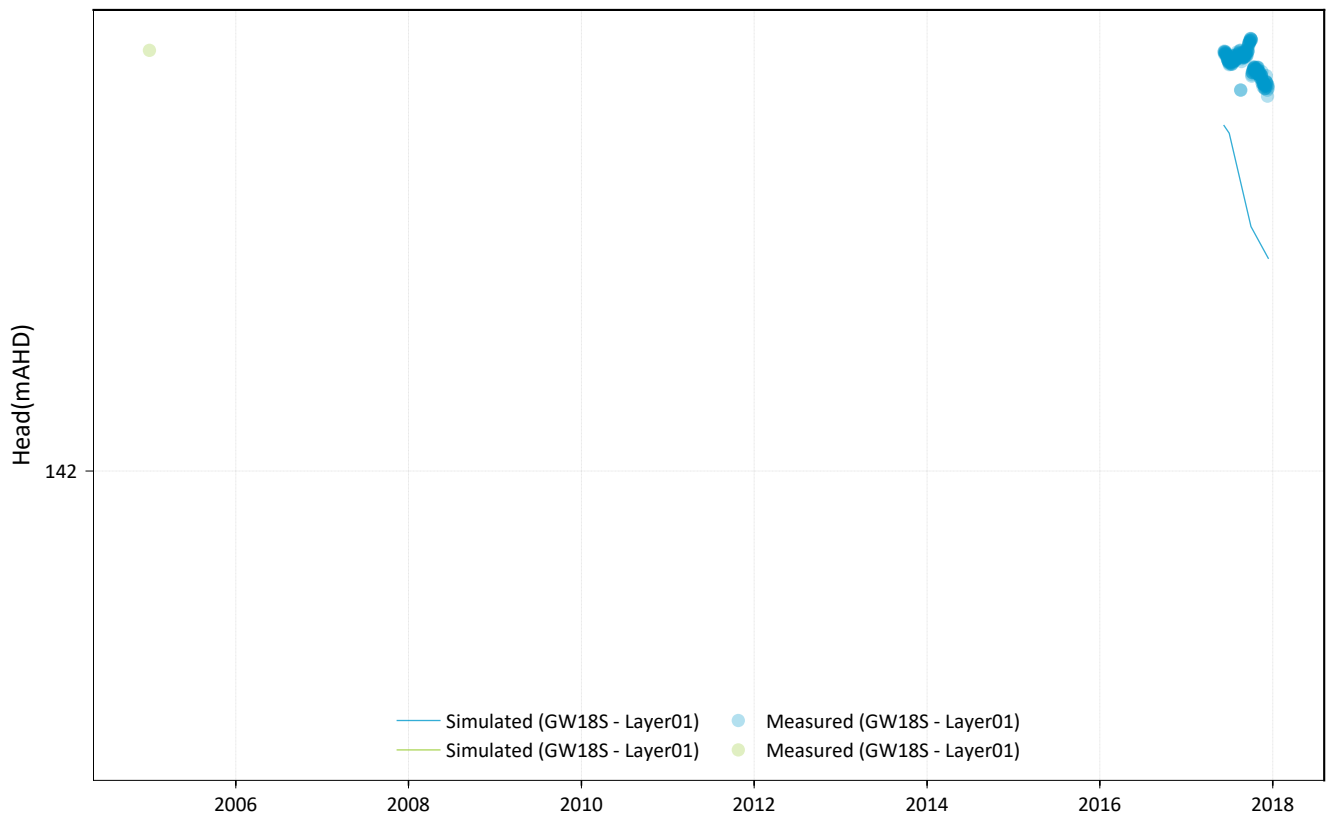
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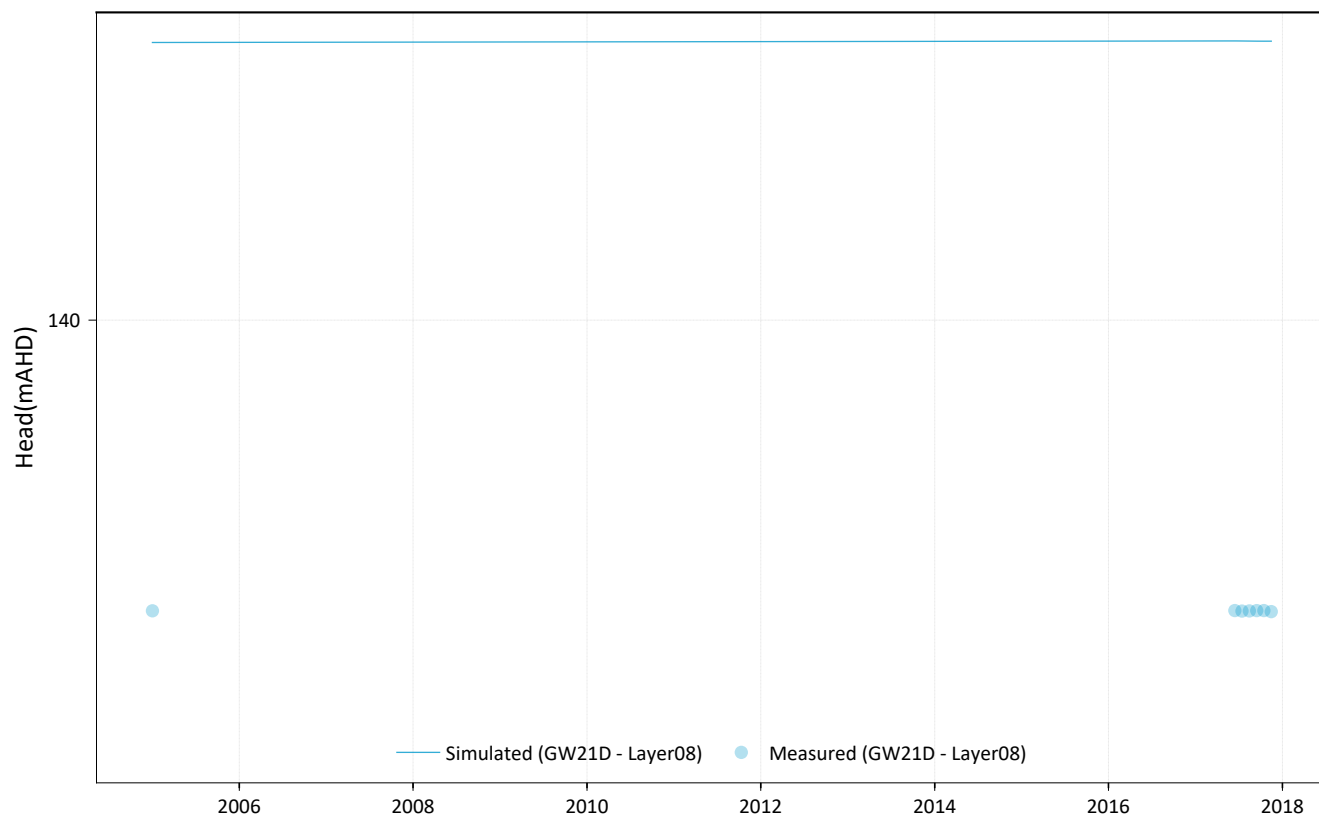
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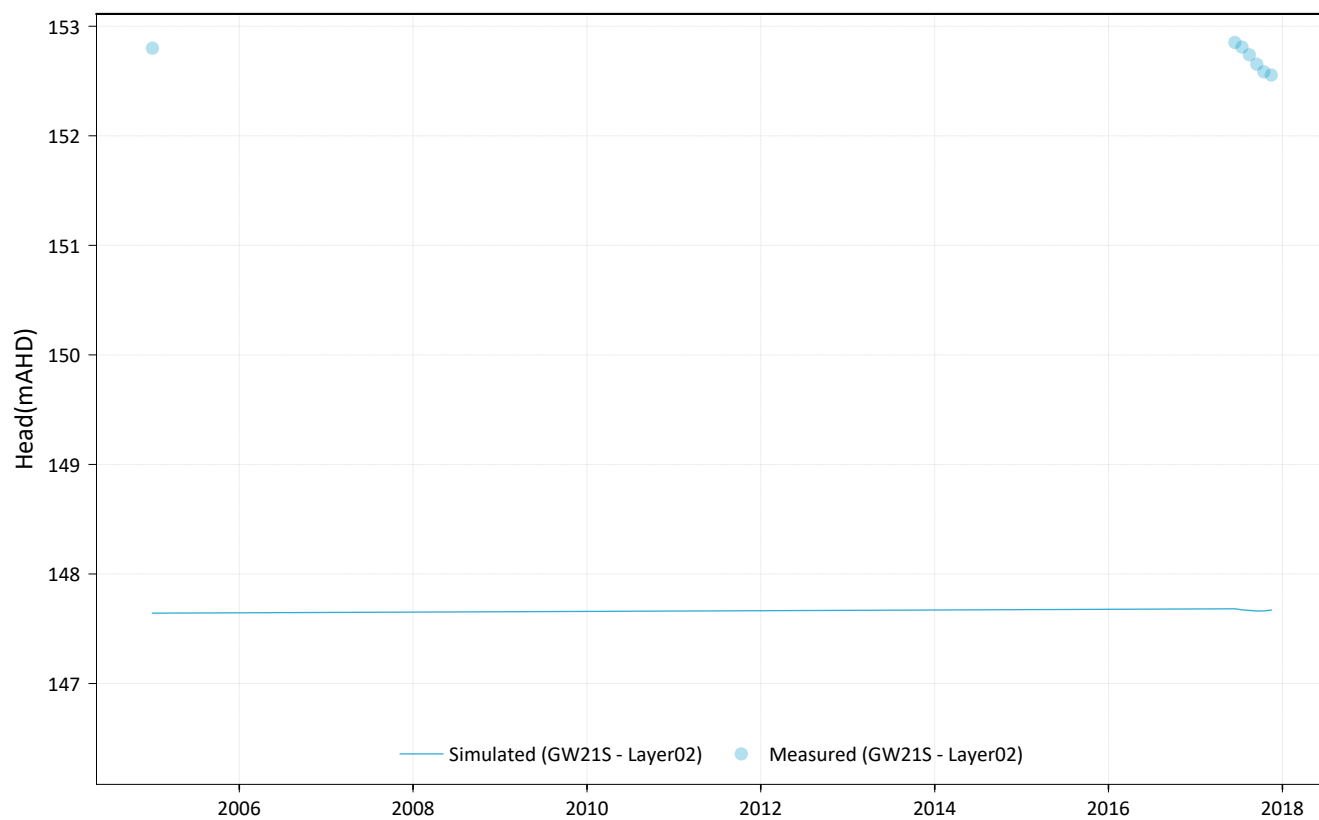
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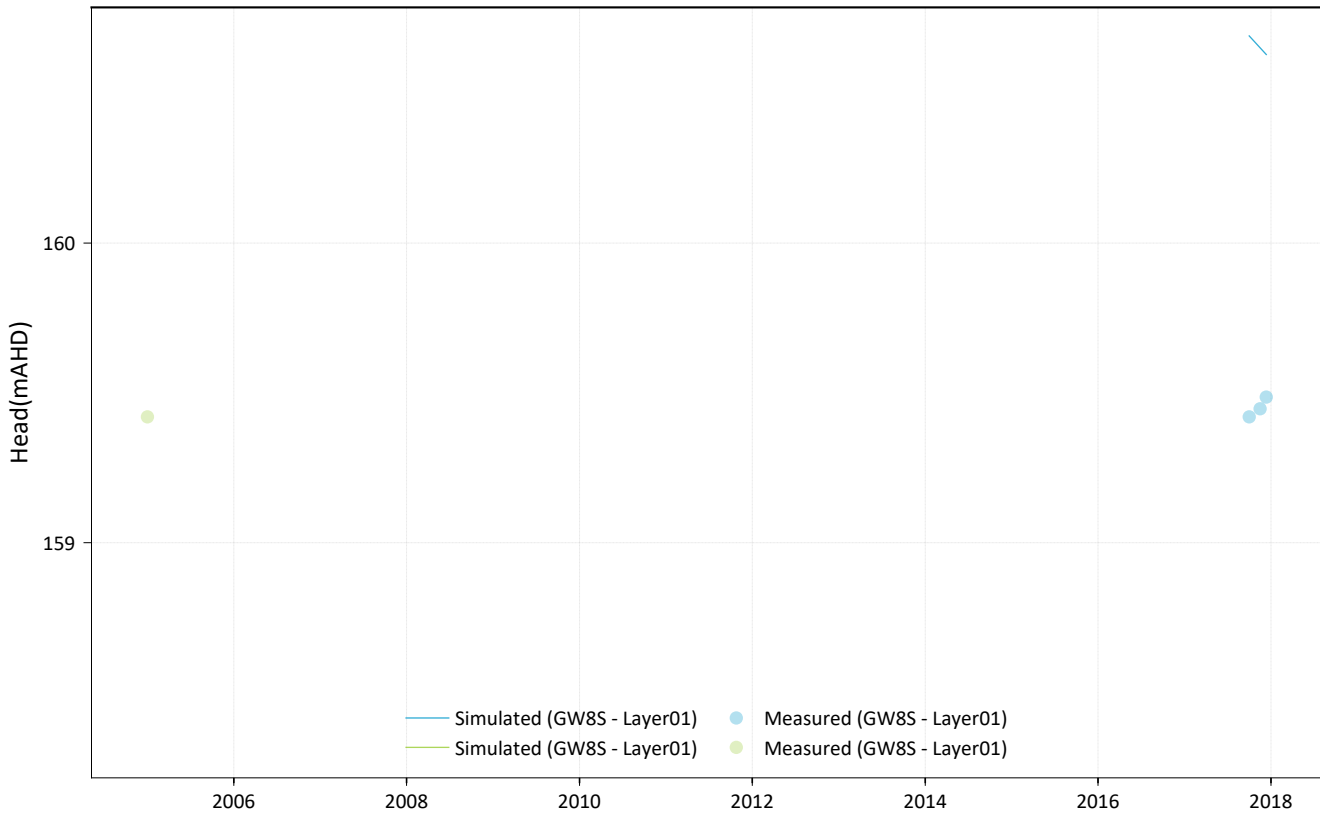
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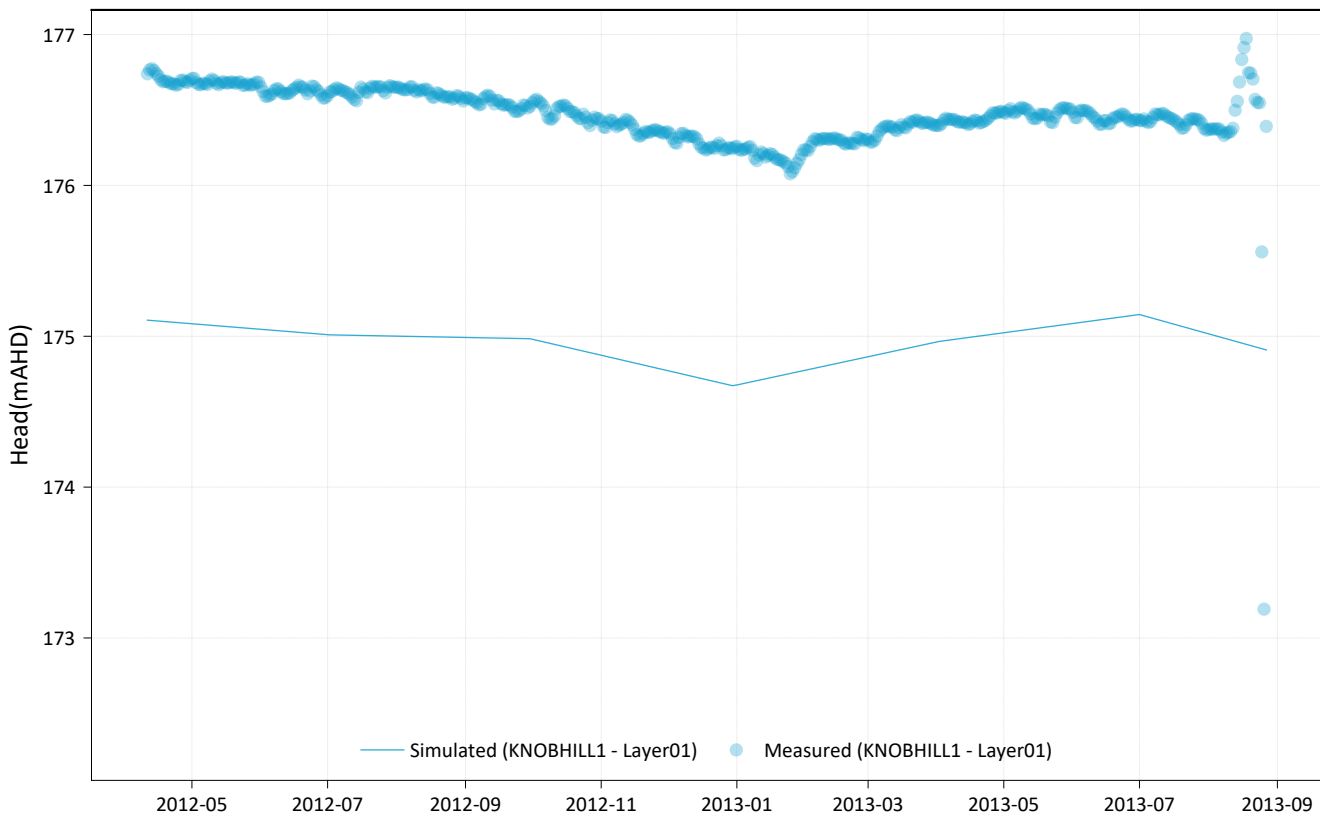
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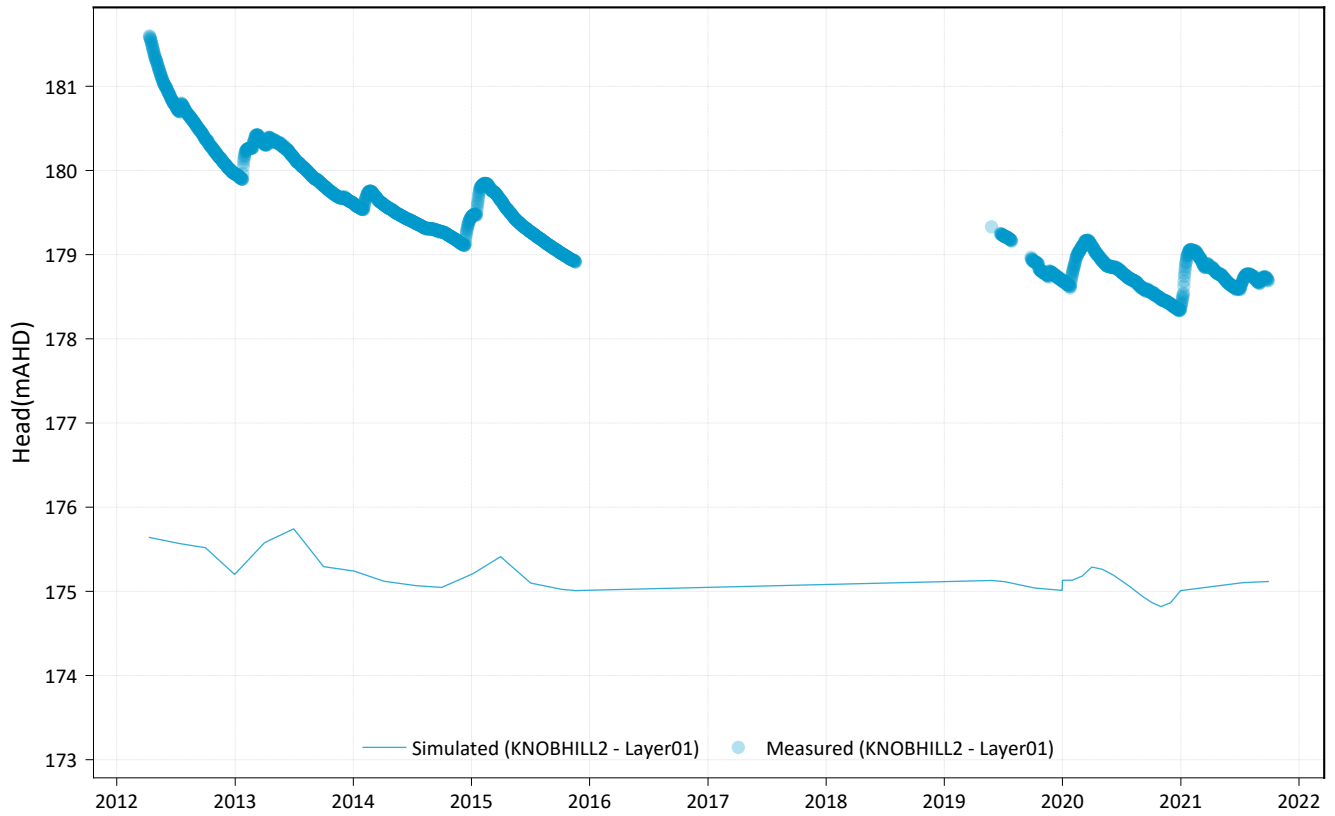
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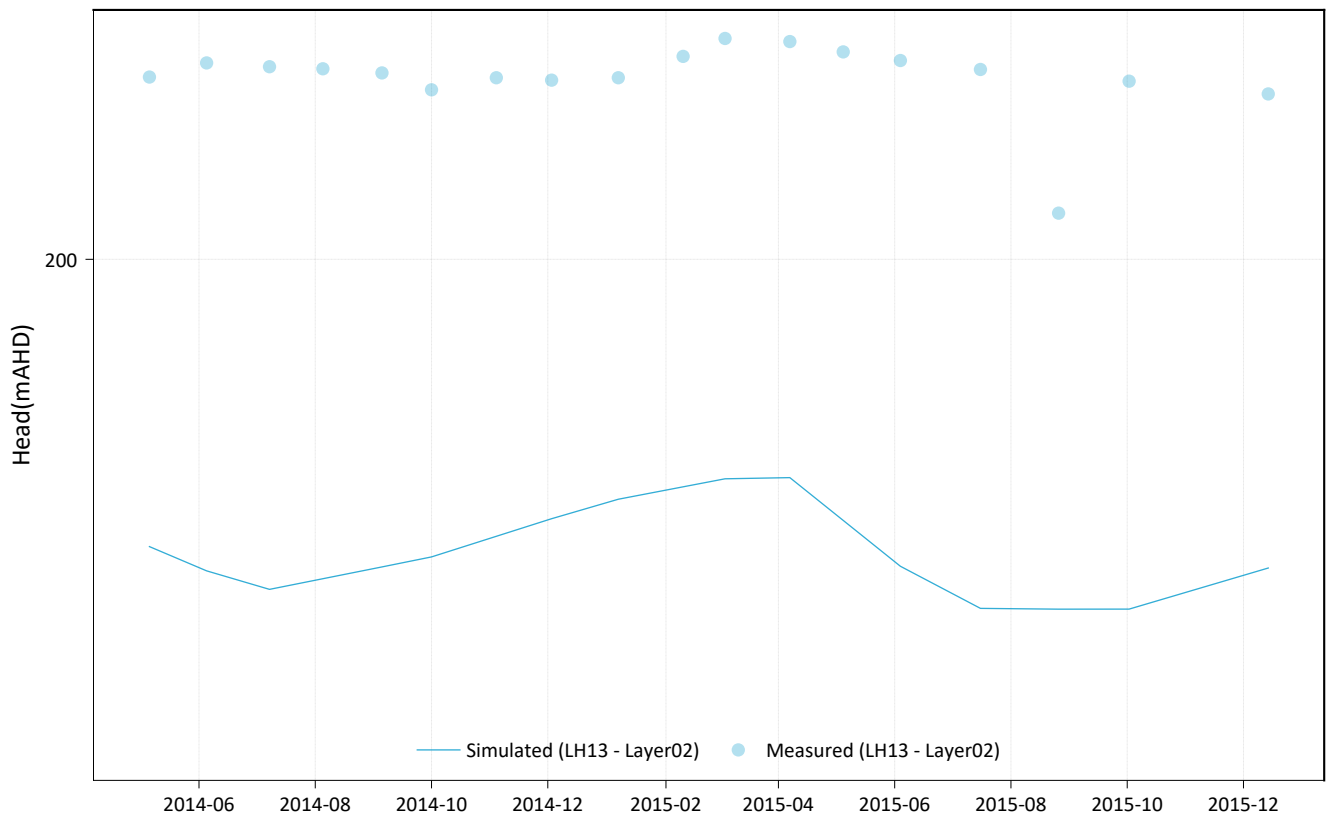
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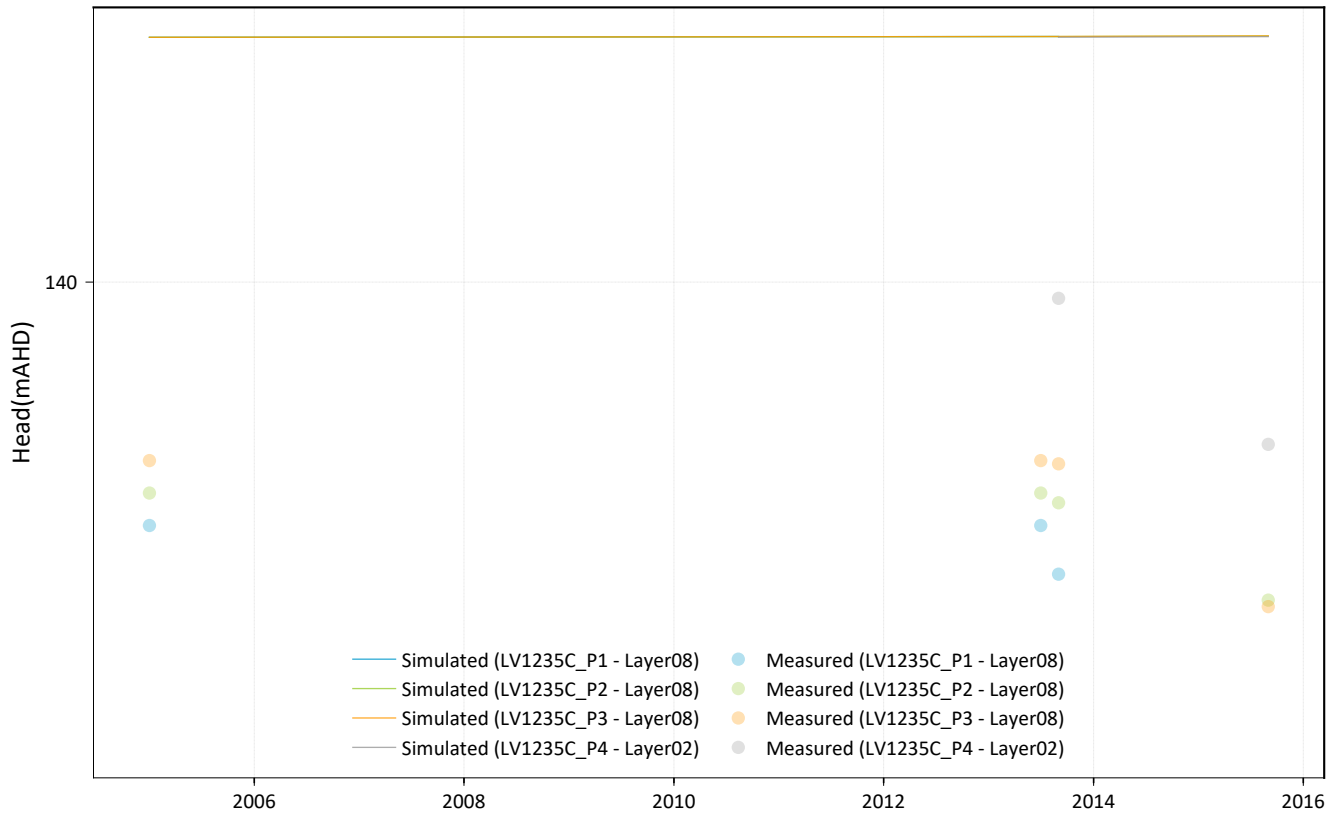
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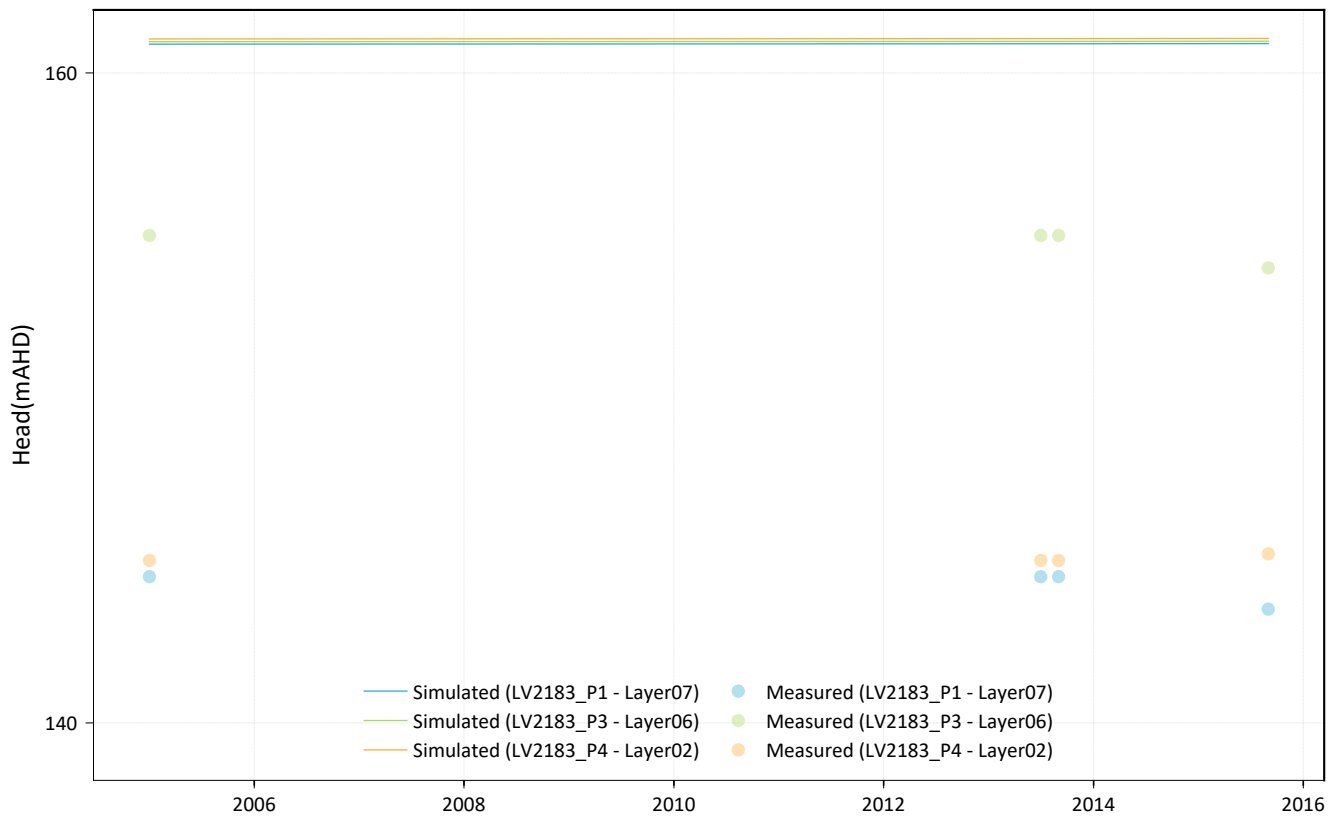
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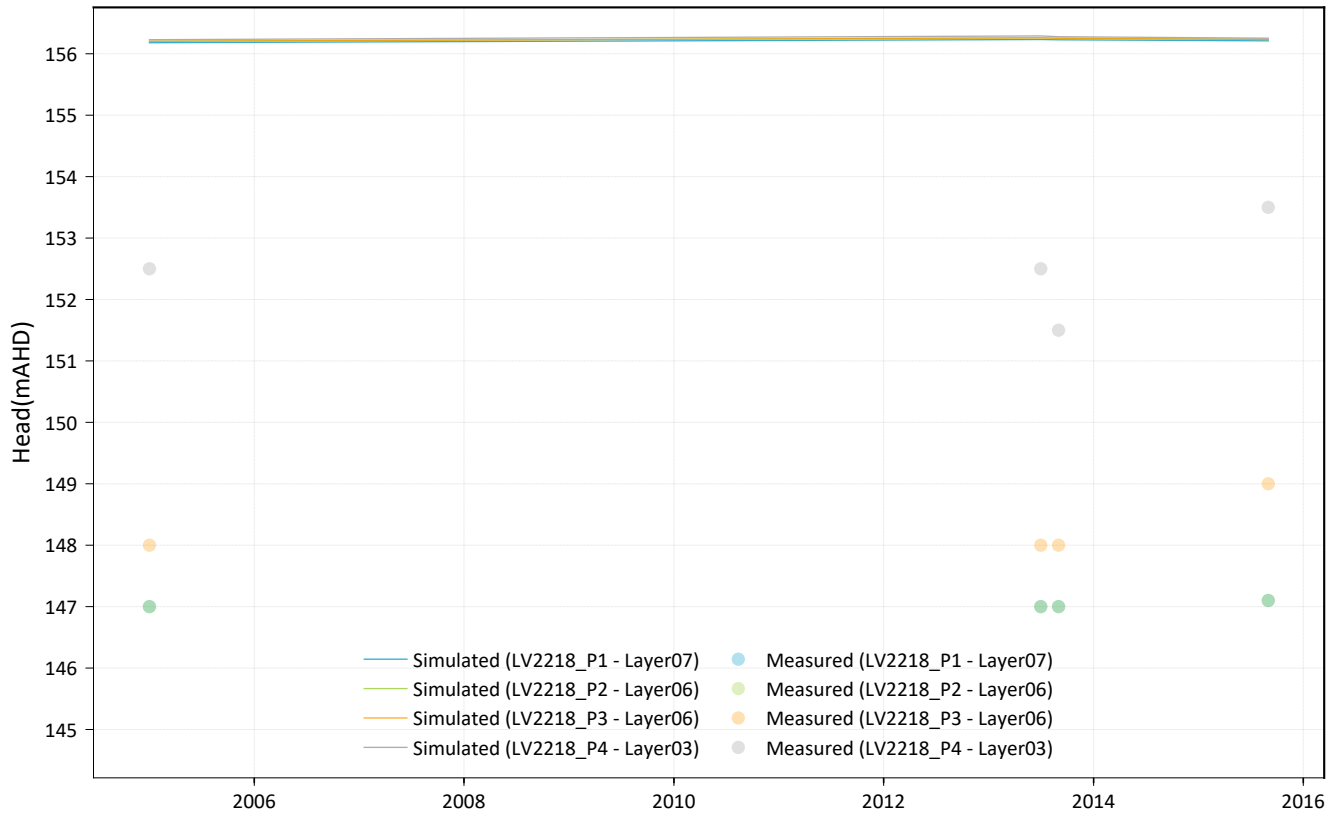
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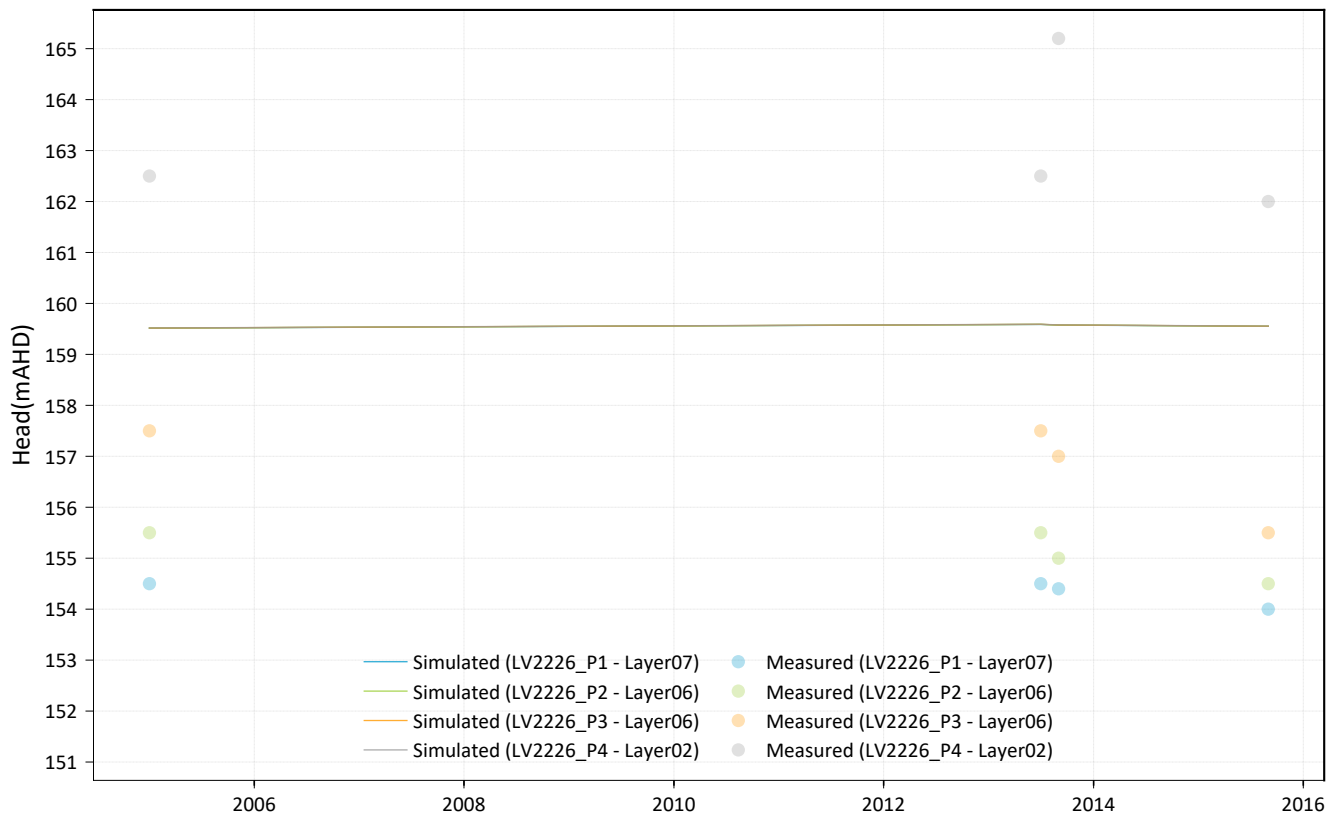
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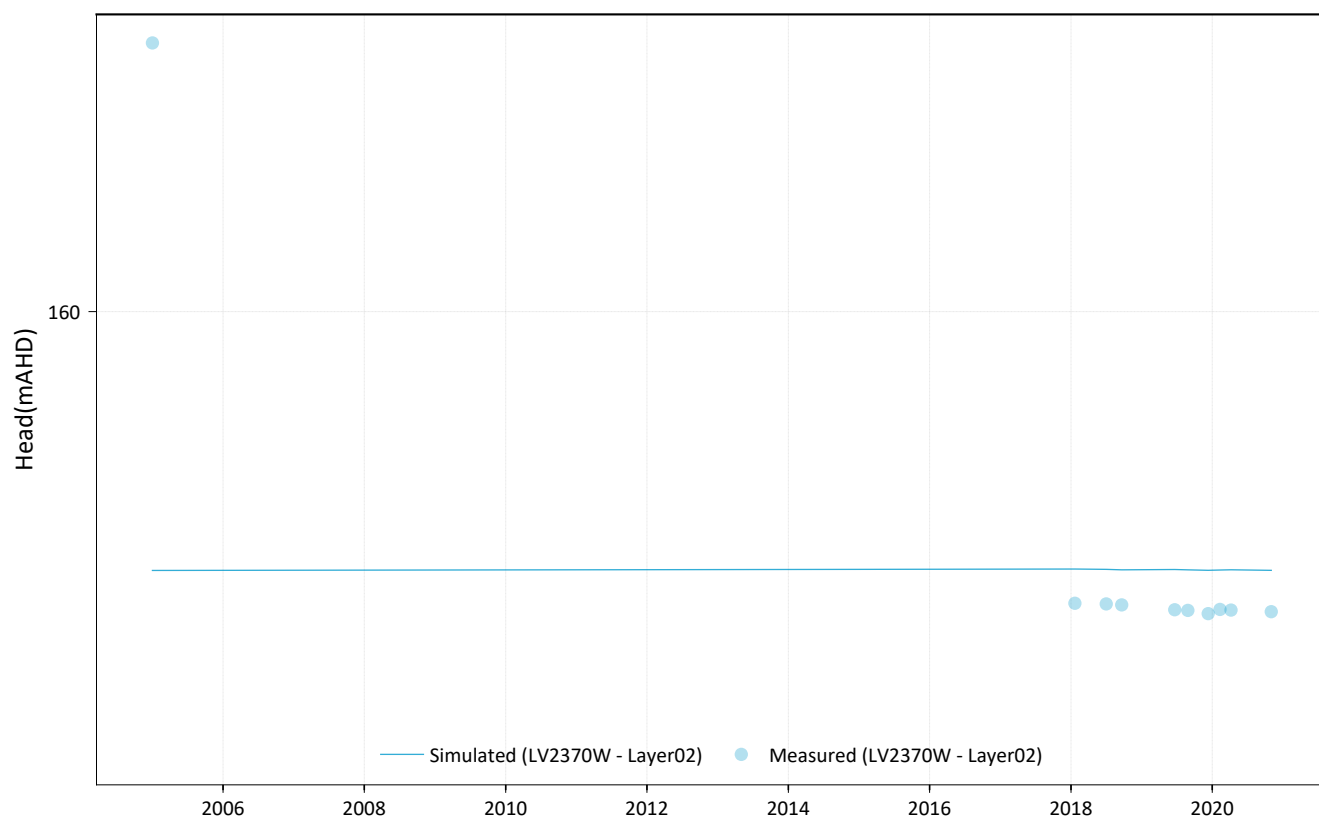
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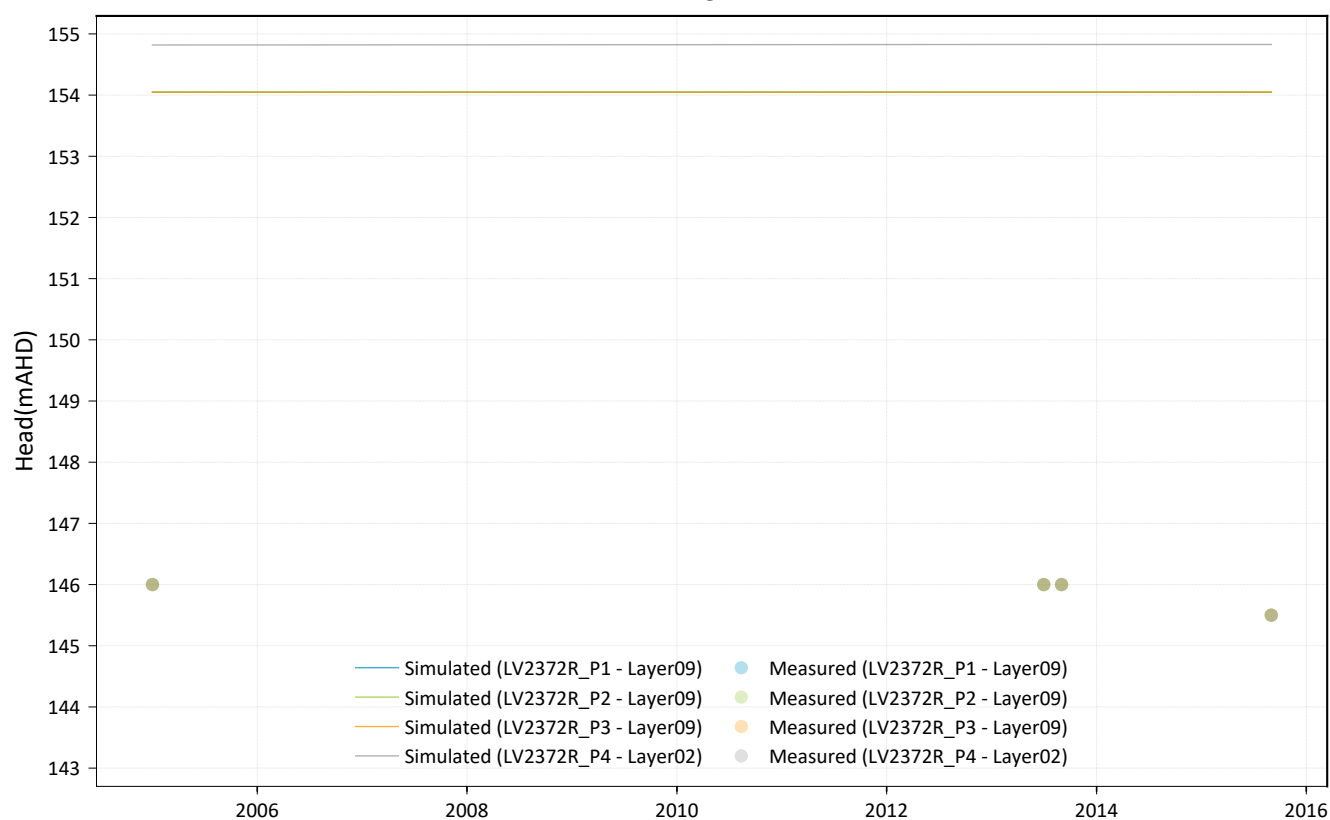
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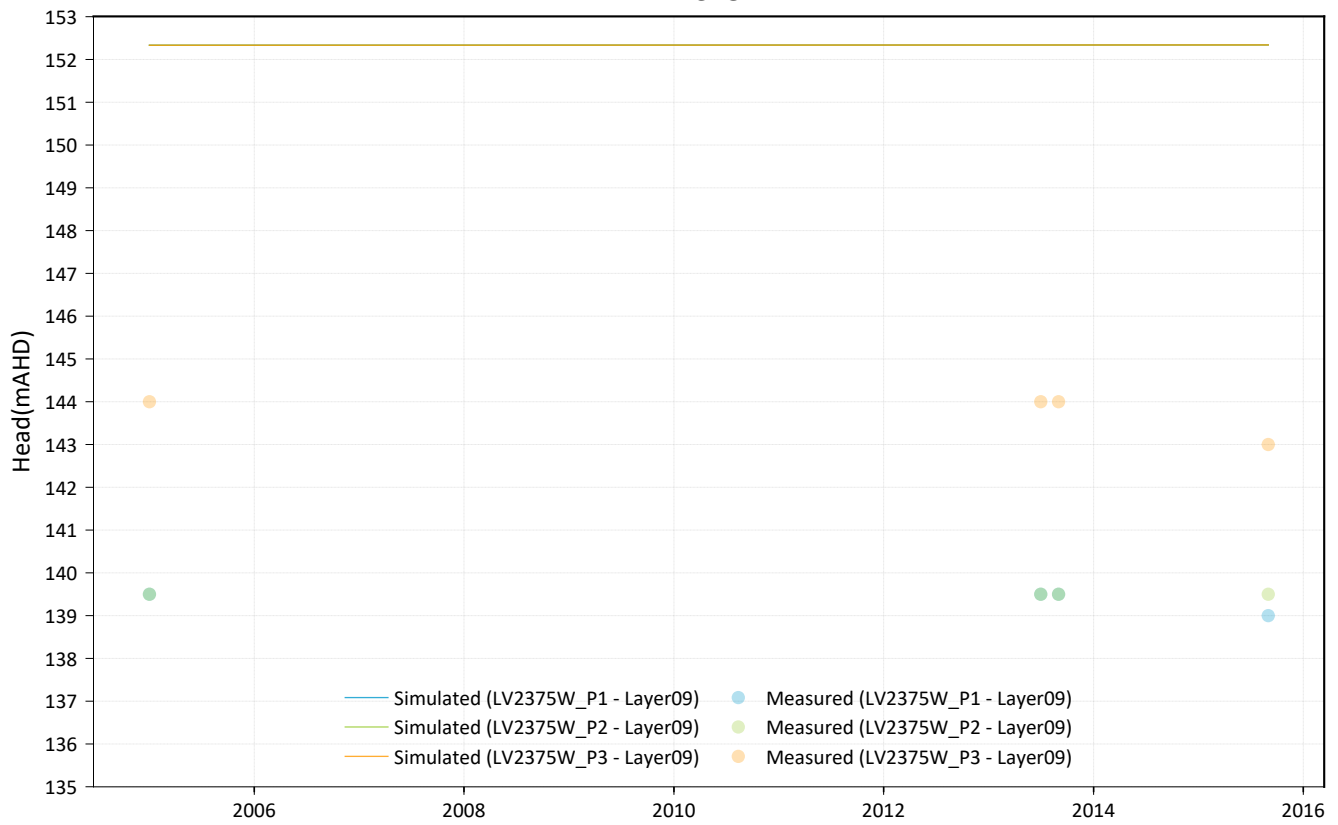
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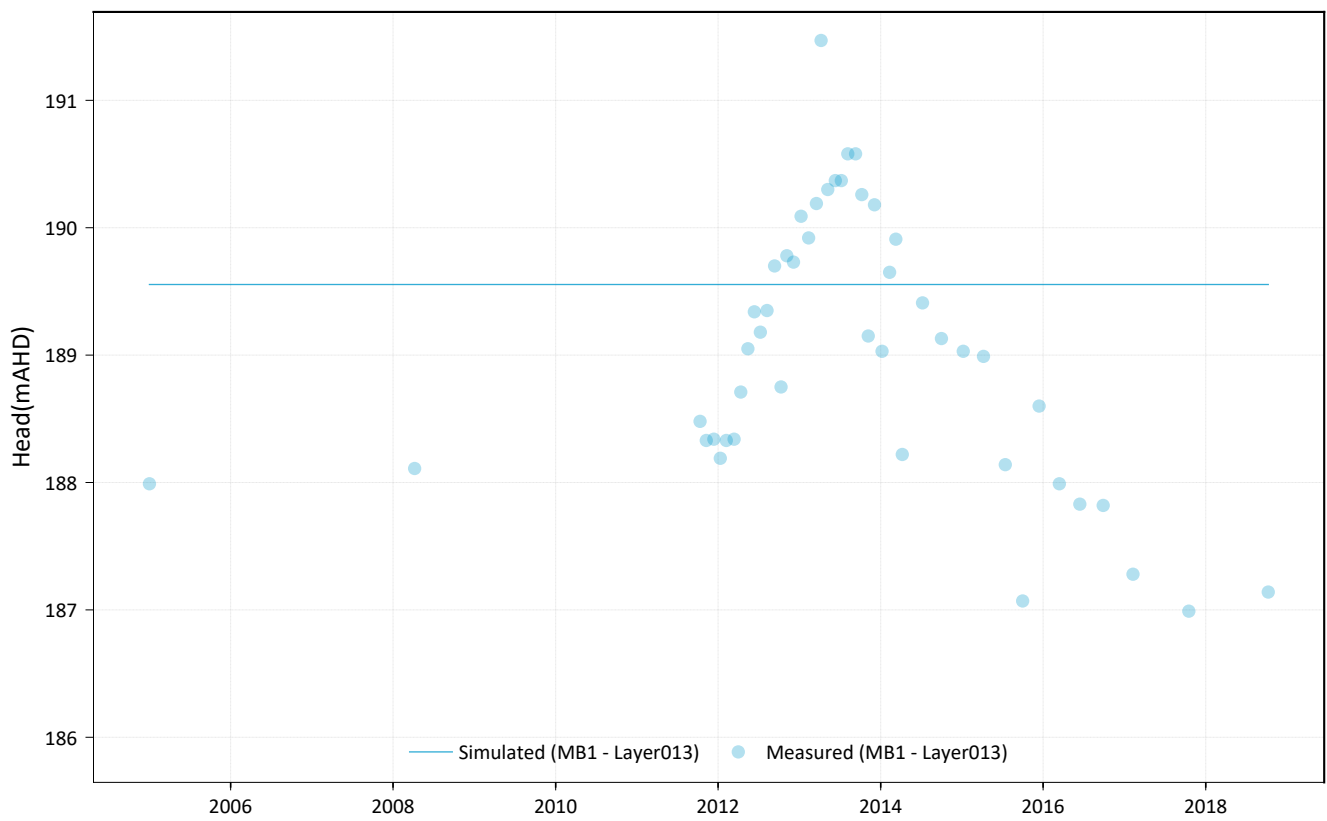
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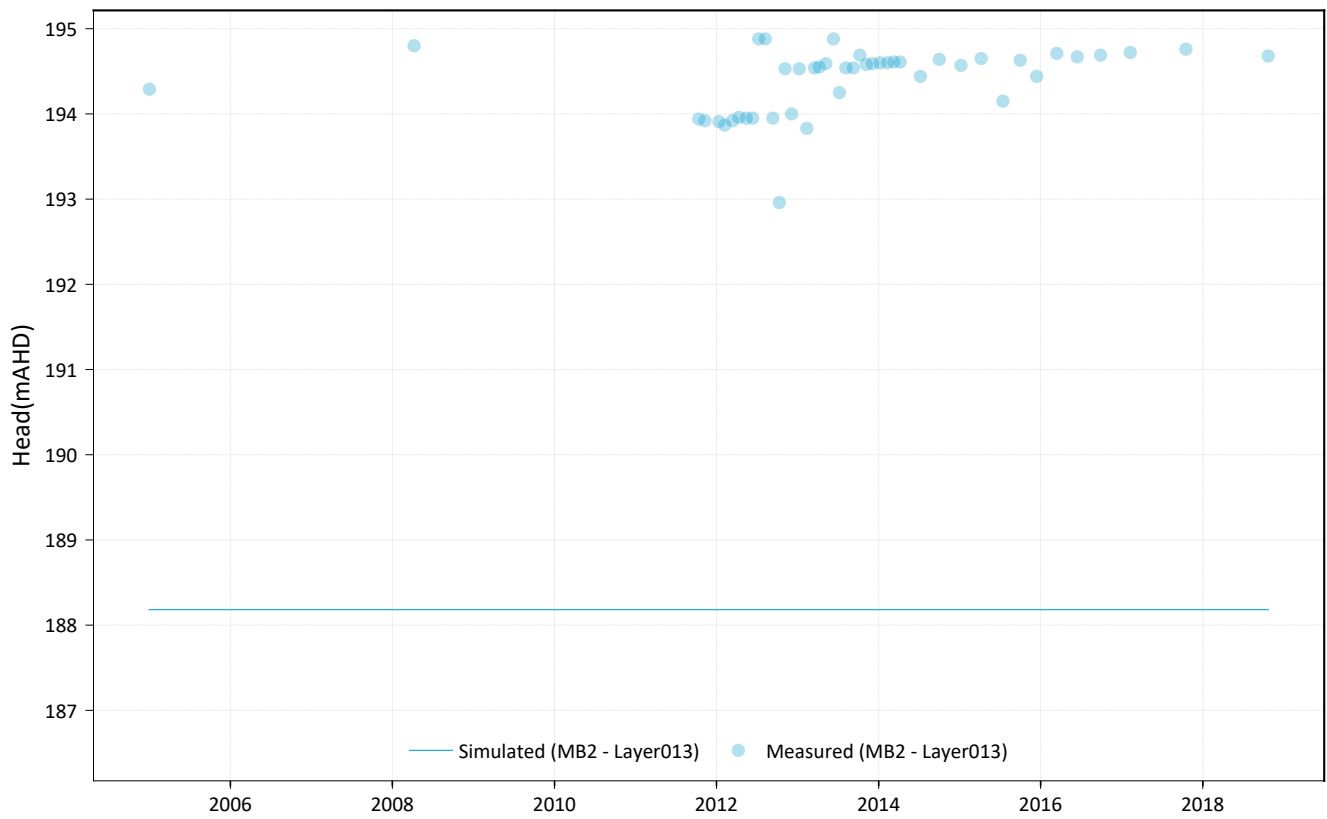
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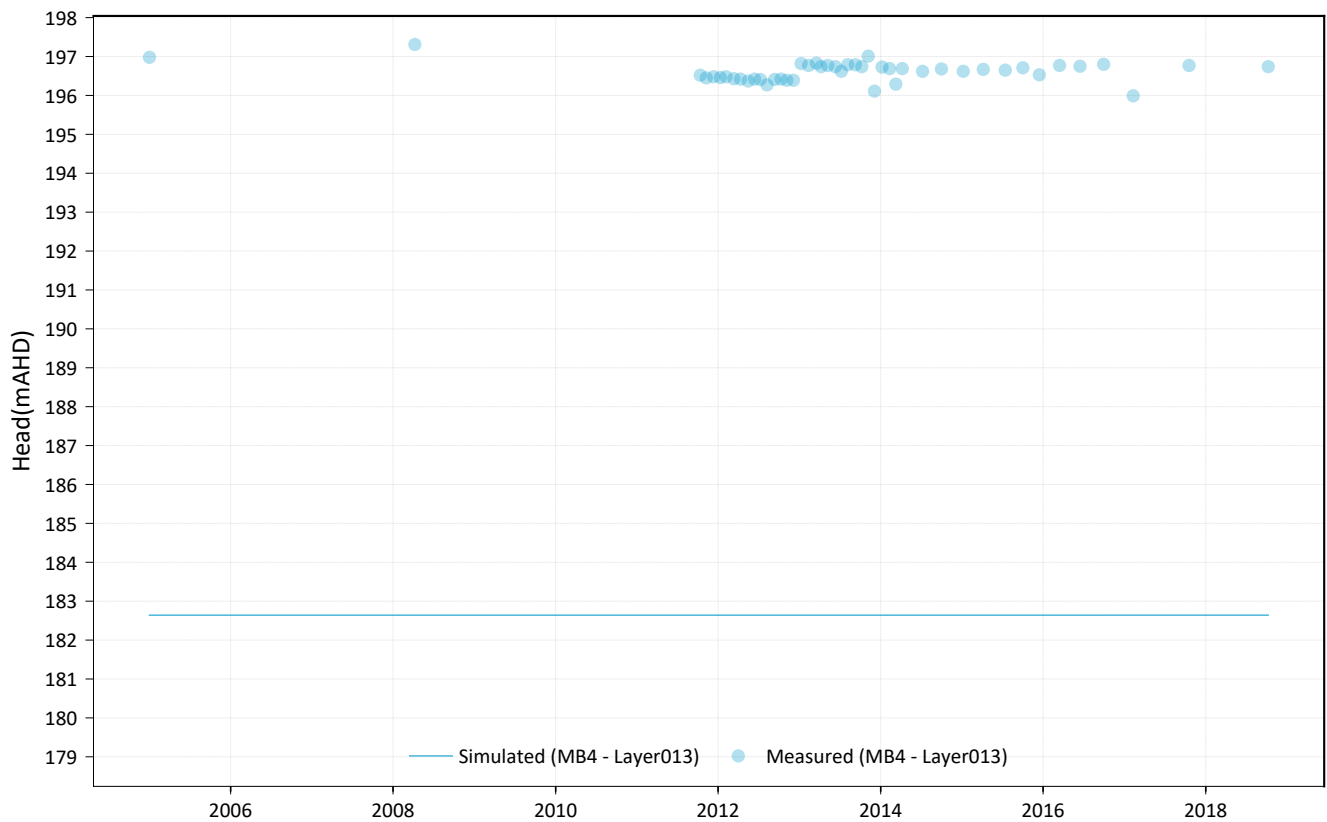
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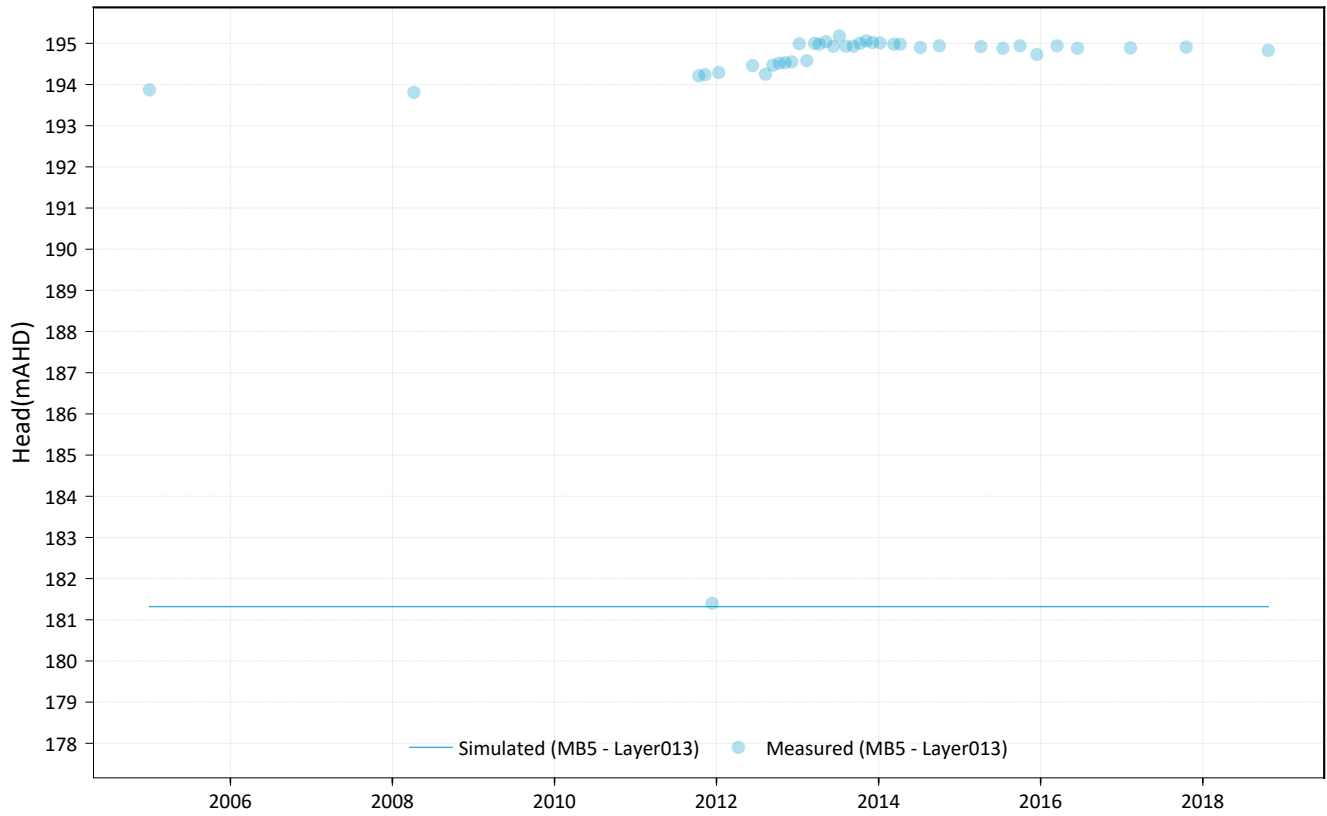
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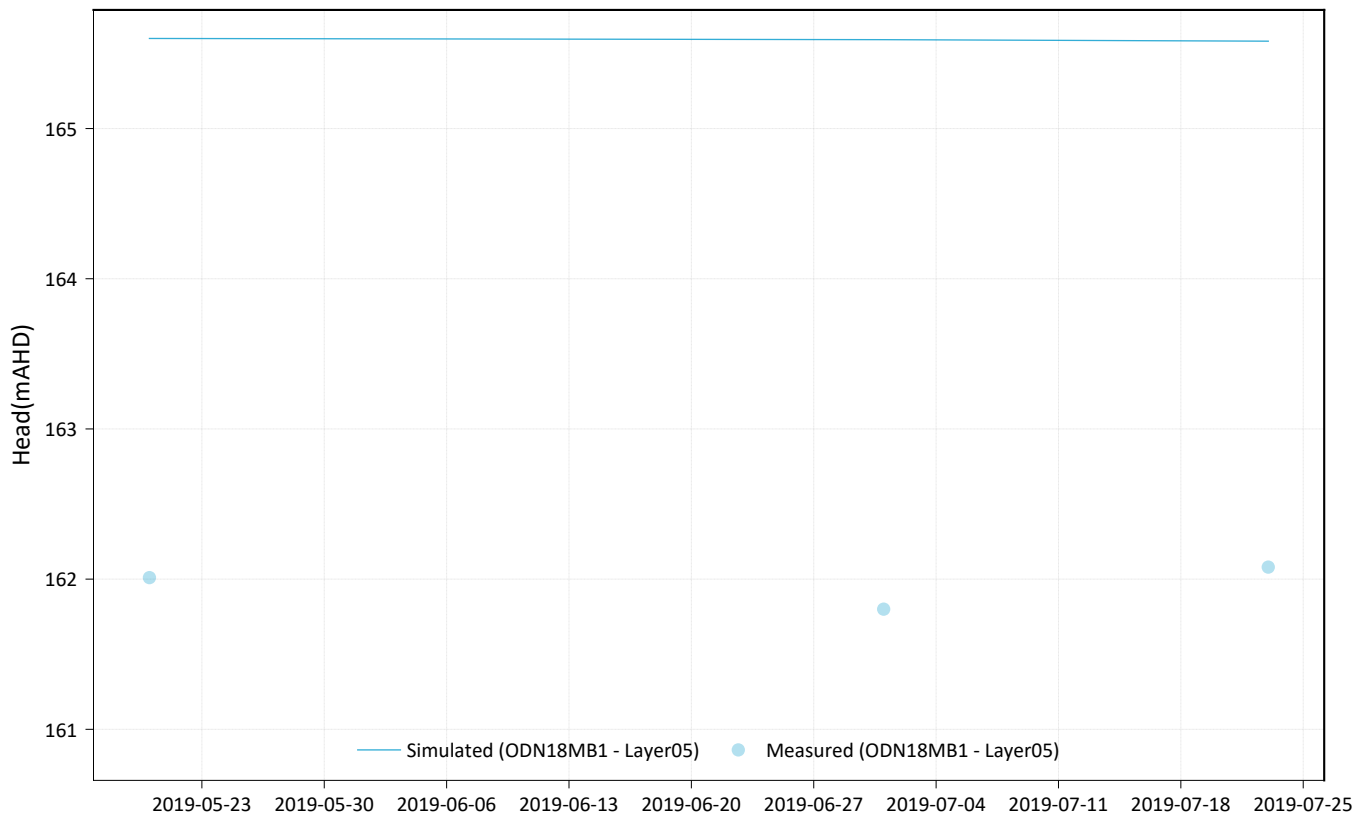
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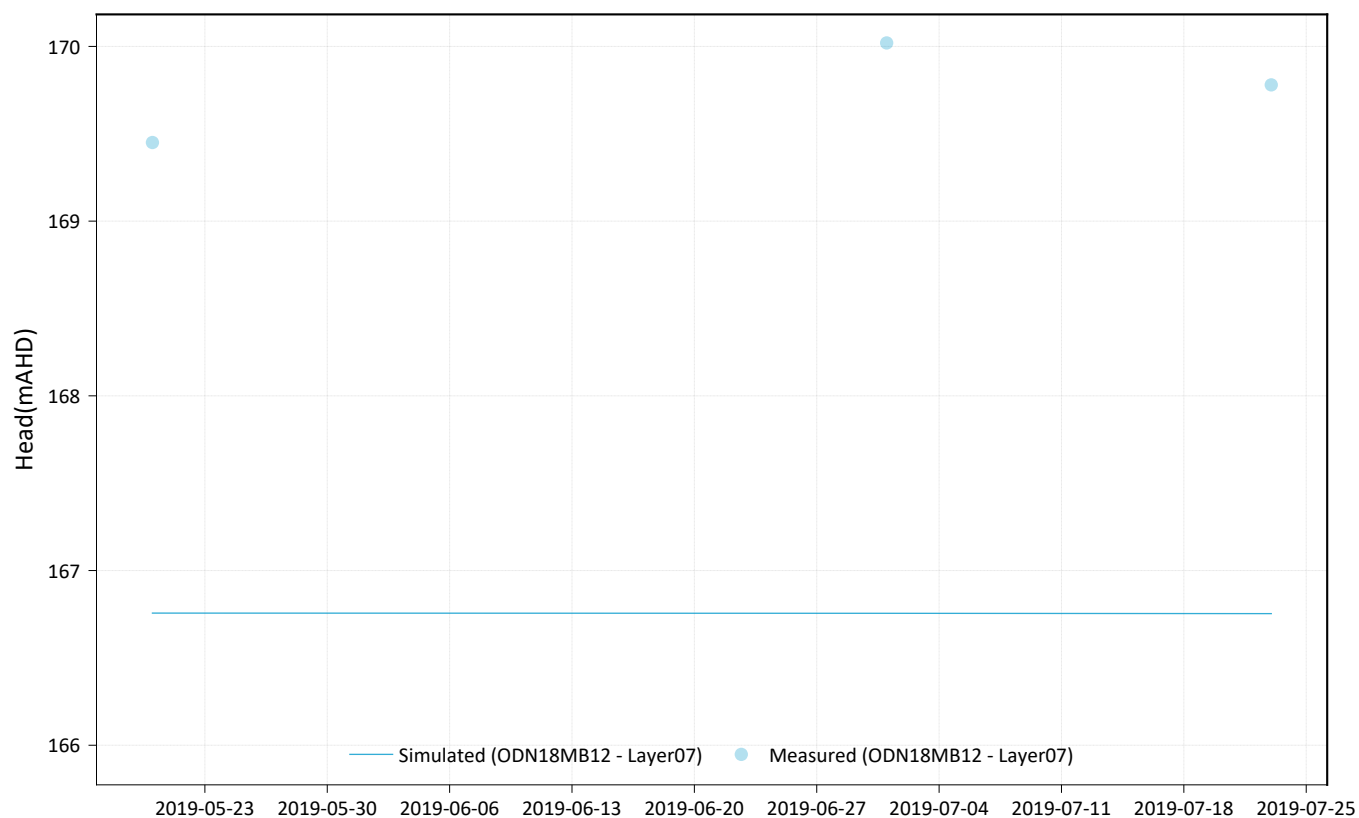
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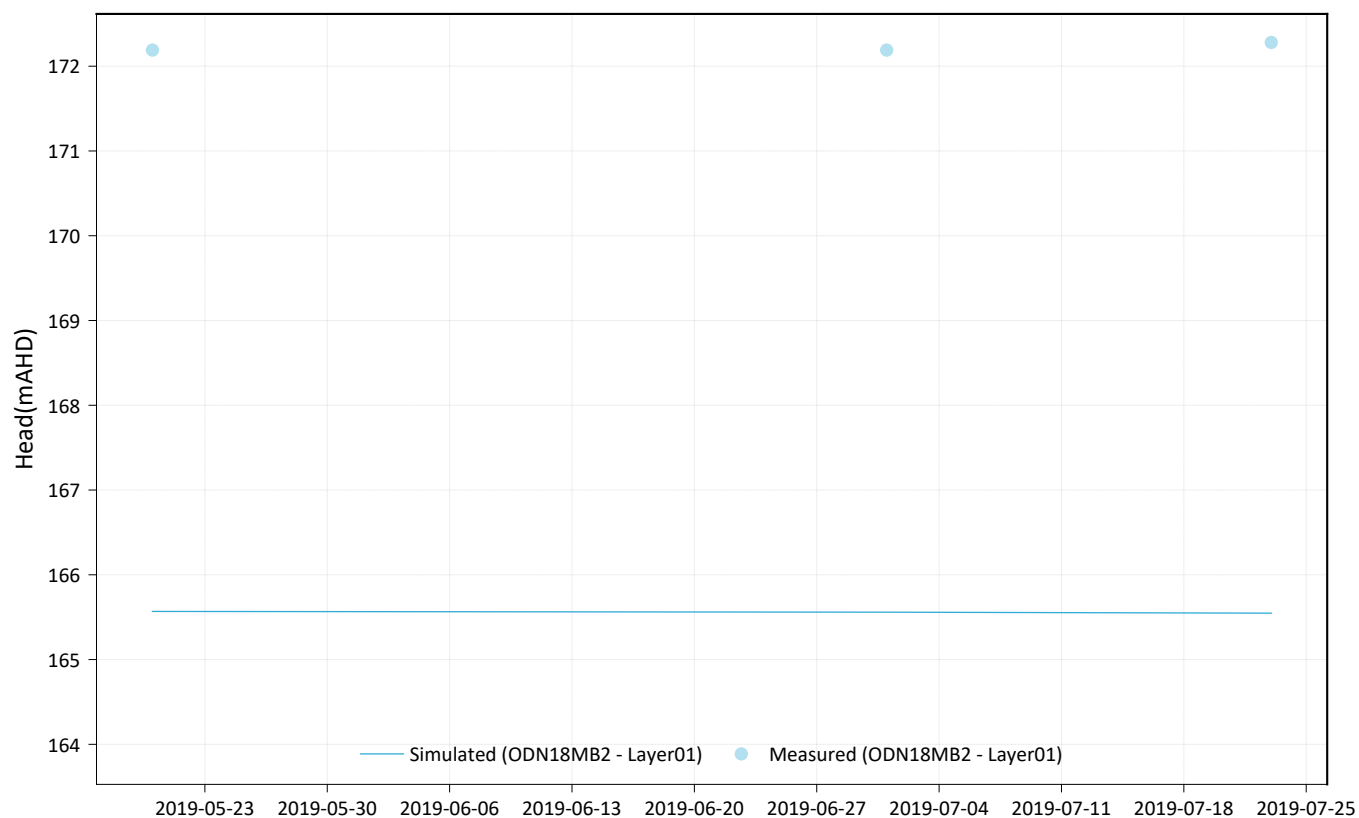
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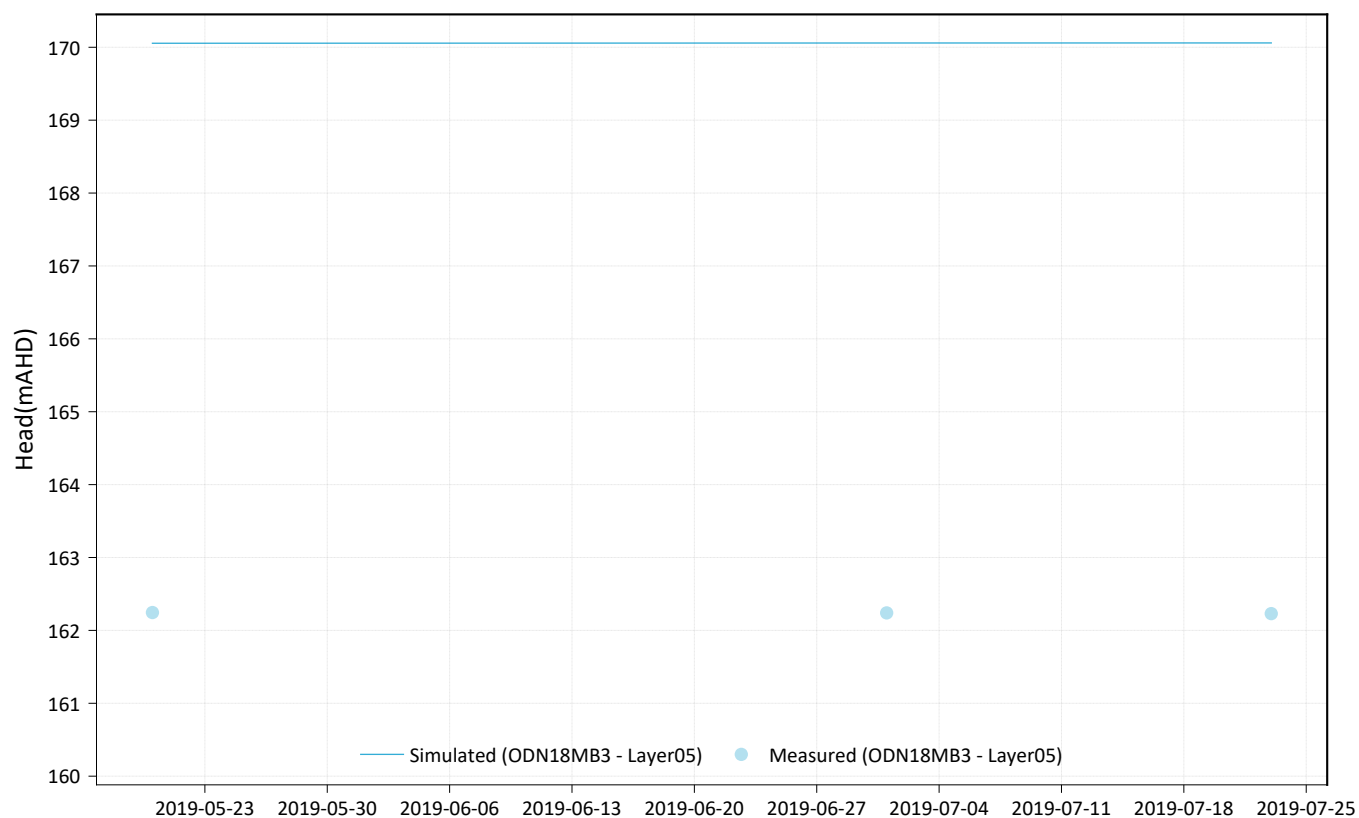
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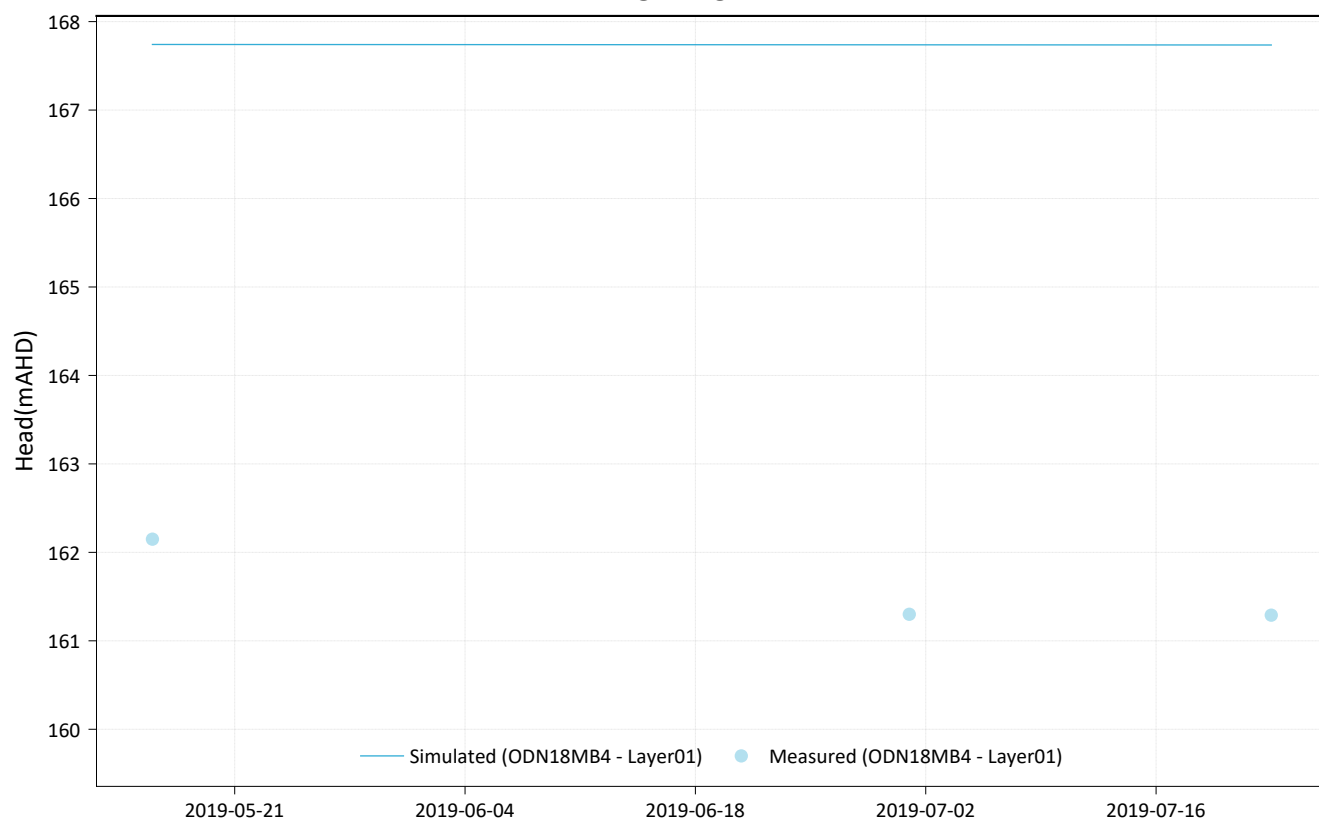
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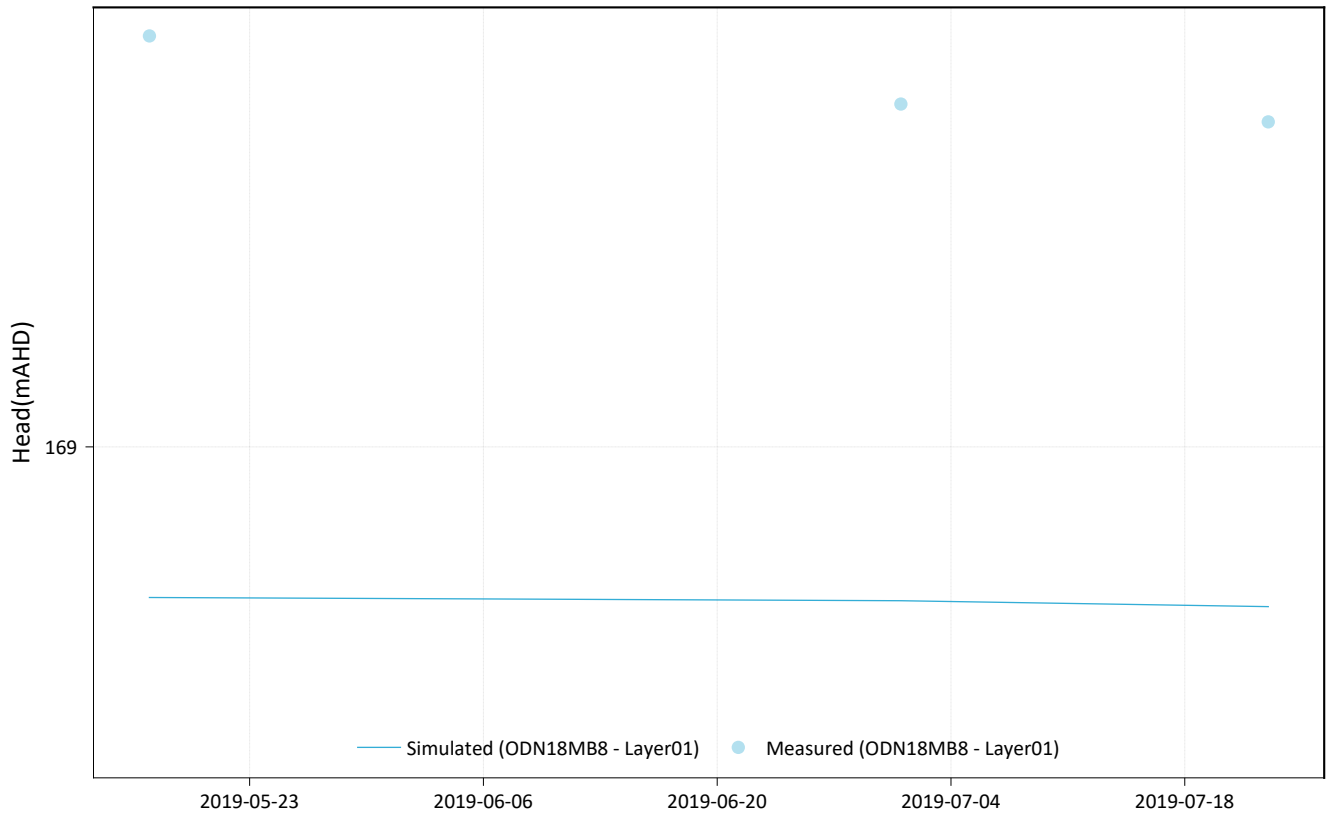
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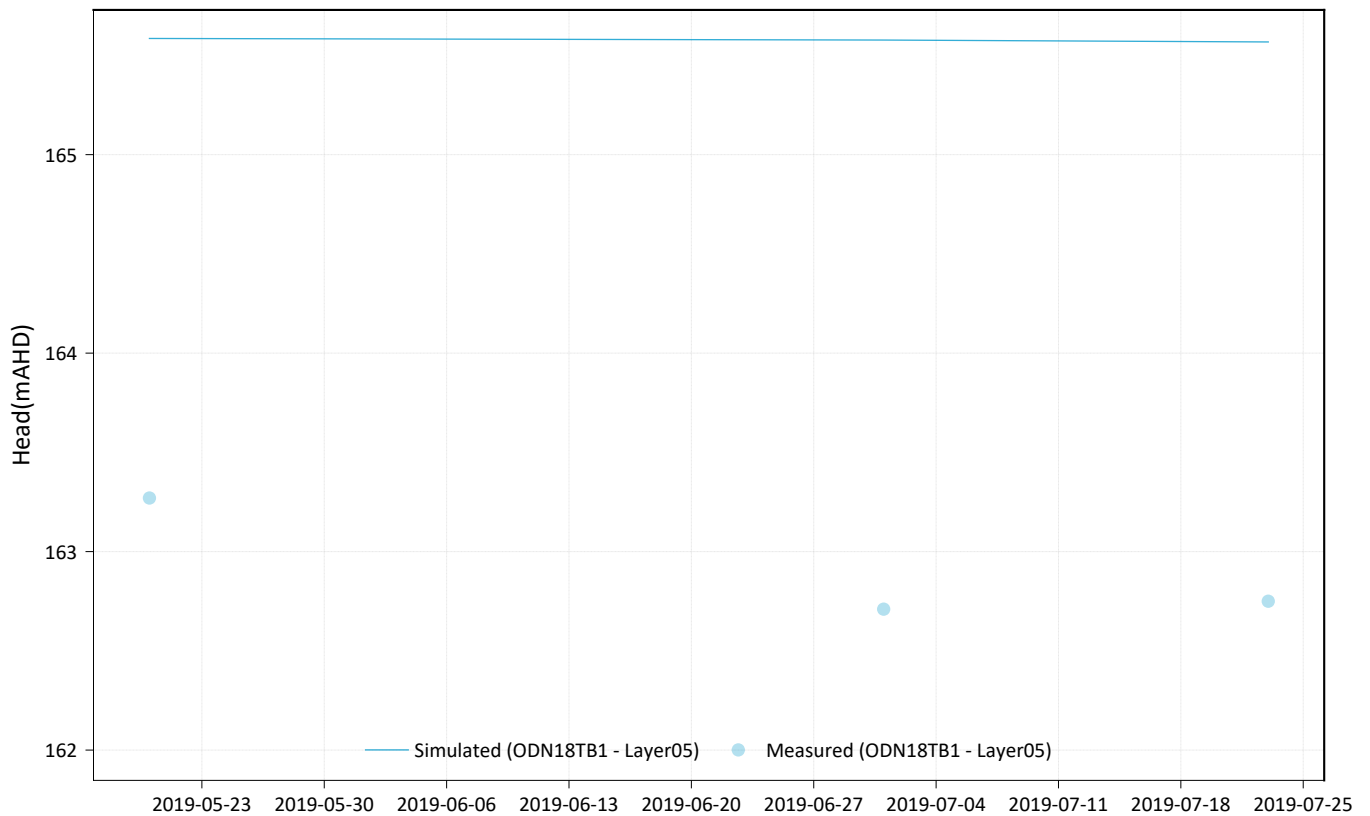
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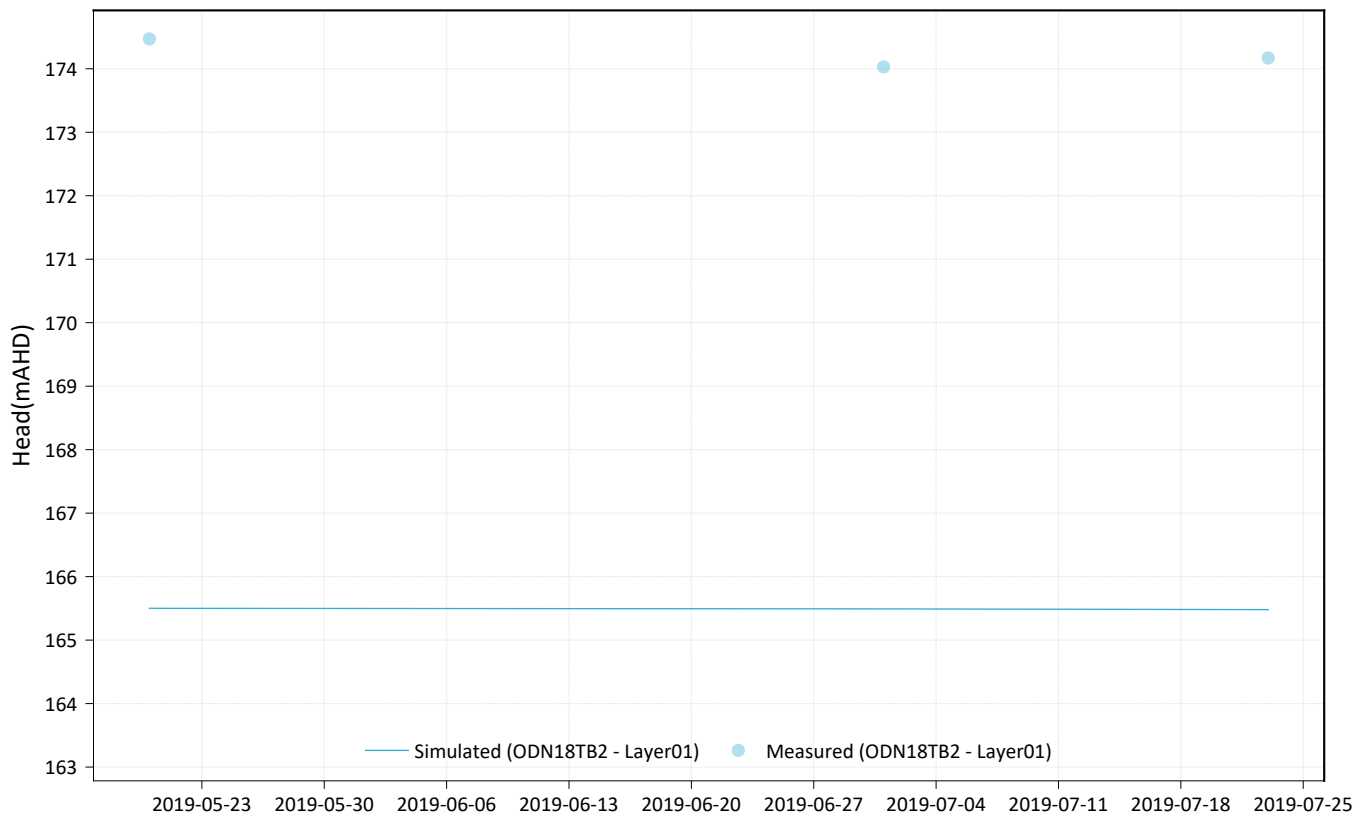
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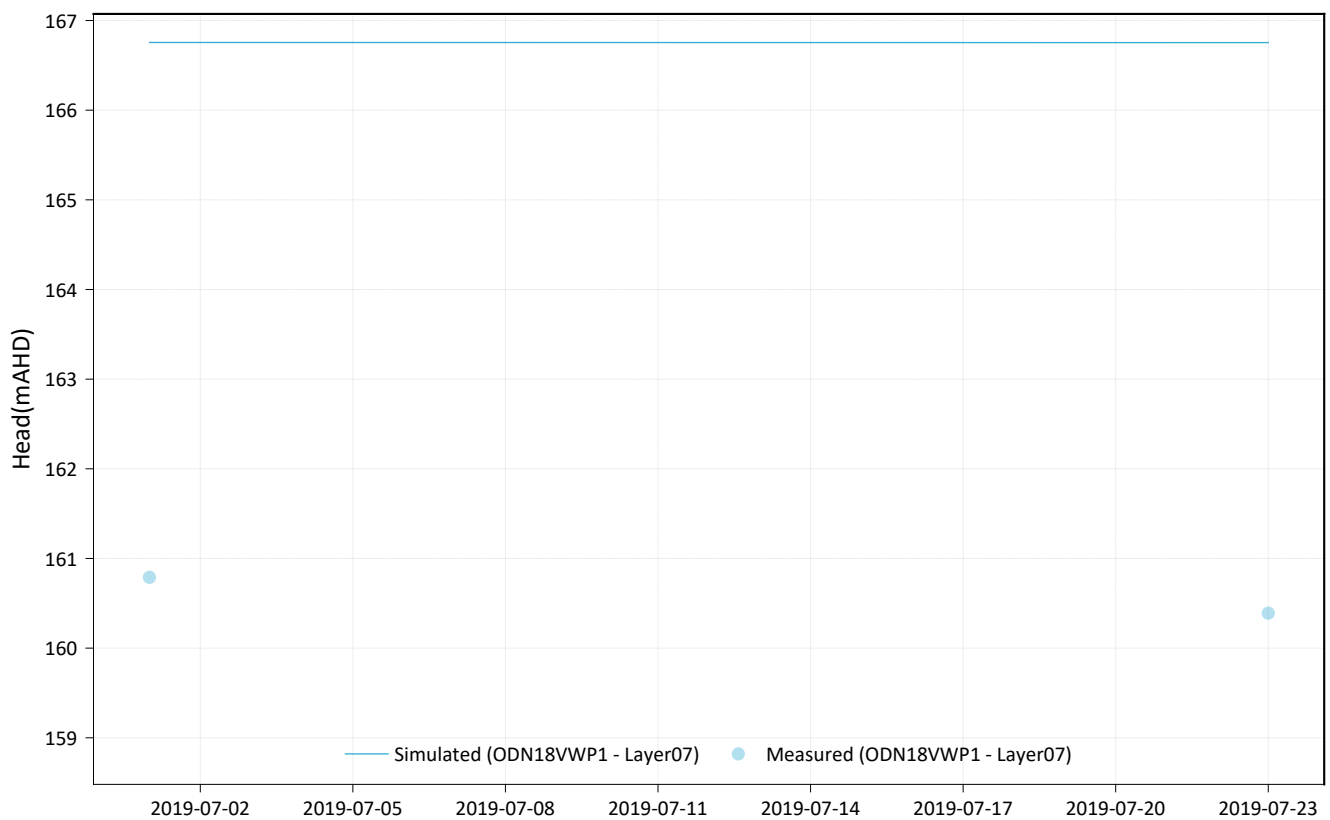
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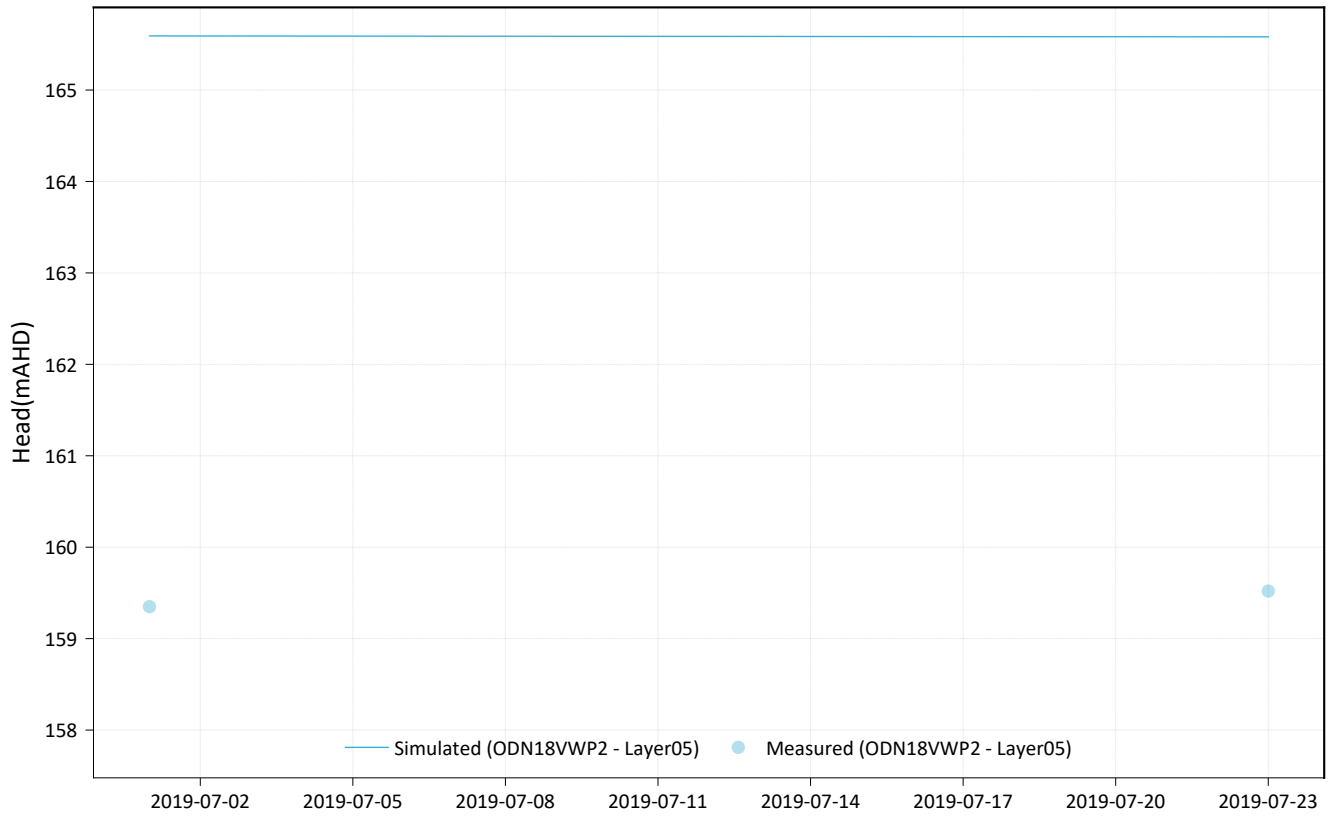
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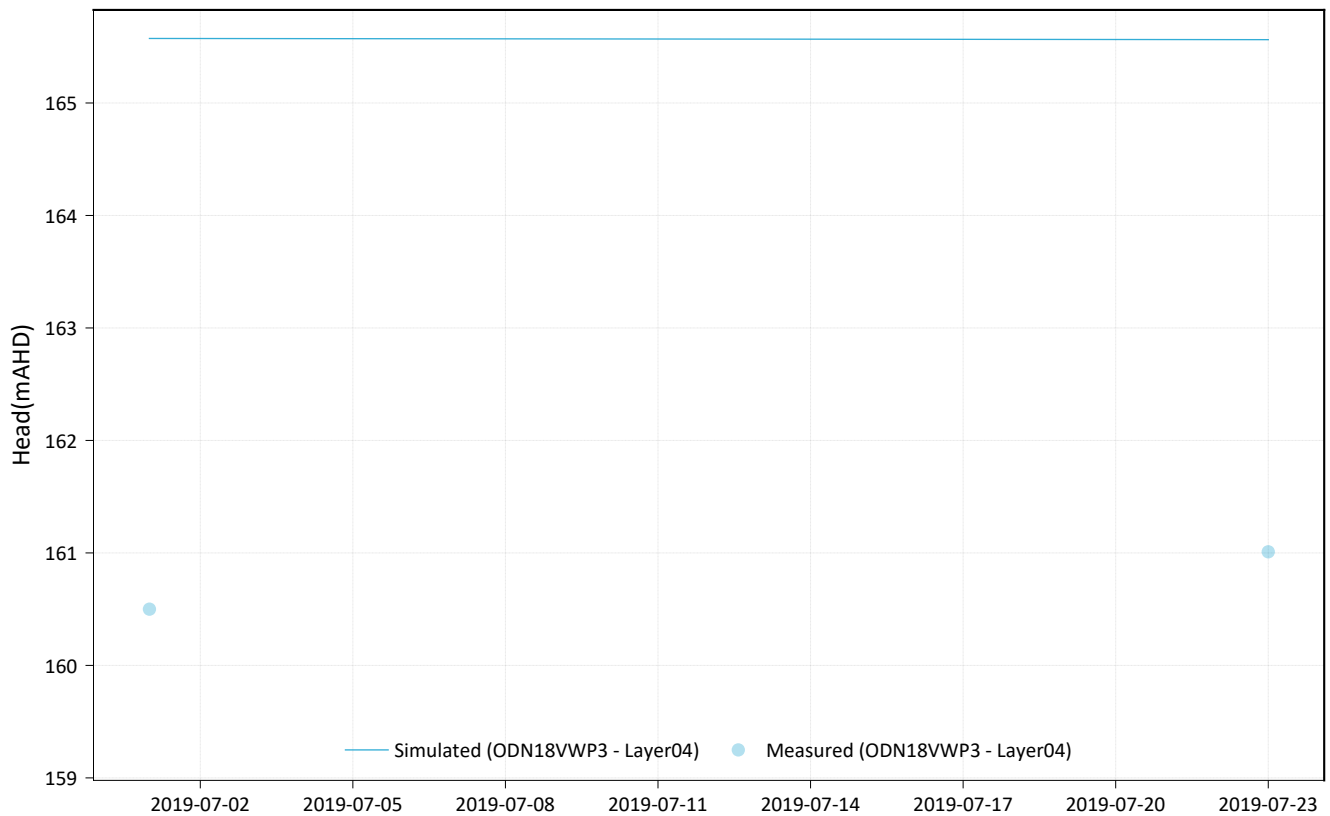
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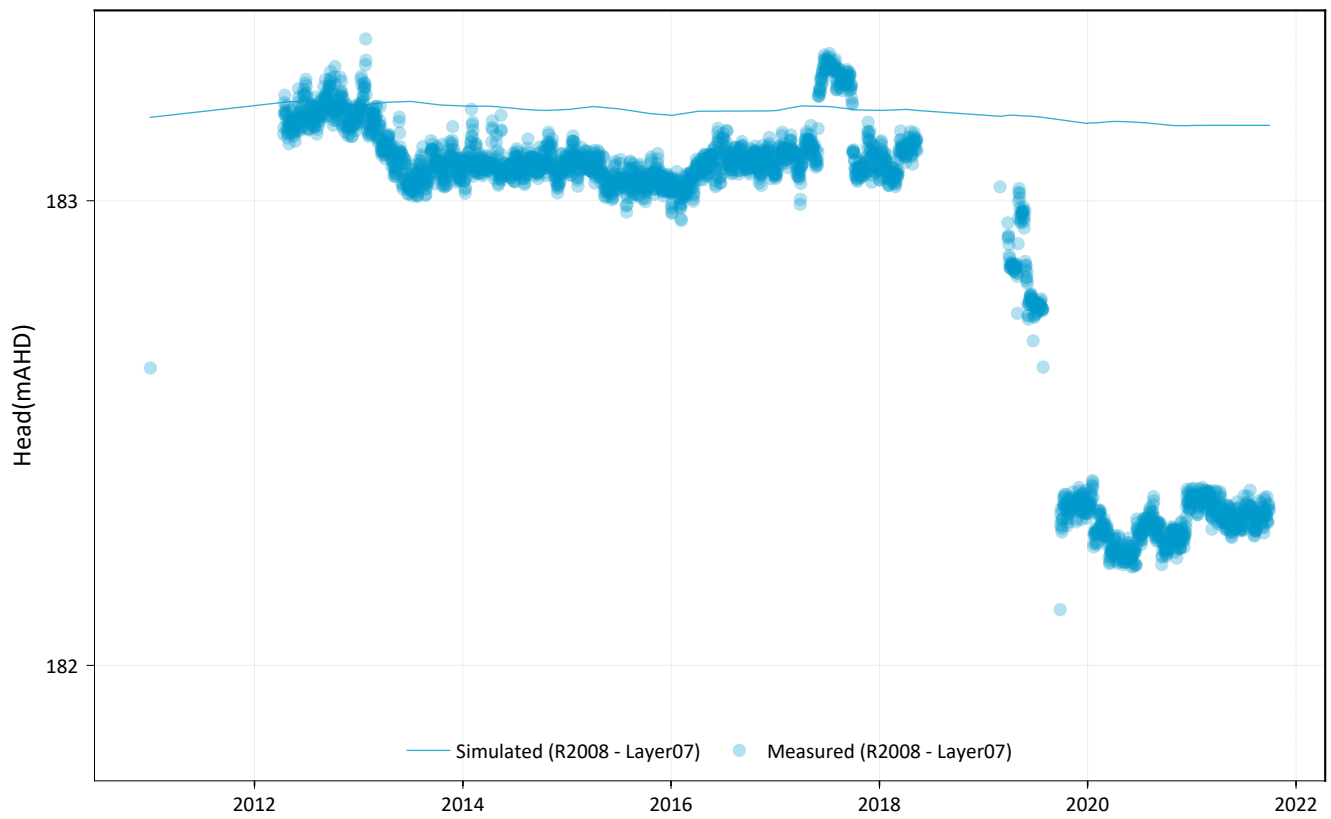
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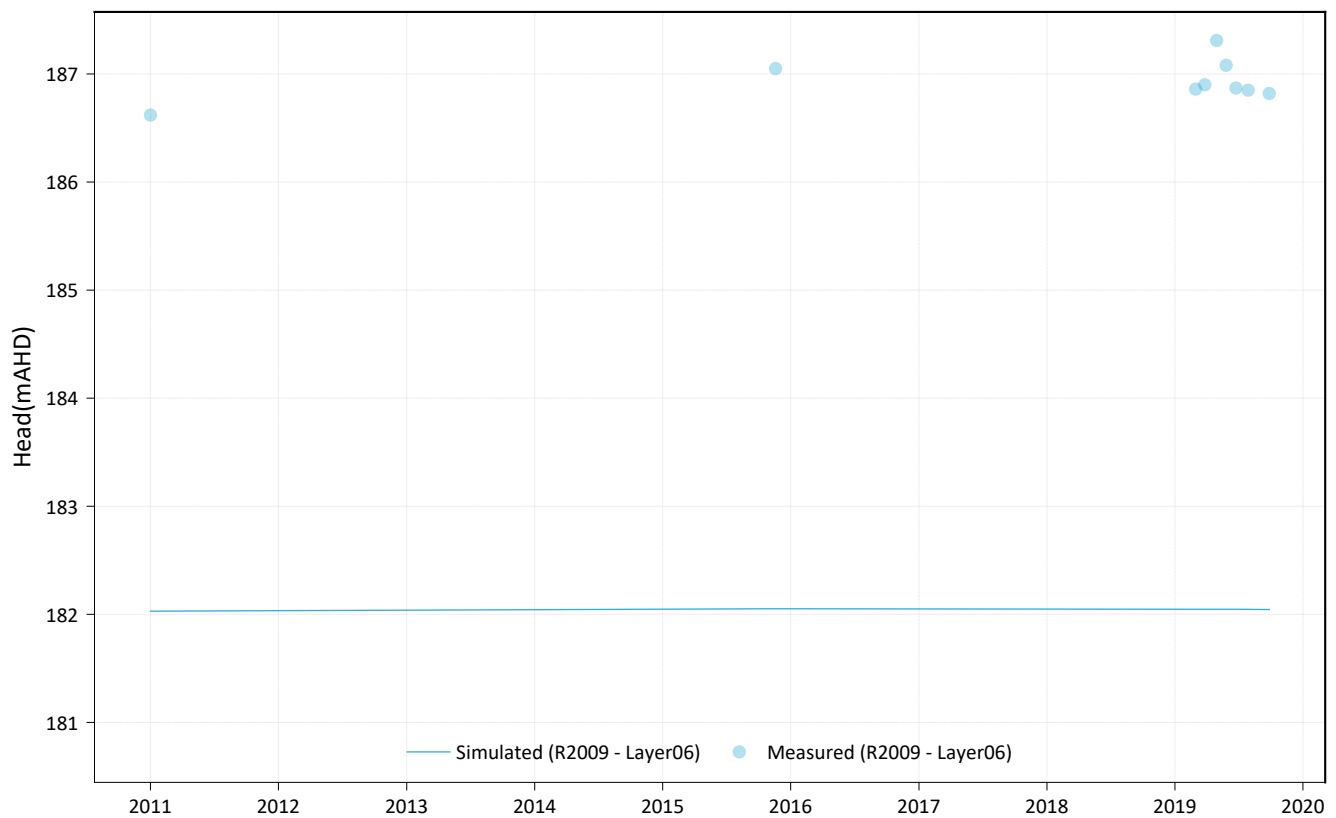
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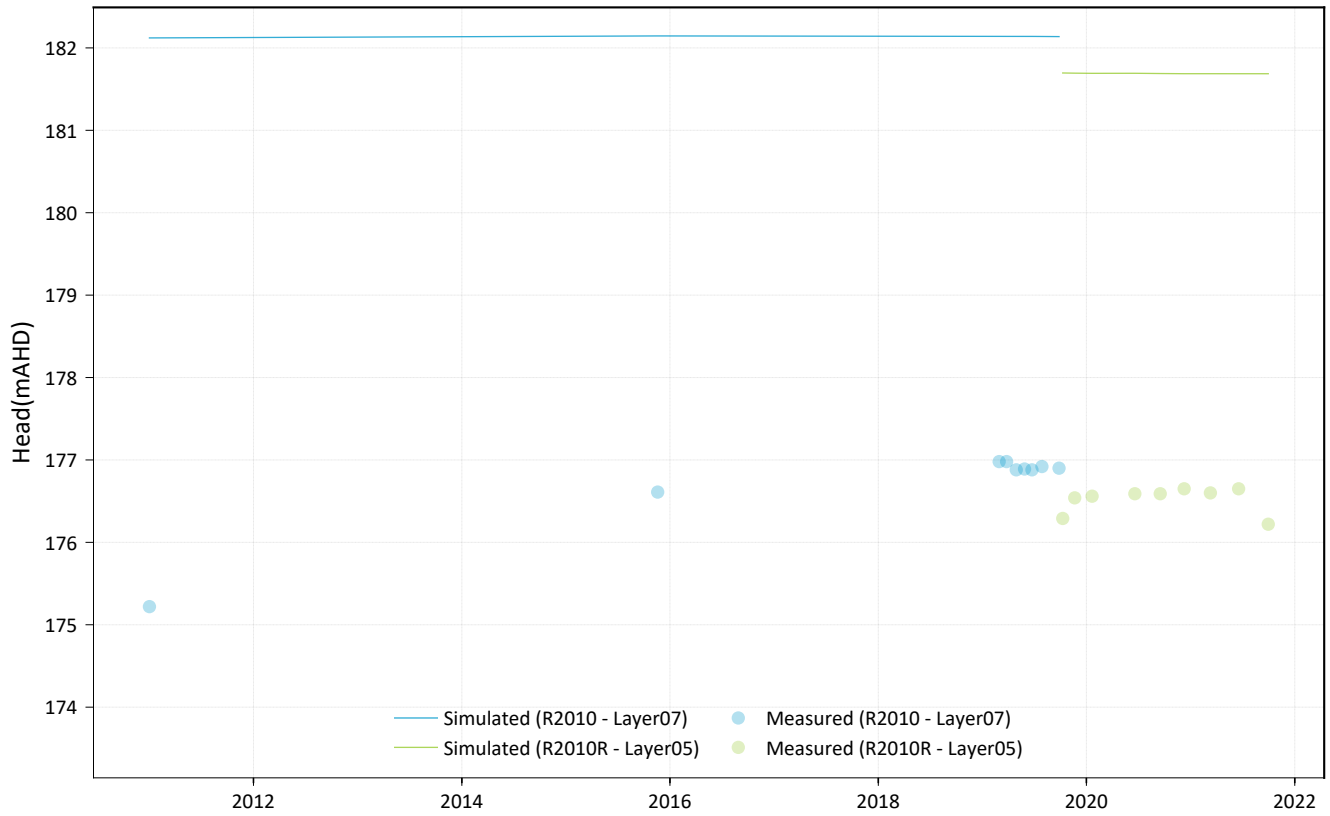
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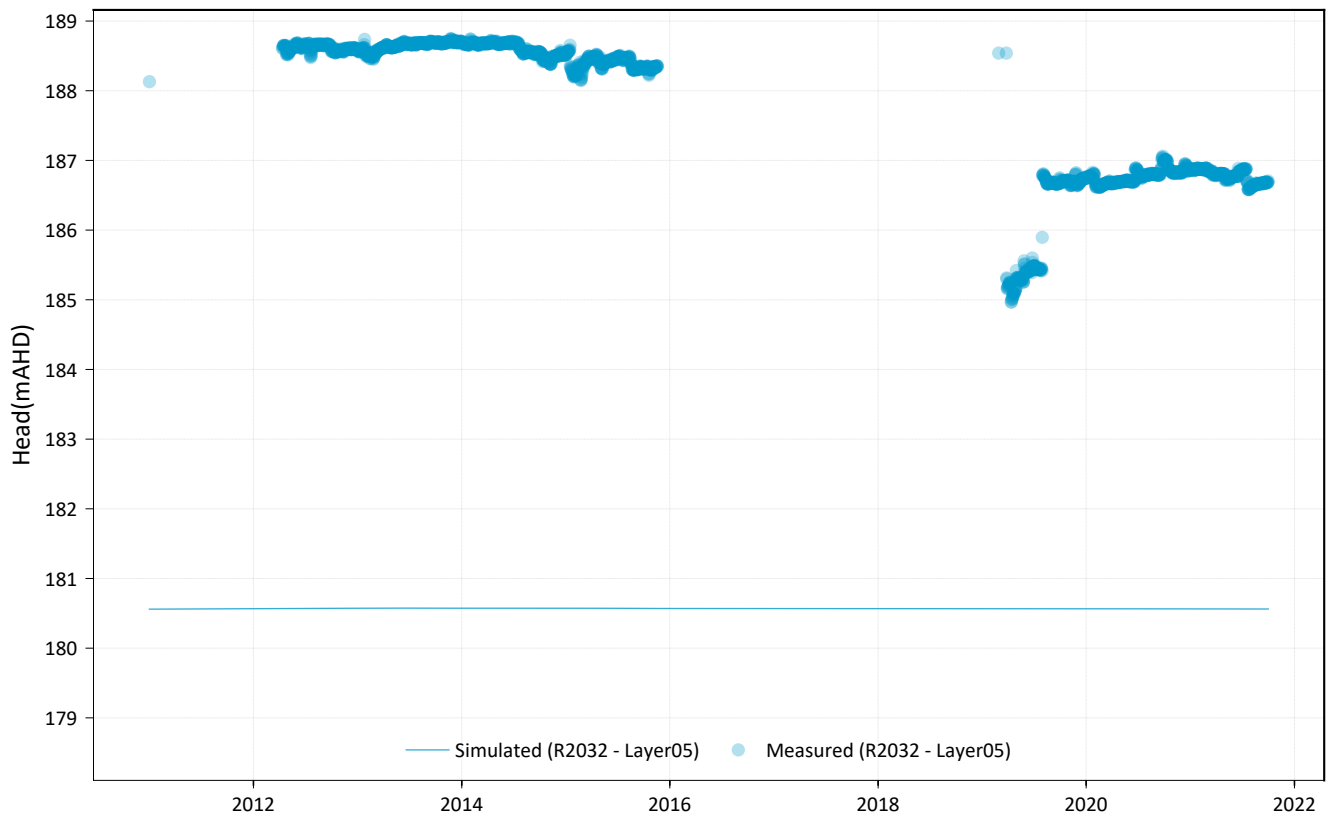
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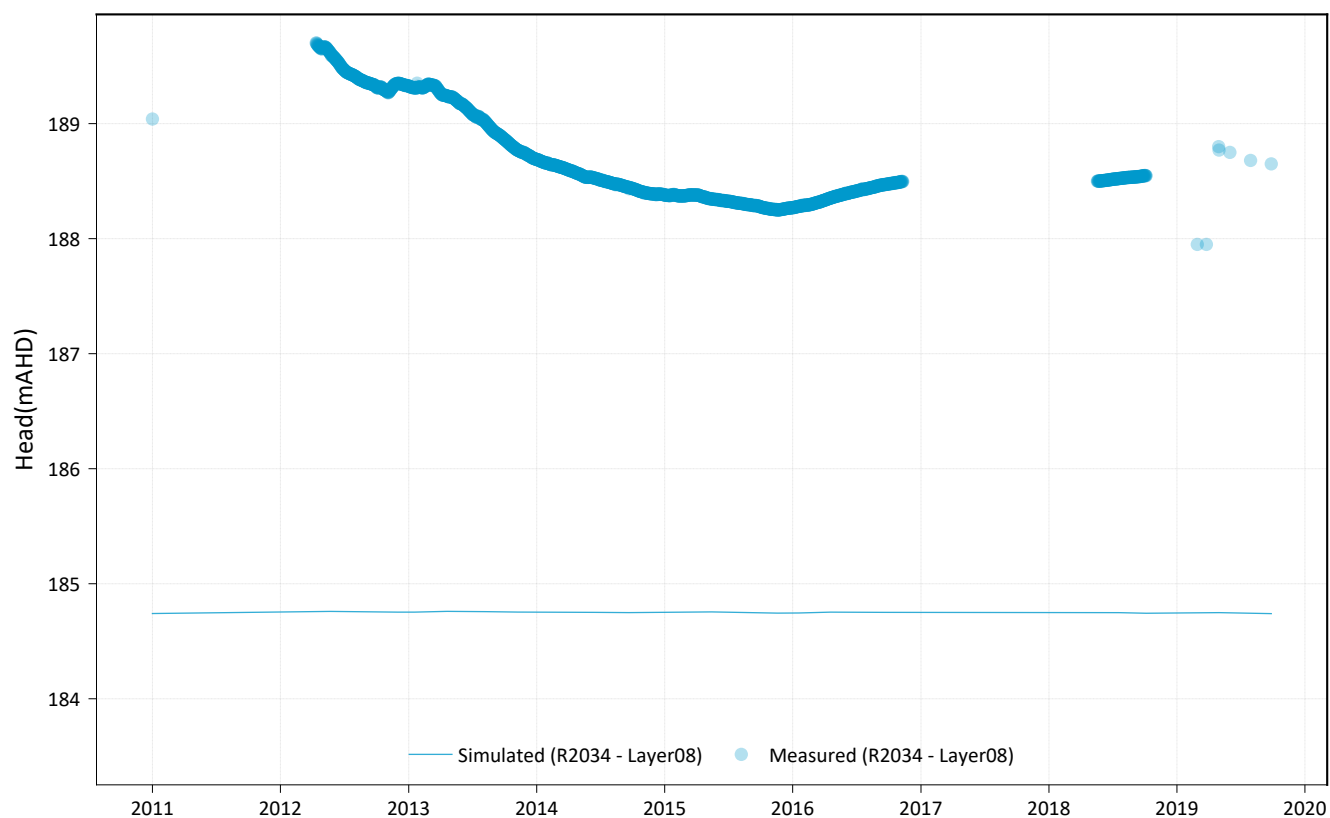
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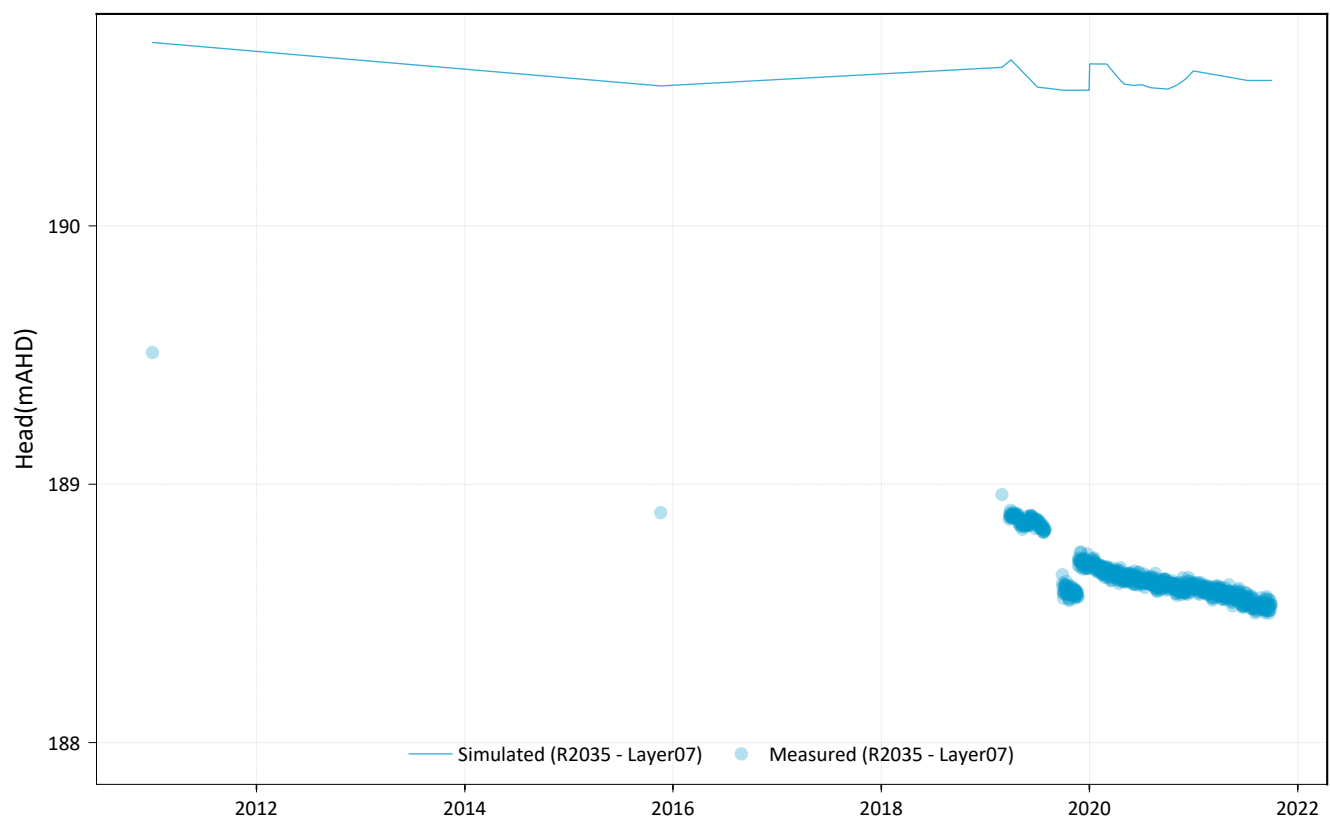
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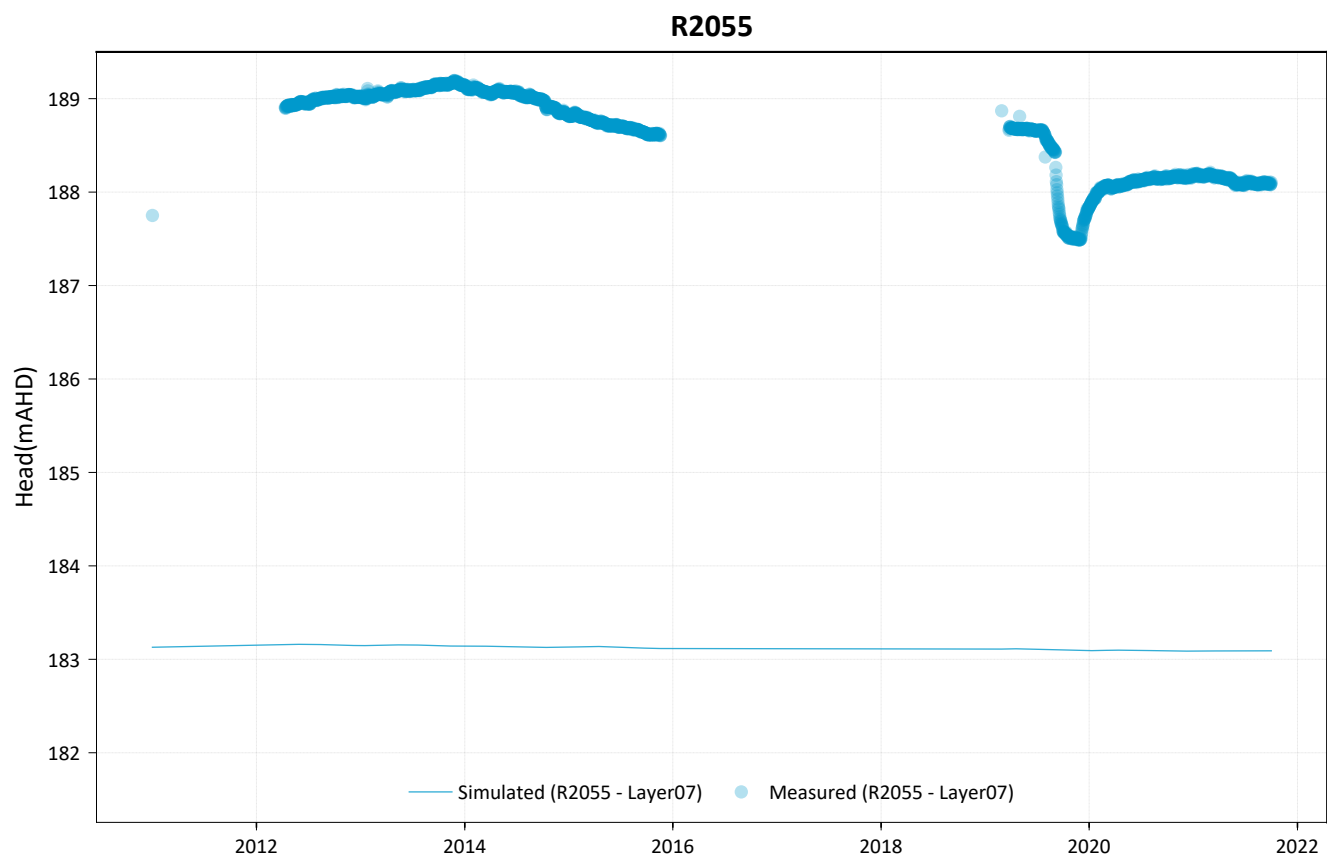
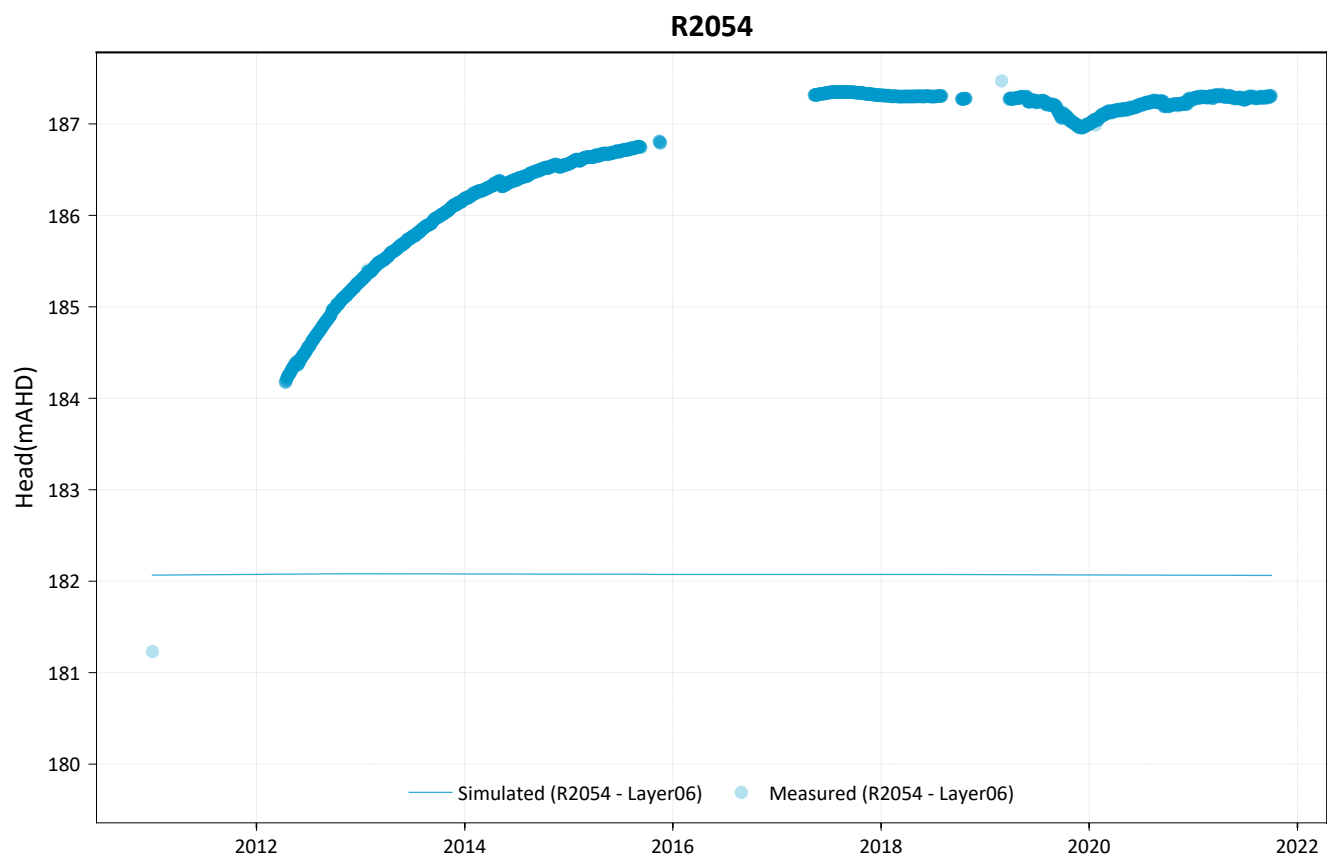


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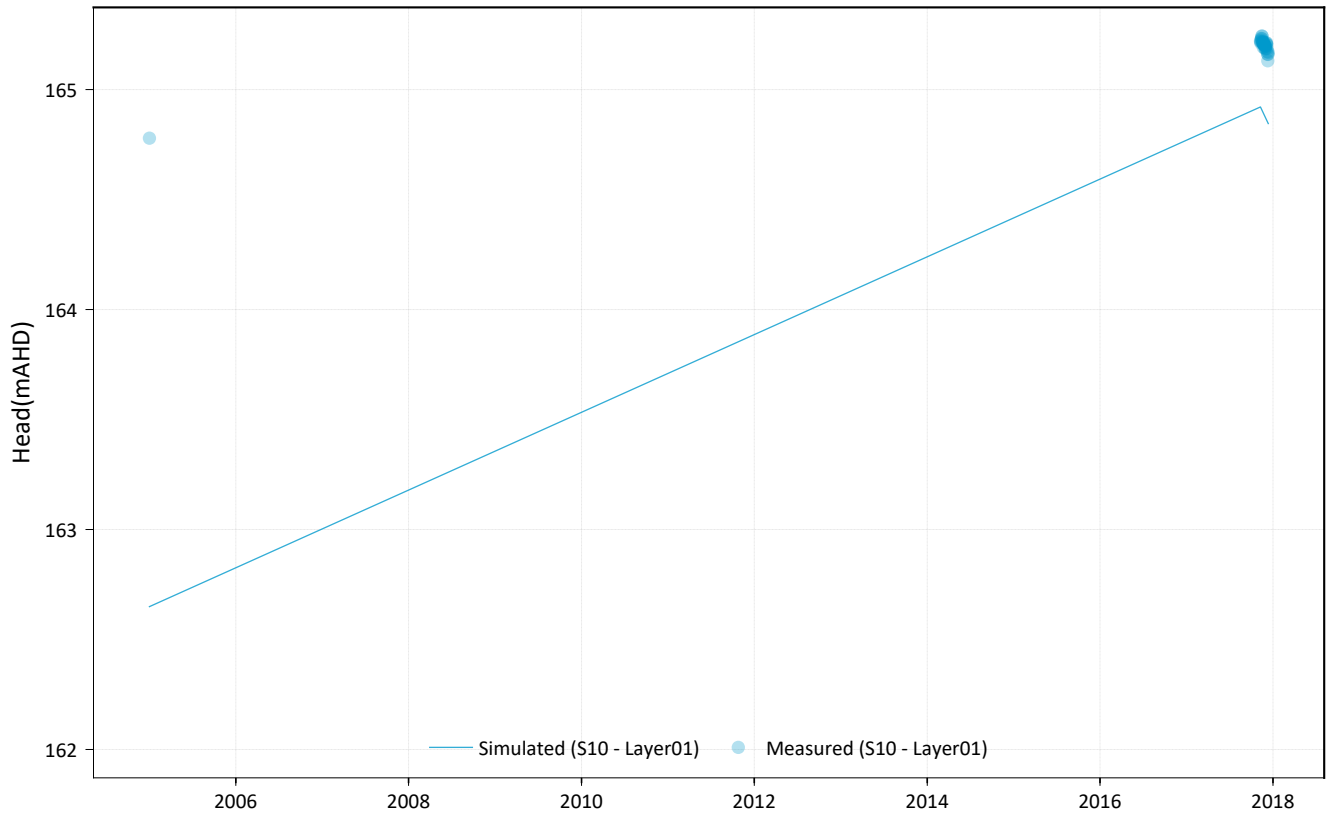


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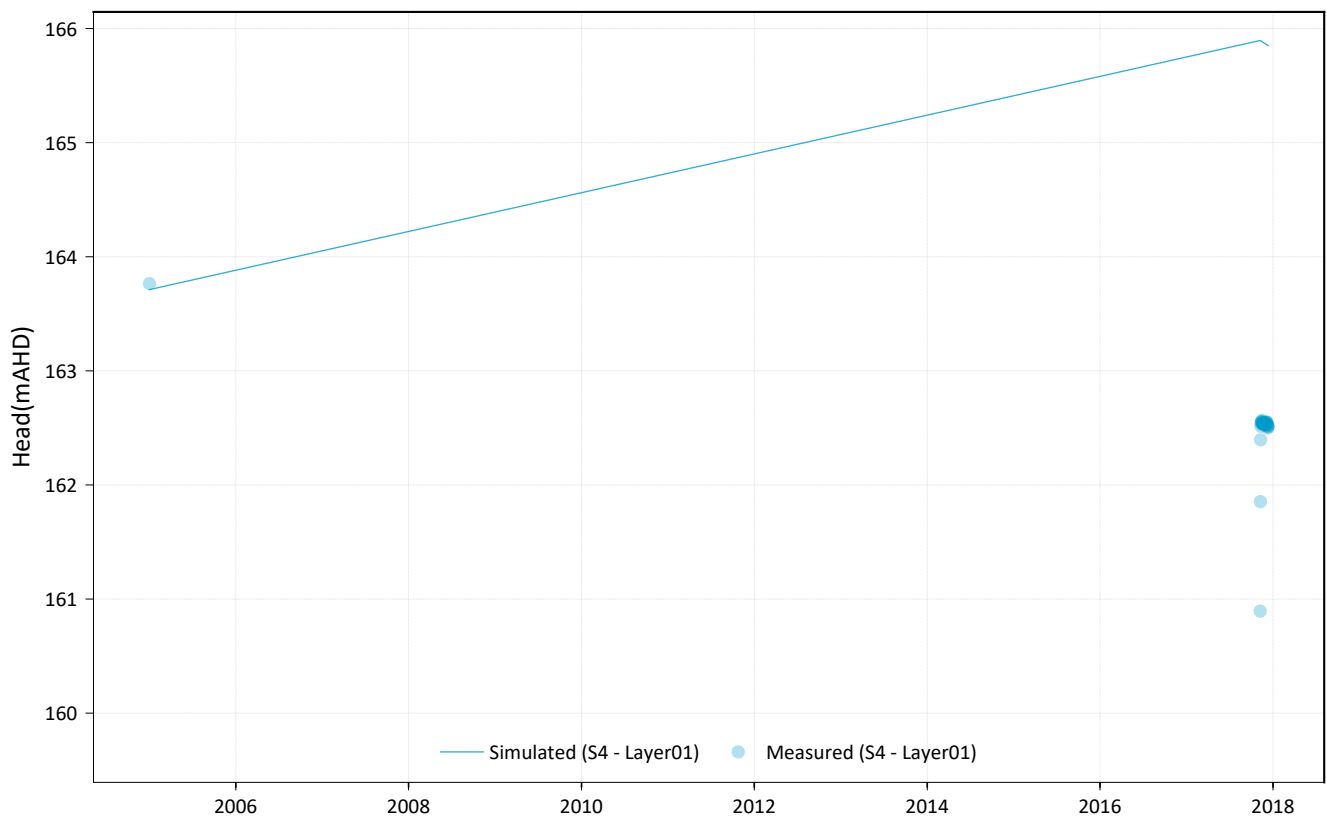




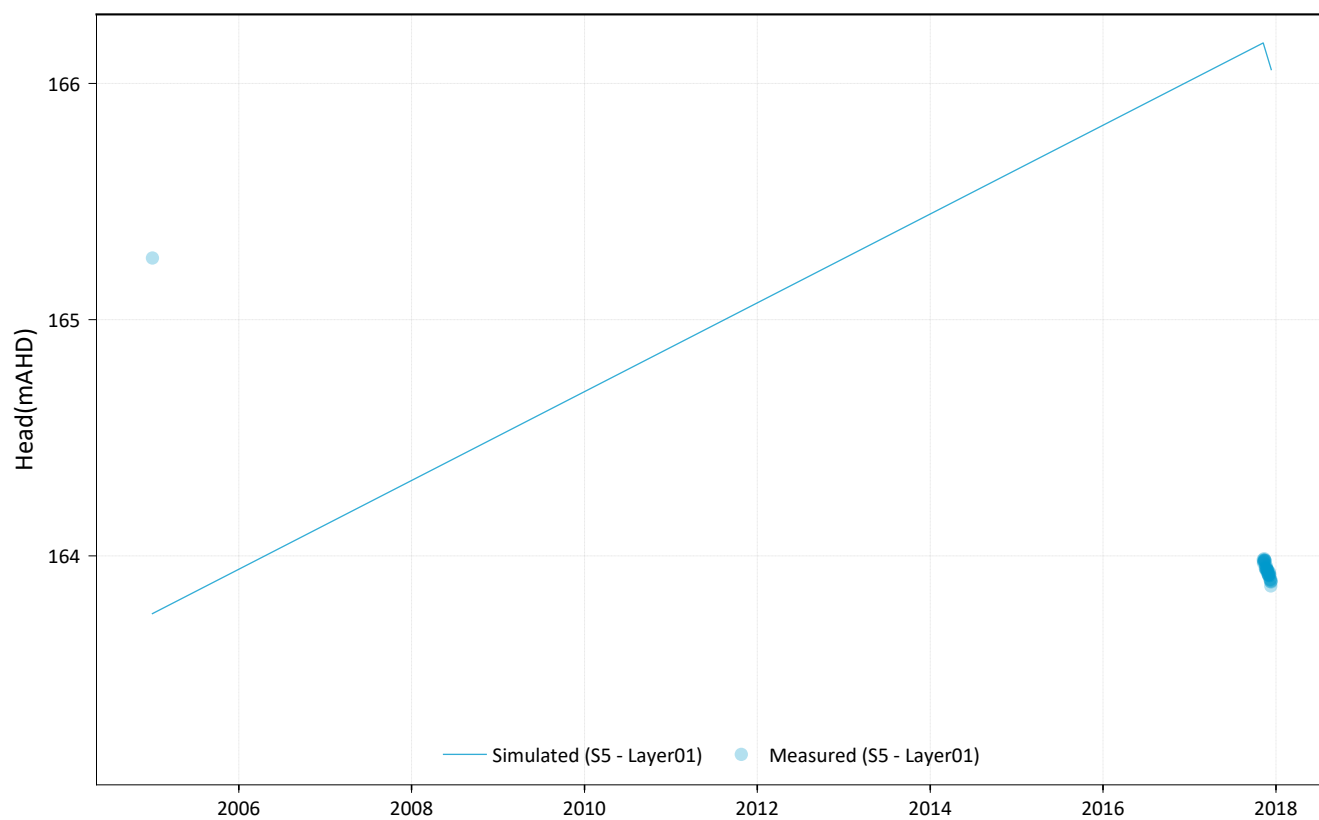
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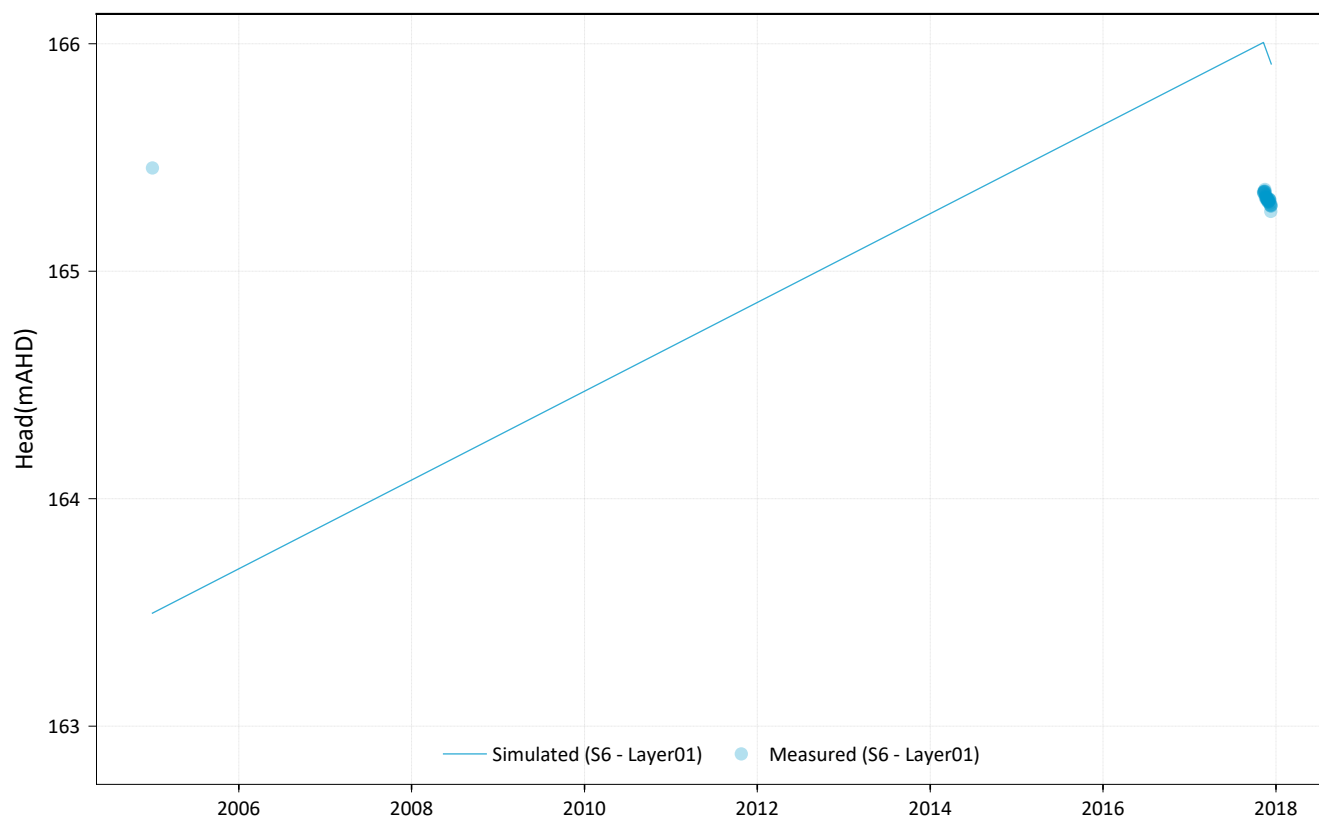
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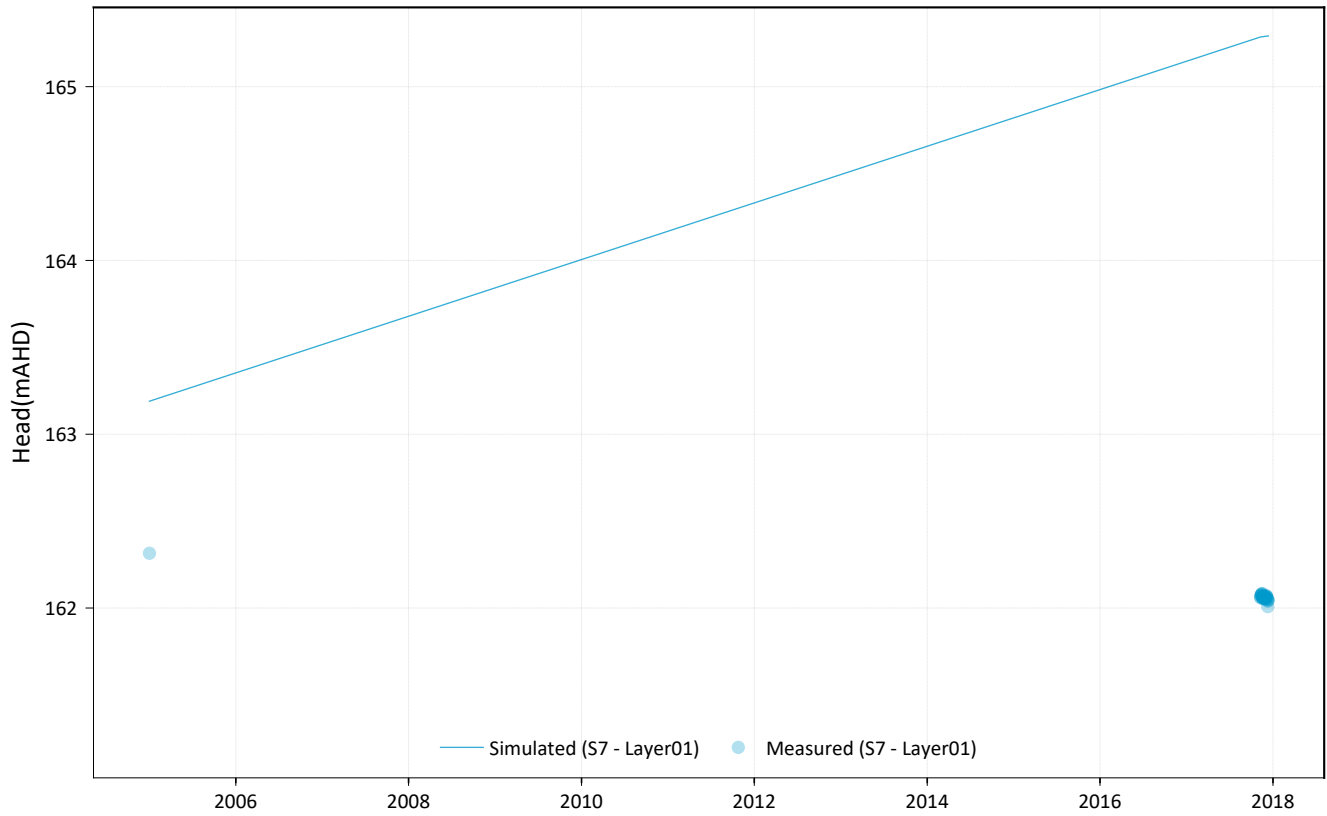
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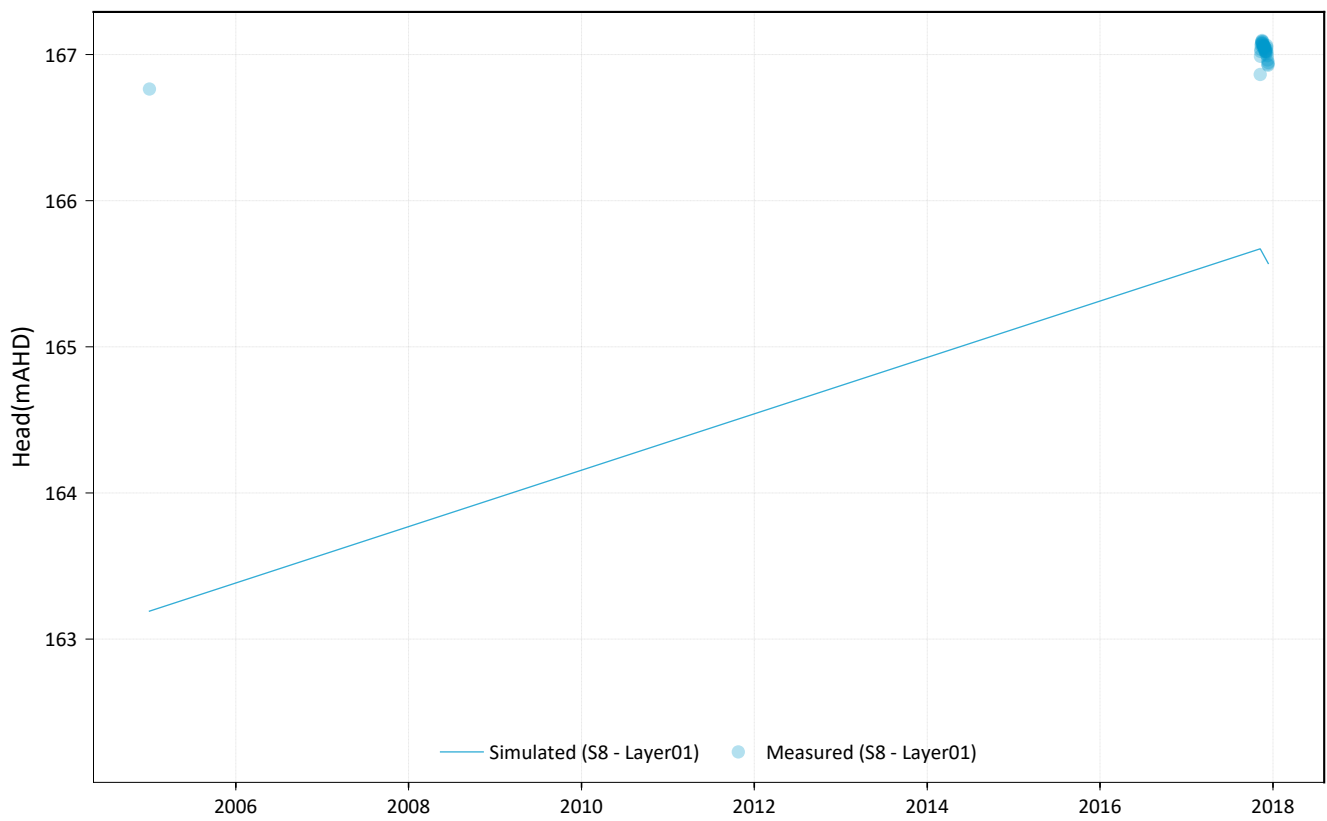
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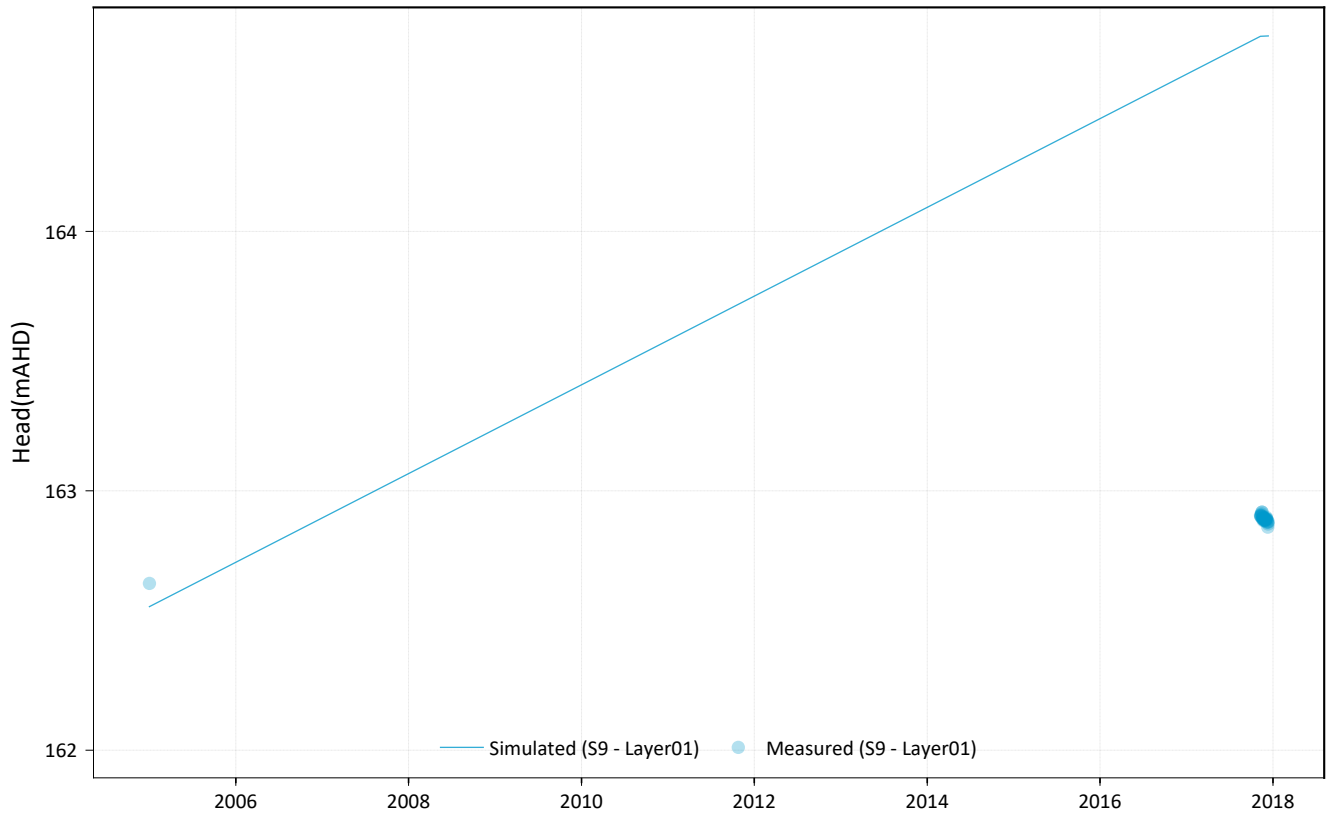
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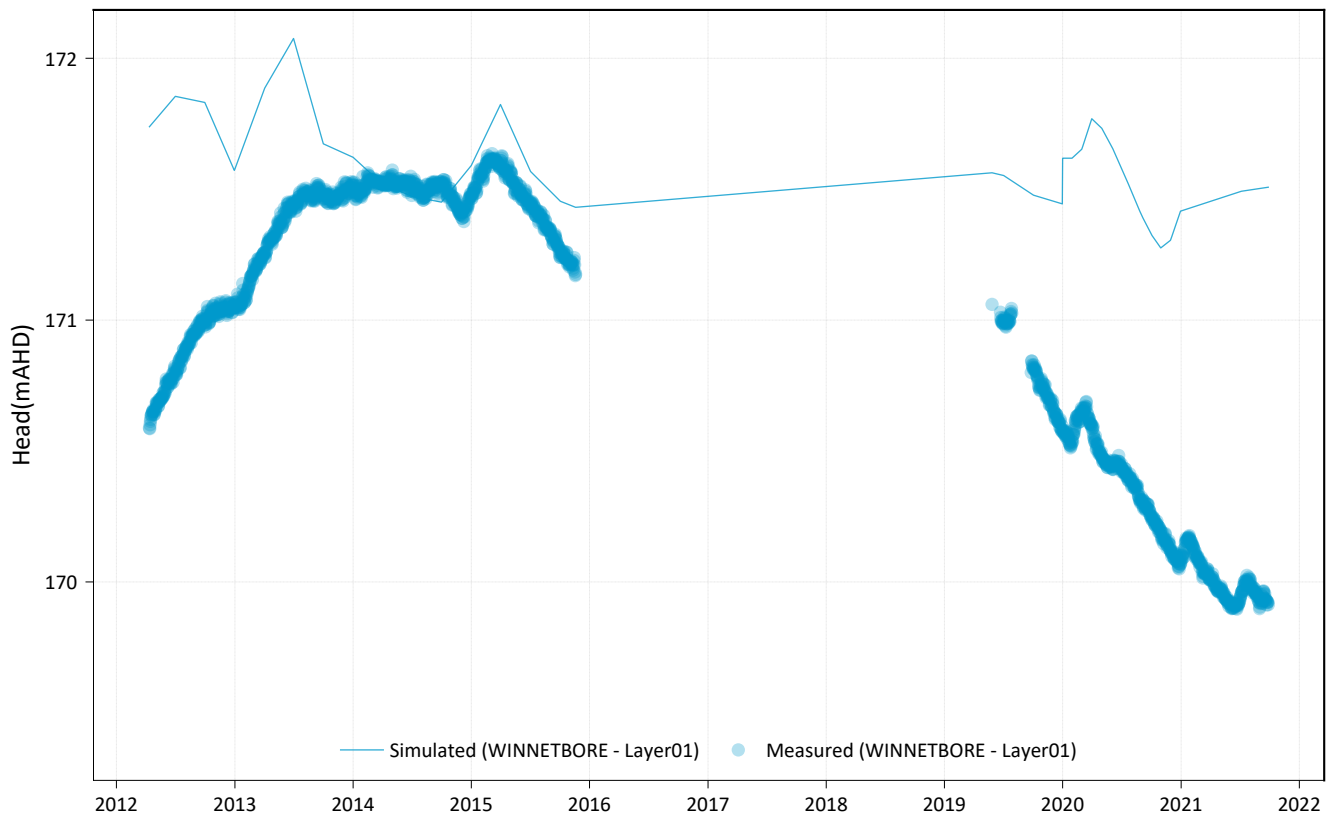
S8



S9



WINNETBORE



APPENDIX B

Calibration Residuals

ID	Easting	Northing	Layer	Average Residual	Min	Max
8	623889	7552264	1	11.98	11.98	11.98
11	627376	7546985	2	11.12	11.12	11.12
13	627376	7546985	2	11.68	11.68	11.68
14	628554	7546783	2	10.24	10.24	10.24
15	629154	7546783	8	11.40	11.40	11.40
16	628105	7543925	12	15.23	15.23	15.23
43639	638941	7511025	14	-6.07	-6.07	-6.07
44161	647523	7540280	1	2.76	2.76	2.76
44164	647850	7541154	2	0.24	0.24	0.24
88525	671081	7522021	12	-13.69	-13.69	-13.69
88526	671663	7519540	12	-11.34	-11.34	-11.34
88527	665218	7516094	1	-6.74	-6.74	-6.74
90074	671595	7510541	12	-13.07	-13.07	-13.07
90076	672457	7515281	12	-6.92	-6.92	-6.92
97180	654739	7527148	1	1.20	1.20	1.20
97181	656380	7524006	1	0.69	0.69	0.69
97182	657183	7522466	1	-2.20	-2.20	-2.20
97183	657433	7522323	1	-2.02	-2.02	-2.02
97184	658993	7519473	1	-6.72	-6.72	-6.72
97185	659218	7519203	1	-6.11	-6.11	-6.11
136090	647458	7540055	2	1.83	1.83	1.83
136689	635855	7528415	2	-4.57	-4.57	-4.57
141653	659102	7555930	2	0.06	0.06	0.06
141654	658962	7555474	2	-1.82	-1.82	-1.82
141657	660625	7555311	2	-7.27	-7.27	-7.27
141659	665671	7557573	2	-7.79	-7.79	-7.79
141660	662598	7556255	2	-3.53	-3.53	-3.53
141661	662120	7553081	2	-6.52	-6.52	-6.52
141662	662940	7553236	2	-5.90	-5.90	-5.90
158010	642725	7520149	9	0.33	0.33	0.33
158011	640268	7514146	12	-4.88	-4.88	-4.88
158484	648156	7524009	2	-0.85	-0.85	-0.85
158485	643151	7522210	9	2.44	2.44	2.44
161572	672769	7538012	14	-6.91	-6.91	-6.91
161573	672769	7538012	12	-6.03	-6.03	-6.03
161575	672285	7543043	13	3.81	3.81	3.81

ID	Easting	Northing	Layer	Average Residual	Min	Max
161578	672399	7535016	14	-4.64	-4.64	-4.64
162043	613392	7560086	2	-0.92	-0.92	-0.92
162044	615620	7560492	2	-7.40	-7.40	-7.40
162045	615691	7559545	9	-8.60	-8.60	-8.60
162046	618287	7557828	10	-4.72	-4.72	-4.72
162048	613232	7556996	2	6.60	6.60	6.60
162137	611430	7557977	13	-2.44	-2.44	-2.44
162138	619718	7547685	12	9.10	9.10	9.10
162141	613823	7562140	1	-0.13	-0.13	-0.13
162143	616091	7561326	2	3.07	3.07	3.07
162163	609581	7560097	13	9.94	3.47	11.55
162165	608928	7556701	2	8.62	2.61	17.08
162166	608928	7556701	13	-0.16	-5.68	1.60
162167	610681	7555334	13	-2.47	-3.23	-1.87
162168	609028	7554103	2	0.61	0.54	0.66
162169	611237	7551944	2	3.49	3.20	4.08
162170	611237	7551944	13	-0.60	-0.63	-0.57
162171	612481	7550690	1	-0.46	-0.53	-0.33
162172	612481	7550690	13	0.81	-0.46	1.86
162175	614292	7548995	13	-8.03	-21.30	-1.01
162177	617002	7547466	12	-0.86	-20.28	7.45
162439	631863	7553638	1	2.05	2.05	2.05
162470	635473	7560503	4	1.54	-1.89	5.78
162471	632372	7558225	6	-5.91	-7.59	3.76
162528	631618	7560959	9	9.42	6.16	11.44
162682	641169	7546542	1	-2.93	-4.10	-2.74
162684	642471	7547492	1	-0.09	-0.33	0.11
165325	640353	7516113	12	-3.82	-3.82	-3.82
13040180	667834	7516281	1	-8.63	-10.15	-6.63
13040181	668043	7516007	1	-5.26	-5.26	-5.26
13040183	668953	7514992	9	-7.20	-7.20	-7.20
13040184	669449	7514426	9	-8.37	-8.37	-8.37
13040287	662008	7558903	1	-4.30	-4.69	-4.05
Bore2	634772	7550077	1	0.81	0.81	0.81
Bore3	634772	7550077	10	-1.04	-1.04	-1.04
Bore7	637495	7552510	9	1.76	1.76	1.76

ID	Easting	Northing	Layer	Average Residual	Min	Max
C2105	634696	7541901	5	-2.38	-4.46	-1.70
C2105R	634646	7541814	5	-3.05	-3.82	-2.51
C2131	630104	7545658	6	7.64	7.64	7.64
C2136	631696	7547270	5	-10.31	-11.25	-10.14
G2300	629734	7544716	6	6.16	6.16	6.16
G2301	631137	7542307	6	16.35	16.35	16.35
G2304	633246	7543200	7	-7.25	-7.25	-7.25
G2304R	633246	7543200	7	-7.14	-7.30	-7.07
G2307	630846	7547876	7	-10.96	-11.19	-10.71
GW01d_p1	642475	7547489	7	-3.30	-4.40	-1.94
GW01d_p2	642475	7547489	5	-6.51	-7.31	-5.11
GW01d_p3	642475	7547489	3	-0.90	-1.31	-0.42
GW01d_p4	642475	7547489	3	1.28	1.05	1.98
GW01s	642471	7547492	1	0.63	0.11	3.40
GW02d	641169	7546542	7	-3.30	-3.58	-2.41
GW02s	641169	7546542	1	-2.66	-3.06	-0.97
GW06d_p1	639334	7542009	11	10.19	9.87	10.59
GW06d_p2	639334	7542009	10	1.14	0.64	1.61
GW06d_p3	639334	7542009	10	0.74	0.53	0.95
GW06d_p4	639334	7542009	9	-3.63	-4.73	-0.67
GW12d_p1	641492	7532790	5	2.65	1.59	7.23
GW12d_p2	641492	7532790	5	-2.19	-3.01	-0.78
GW12s	641498	7532791	2	-1.23	-1.27	-1.05
GW16d_p2	660834	7525288	5	-9.66	-9.79	-9.54
GW16d_p4	660834	7525288	3	-8.60	-8.77	-8.42
GW18d	656891	7522810	7	-0.18	-0.35	0.53
GW18s	656885	7522811	1	0.29	0.15	1.20
GW21d	661580	7521648	8	-15.68	-15.70	-15.64
GW21s	661580	7521653	2	5.05	4.88	5.17
GW8s	645324	7539848	1	-0.75	-1.27	0.63
KnobHill1	631036	7553867	1	1.51	-1.72	2.03
KnobHill2	630441	7554066	1	4.22	3.33	5.96
LakeV3	648029	7523934	2	7.14	7.14	7.14
LH13	627376	7546985	2	13.72	11.49	15.64
LV1235C_P1	649873	7522139	8	-15.56	-16.57	-15.04
LV1235C_P2	649873	7522139	8	-14.97	-17.39	-14.04

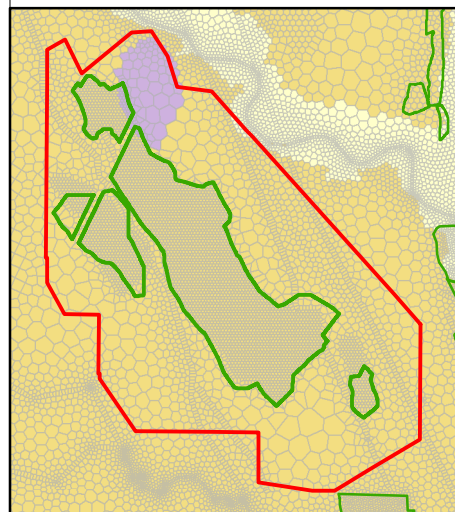
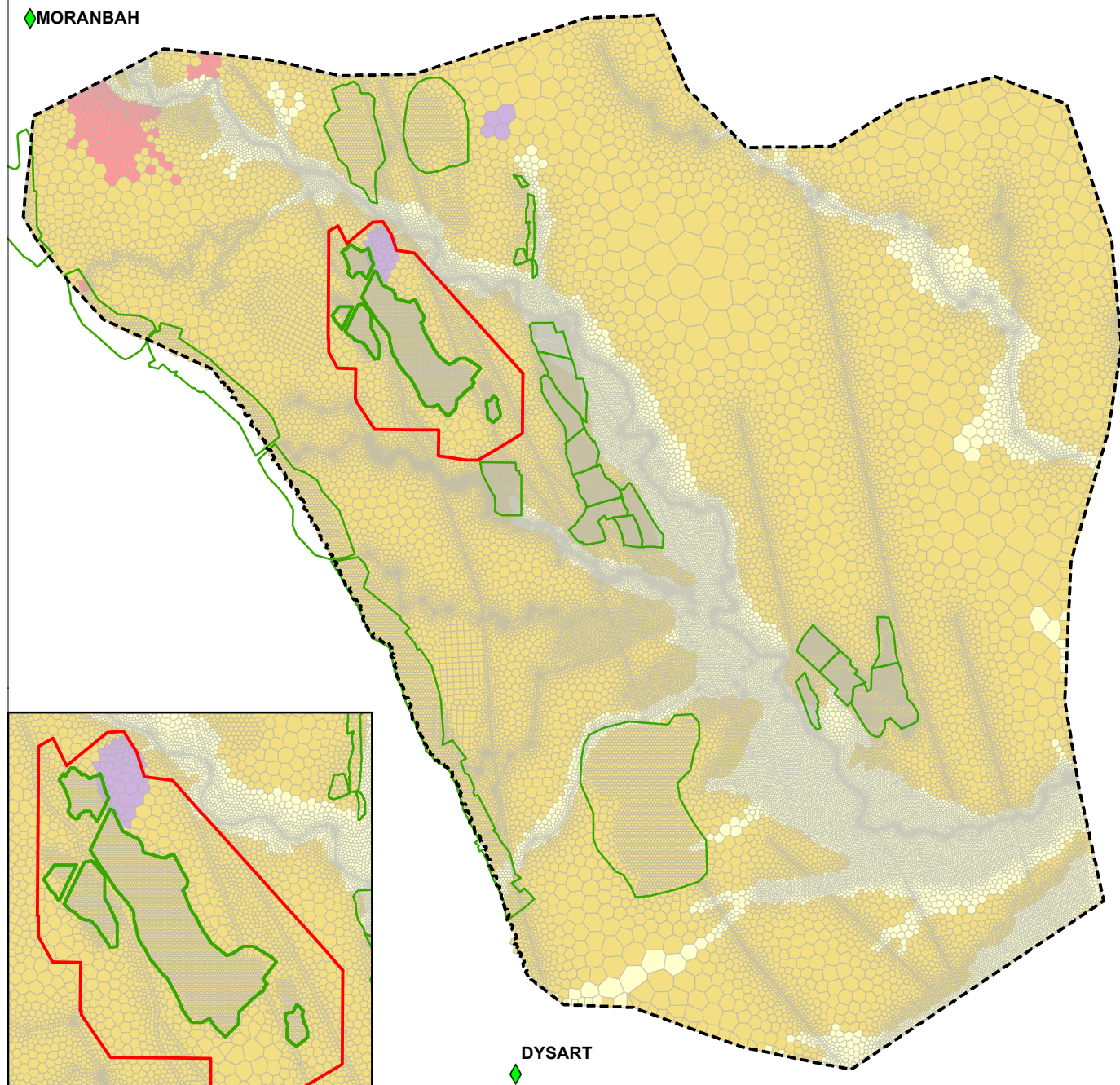
ID	Easting	Northing	Layer	Average Residual	Min	Max
LV1235C_P3	649873	7522139	8	-14.22	-17.59	-13.04
LV1235C_P4	649873	7522139	2	-10.30	-12.56	-8.05
LV2183_P1	644173	7520580	7	-16.65	-17.40	-16.39
LV2183_P3	644173	7520580	6	-6.22	-6.98	-5.96
LV2183_P4	644173	7520580	2	-16.00	-16.06	-15.86
LV2218_P1	645739	7522967	7	-9.19	-9.23	-9.11
LV2218_P2	645739	7522967	6	-9.21	-9.26	-9.13
LV2218_P3	645739	7522967	6	-7.99	-8.26	-7.23
LV2218_P4	645739	7522967	3	-3.76	-4.78	-2.75
LV2226_P1	643151	7522210	7	-5.21	-5.55	-5.01
LV2226_P2	643151	7522210	6	-4.43	-5.05	-4.02
LV2226_P3	643151	7522210	6	-2.68	-4.05	-2.02
LV2226_P4	643151	7522210	2	3.49	2.44	5.62
LV2370W	648156	7524009	2	0.55	-1.34	16.30
LV2372R_P1	647603	7526258	9	-8.17	-8.55	-8.05
LV2372R_P2	647603	7526258	9	-8.17	-8.55	-8.05
LV2372R_P3	647603	7526258	9	-8.17	-8.55	-8.05
LV2372R_P4	647603	7526258	2	-8.95	-9.33	-8.82
LV2375W_P1	648156	7524009	9	-12.96	-13.34	-12.83
LV2375W_P2	648156	7524009	9	-12.84	-12.84	-12.83
LV2375W_P3	648156	7524009	9	-8.59	-9.34	-8.33
MB1	623443	7551730	13	-0.52	-2.56	1.92
MB2	623738	7549293	13	6.22	4.78	6.70
MB4	626410	7544101	13	13.97	13.35	14.67
MB5	628453	7542621	13	13.07	0.08	13.86
ODN18MB1	640261	7547974	5	-3.63	-3.79	-3.50
ODN18MB10	639389	7554580	9	-7.07	-7.07	-7.07
ODN18MB11	638569	7553362	9	0.01	0.01	0.01
ODN18MB12	640261	7547974	7	3.00	2.69	3.26
ODN18MB2	640261	7547974	1	6.66	6.62	6.73
ODN18MB3	639664	7551385	5	-7.82	-7.83	-7.81
ODN18MB4	640683	7549886	1	-6.16	-6.45	-5.59
ODN18MB6	639988	7551780	5	-6.81	-6.81	-6.81
ODN18MB7	640421	7554763	1	-2.53	-2.53	-2.53
ODN18MB8	638936	7550145	1	1.16	1.09	1.26
ODN18MB9	640029	7557122	5	1.57	1.57	1.57

ID	Easting	Northing	Layer	Average Residual	Min	Max
ODN18TB1	640332	7547946	5	-2.67	-2.87	-2.32
ODN18TB2	640332	7547946	1	8.73	8.54	8.97
ODN18VWP1	640261	7547974	7	-6.16	-6.36	-5.97
ODN18VWP2	640261	7547974	5	-6.15	-6.24	-6.06
ODN18VWP3	640261	7547974	4	-4.81	-5.07	-4.55
R2007	630451	7542573	7	-2.27	-2.27	-2.27
R2008	630887	7542597	7	-0.29	-1.05	0.14
R2009	631339	7542834	6	4.88	4.59	5.26
R2009R	631339	7542834	6	-39.44	-6.90	-5.16
R2010	631760	7543062	7	-5.44	-5.47	-5.04
R2010R	631760	7543062	5	-5.17	6.81	6.81
R2030	630034	7545019	6	6.81	4.40	8.18
R2032	630526	7545776	5	7.18	3.20	4.94
R2034	629654	7545398	8	3.94	-2.06	-1.20
R2034R	629654	7545398	8	1.10	-0.84	5.40
R2035	629288	7544983	7	-1.92	4.39	6.05
R2054	629204	7548082	6	4.54	9.00	9.00
R2055	628804	7547909	7	5.50	-5.00	-5.00
R2056	628296	7547568	8	9.00	0.28	2.13
River_Bore	654031	7526962	3	-5.00	3.04	3.04
S10	642561	7546017	1	0.37	1.66	1.66
S11	642463	7545335	1	3.04	-5.00	0.05
S2	641380	7547642	1	1.66	-2.20	1.51
S4	641569	7546889	1	-3.31	-0.66	1.96
S5	642156	7547341	1	-2.07	-3.28	-0.88
S6	642069	7546715	1	-0.57	1.19	3.57
S7	641408	7545845	1	-3.16	-1.89	0.09
S8	642345	7546383	1	1.47	-6.84	-6.84
S9	641798	7545435	1	-1.80	0.14	0.14
Swamp_Bore	645609	7528626	9	-6.84	-2.55	-2.55
Unknown1	670375	7516365	1	0.14	-9.37	-9.37
Unknown1_9	656902	7516004	9	-2.55	-1.60	0.08
Unknown2	656902	7516004	2	-9.37	-4.03	-4.03
WinnetBore	634772	7550077	1	-0.65	11.98	11.98
YardBore1	642564	7519421	11	-4.03	11.12	11.12

APPENDIX C

Hydraulic Zone Distributions

H:\Projects\SLR\620-BNE\620-BNE\020-13245 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-1 Hydraulic Zone Distribution (Layer 1).mxd



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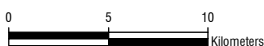
- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh
- Parameter Zone**
- Alluvium
- Colluvium
- Tertiary basalt
- Intrusives

Scale:1:380,000
GDA 1994 MGA Zone 55



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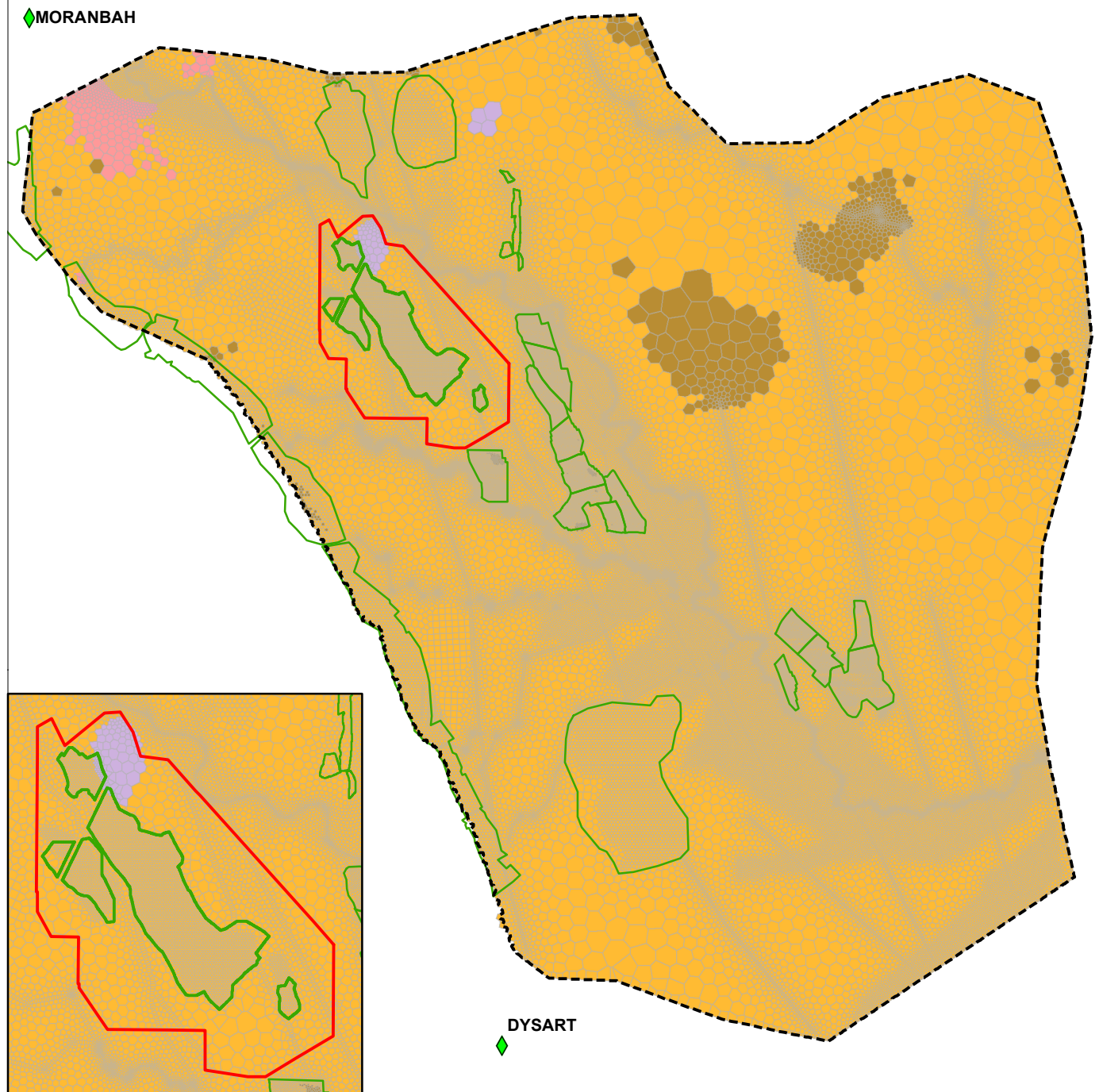
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Hydraulic Parameter Zone Distribution Layer 1

FIGURE C-1

H:\Projects\SLR\620-BNE\620-BNE\620-13245 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-2 Hydraulic Zone Distribution (Layer 2).mxd



LEGEND

- Project Mining Lease Boundary
 - Mine Outline
 - Model Boundary
 - Model Mesh
- Parameter Zone**
- Regolith (< 65 mbgl)
 - Regolith (> 65 mbgl)
 - Tertiary basalt
 - Intrusives

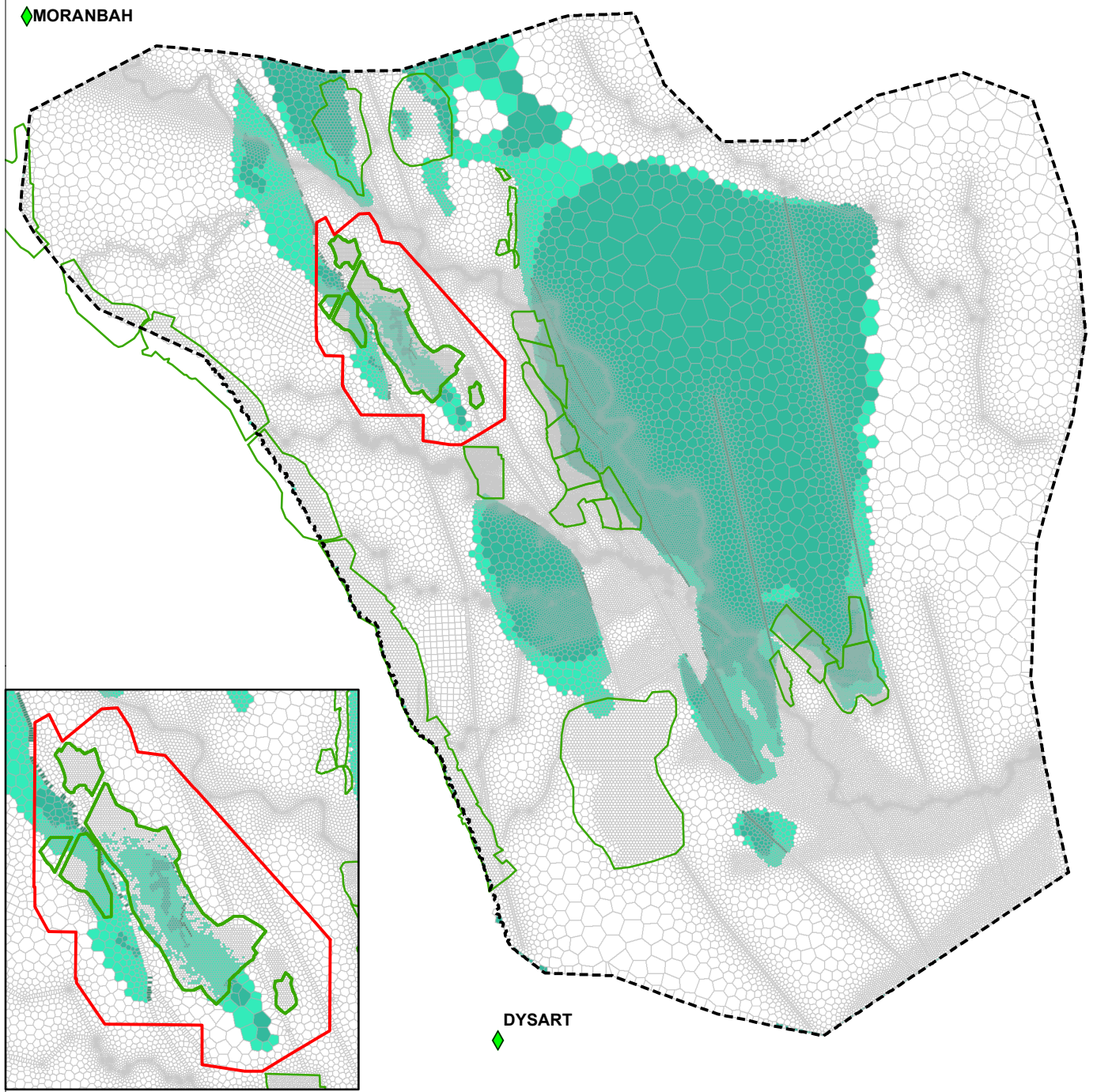
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GDA 1994 MGA Zone 55



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LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

- Rewan Group (< 65 mbgl)
- Rewan Group (> 65 mbgl)
- Rewan Group Fault

Scale: 1:380,000
GDA 1994 MGA Zone 55



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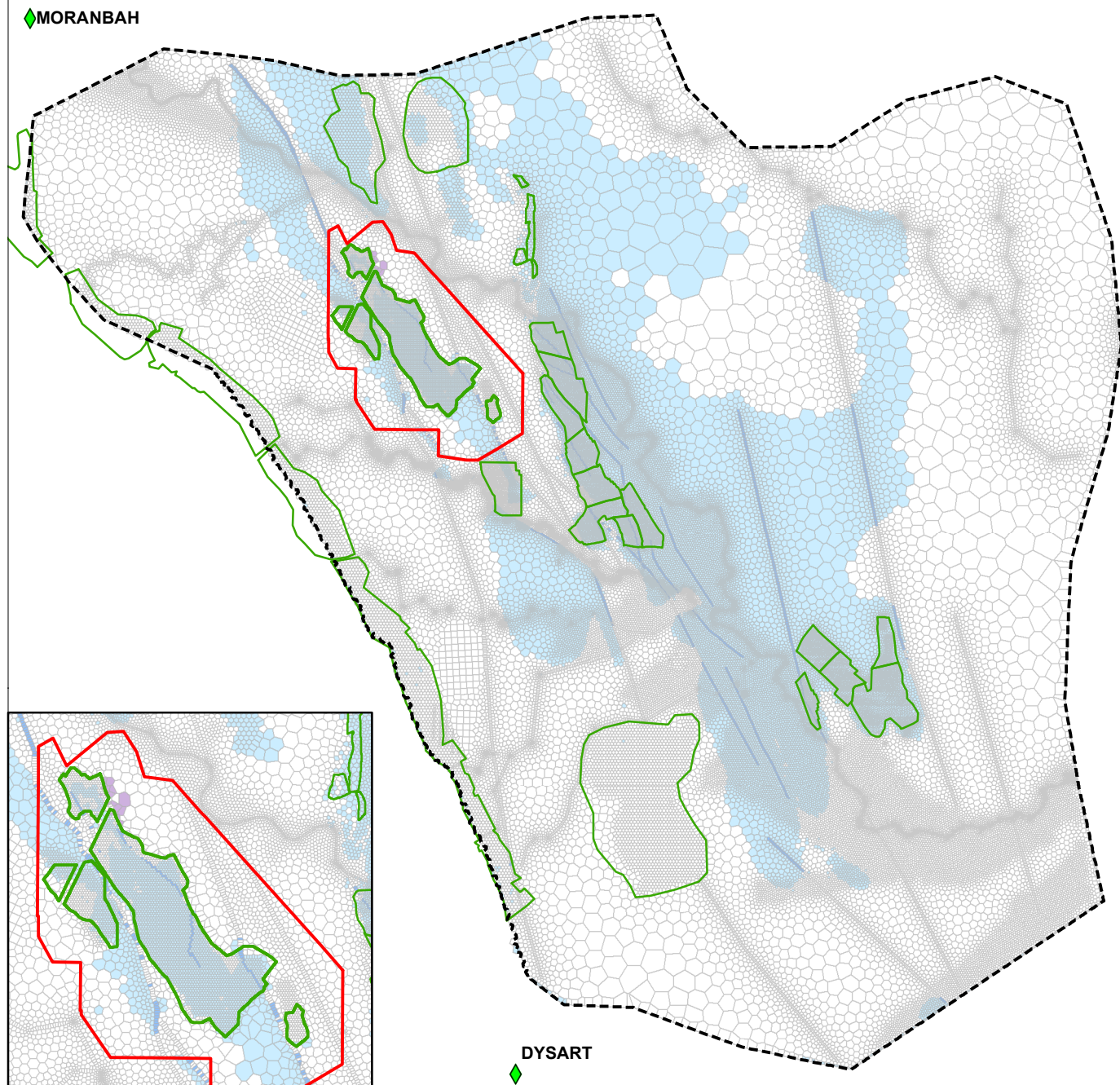
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**Hydraulic Parameter
Zone Distribution
Layer 3**

FIGURE C-3

H:\Projects\SLR\620-BNE\620-BNE\020-13-245 Winchester South Groundwater\GIS\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-4 Hydraulic Zone Distribution (Layer 4).mxd



LEGEND

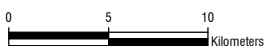
- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh
- Parameter Zone**
- RCM O/B
- RCM O/B Fault
- Intrusives

Scale:1:380,000
GDA 1994 MGA Zone 55



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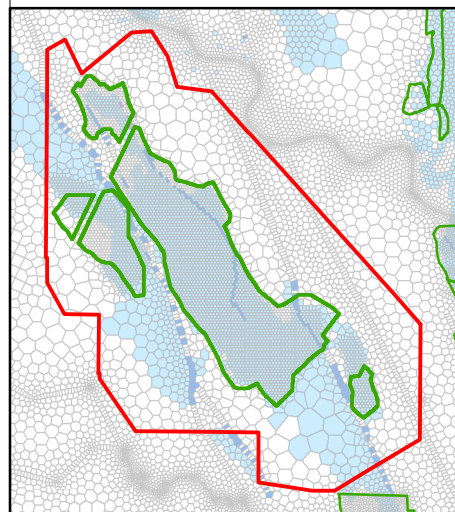
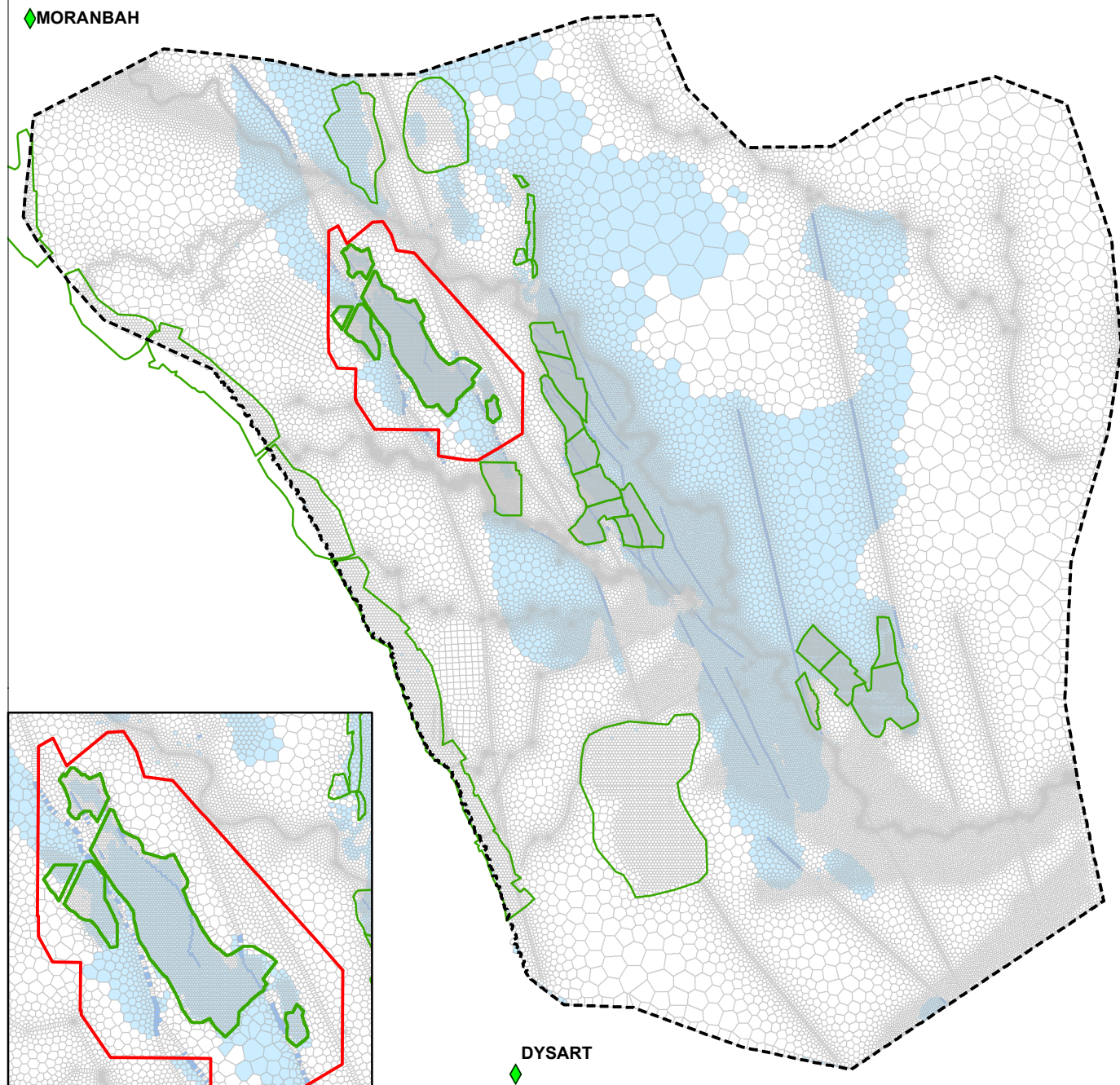
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Hydraulic Parameter Zone Distribution Layer 4

FIGURE C-4

H:\Projects\SLR\620-BNE\620-BNE\02-13-246 Winchester South Groundwater\GIS\Modelling\Report_Figures\AppendixK_Zones\62013245 C-5 Hydraulic Zone Distribution (Layer 5).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

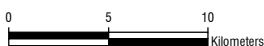
- Leichhardt Seam
- Leichhardt Seam Fault
- Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55



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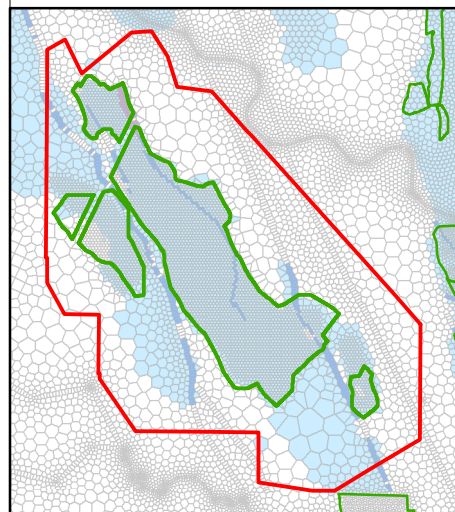
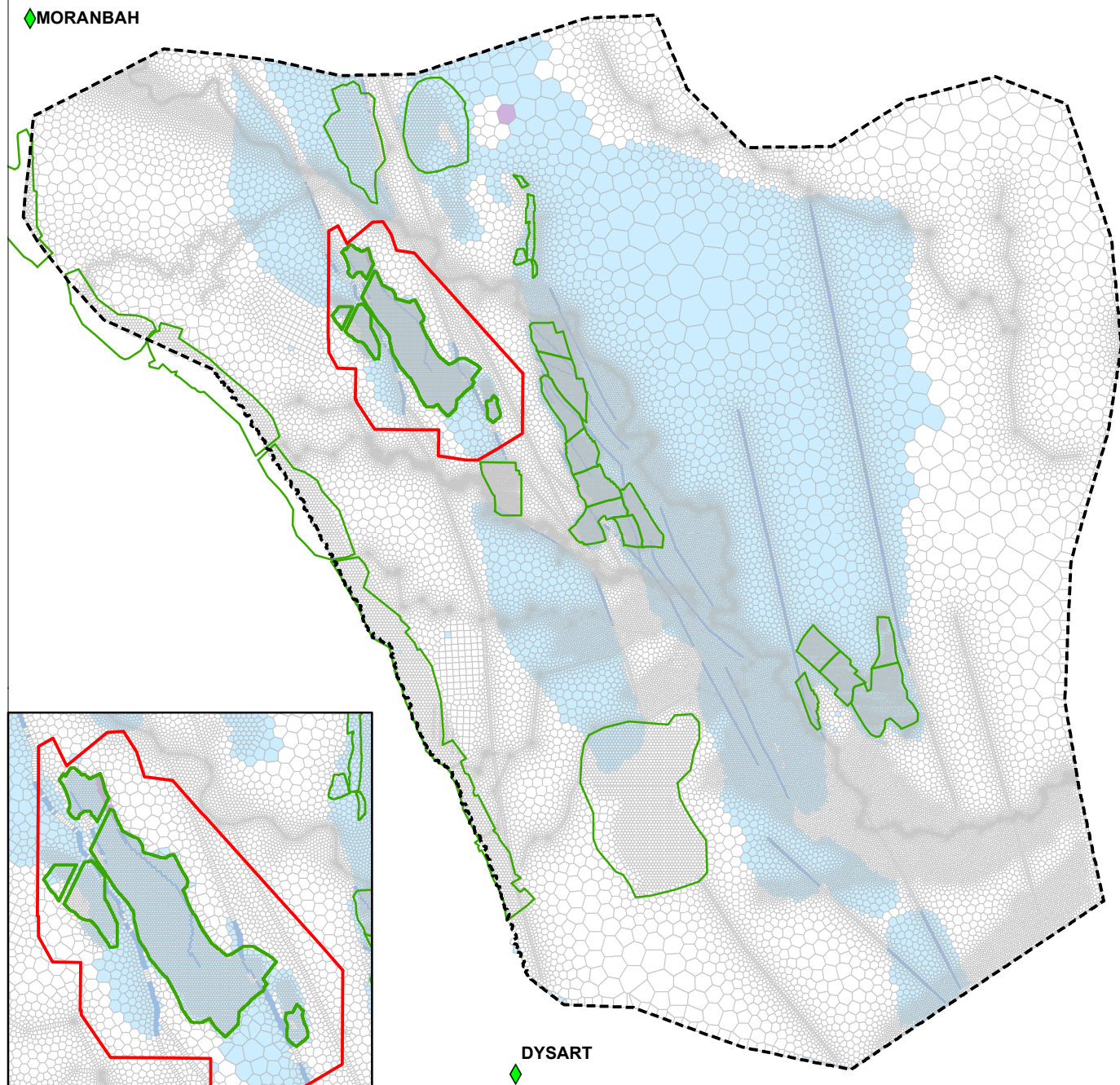
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Hydraulic Parameter Zone Distribution Layer 5

FIGURE C-5

H:\Projects\SLR\620-BNE\620-BNE\020-13245 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-6 Hydraulic Zone Distribution (Layer 6).mxd



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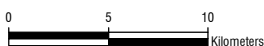
- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh
- Parameter Zone**
- RCM I/B
- RCM I/B Fault
- Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55



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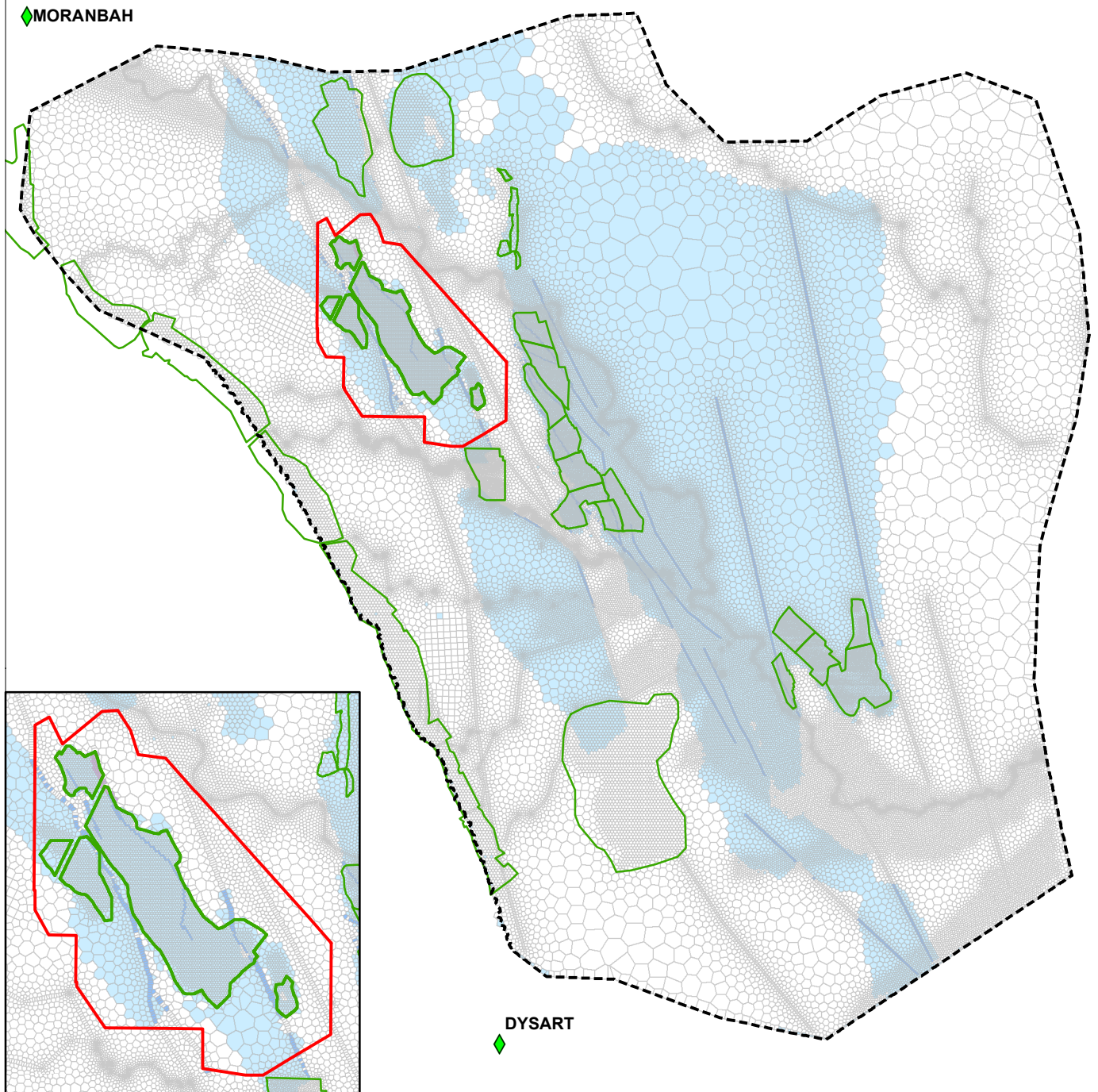
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Hydraulic Parameter Zone Distribution Layer 6

FIGURE C-6

H:\Projects\SLR\620-BNE\620-BNE\02-13-246 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling Report\Figures\AppendixK_Zones\62013245 C-7 Hydraulic Zone Distribution (Layer 7).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

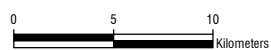
Parameter Zone

- Vermont Seam
- Vermont Seam Fault
- Intrusives

Scale:1:380,000
GDA 1994 MGA Zone 55

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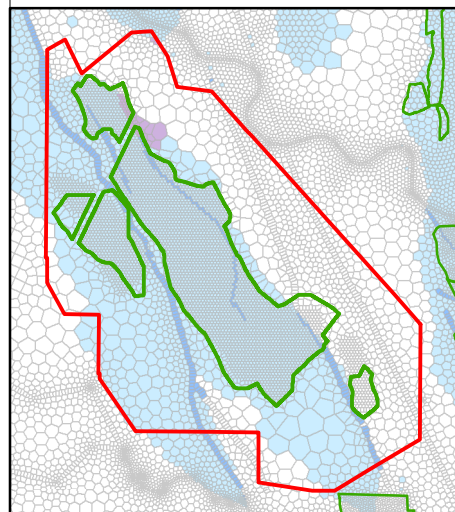
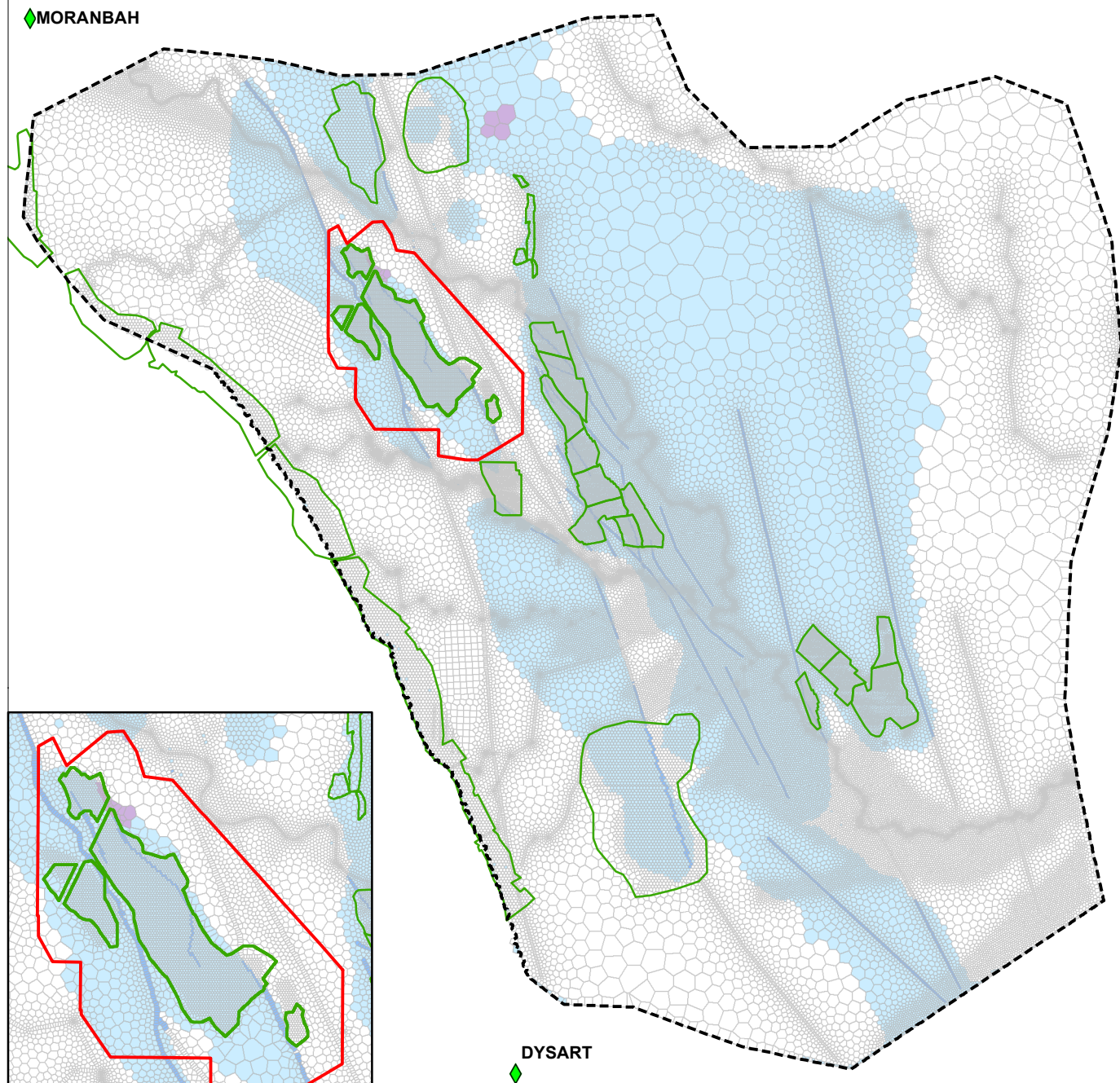
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**Hydraulic Parameter
Zone Distribution
Layer 7**

FIGURE C-7

H:\Projects\SLR\620-BNE\620-BNE\020-13-245 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling Report\Figures\AppendixK_Zones\62013245 C-8 Hydraulic Zone Distribution (Layer 8).mxd



LEGEND

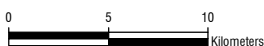
- Project Mining Lease Boundary
 - Mine Outline
 - Model Boundary
 - Model Mesh
- Parameter Zone**
- RCM U/B
 - RCM U/B Fault
 - Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55



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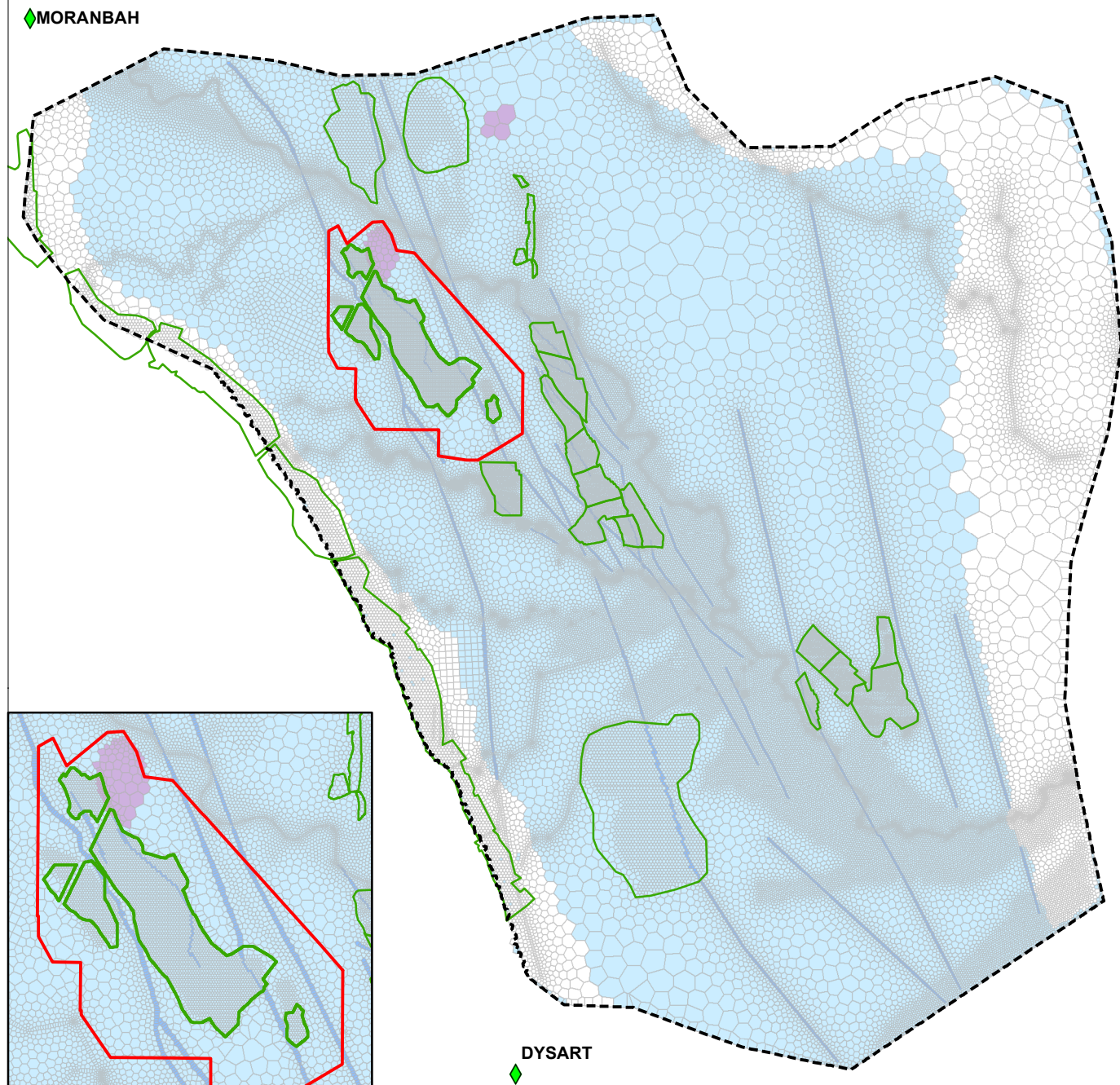
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Hydraulic Parameter Zone Distribution Layer 8

FIGURE C-8

H:\Projects\SLR\620-BNE\620-BNE\020-13245 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-9 Hydraulic Zone Distribution (Layer 9).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

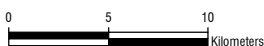
- FCCM O/B
- FCCM O/B Fault
- Intrusives

Scale:1:380,000
GDA 1994 MGA Zone 55



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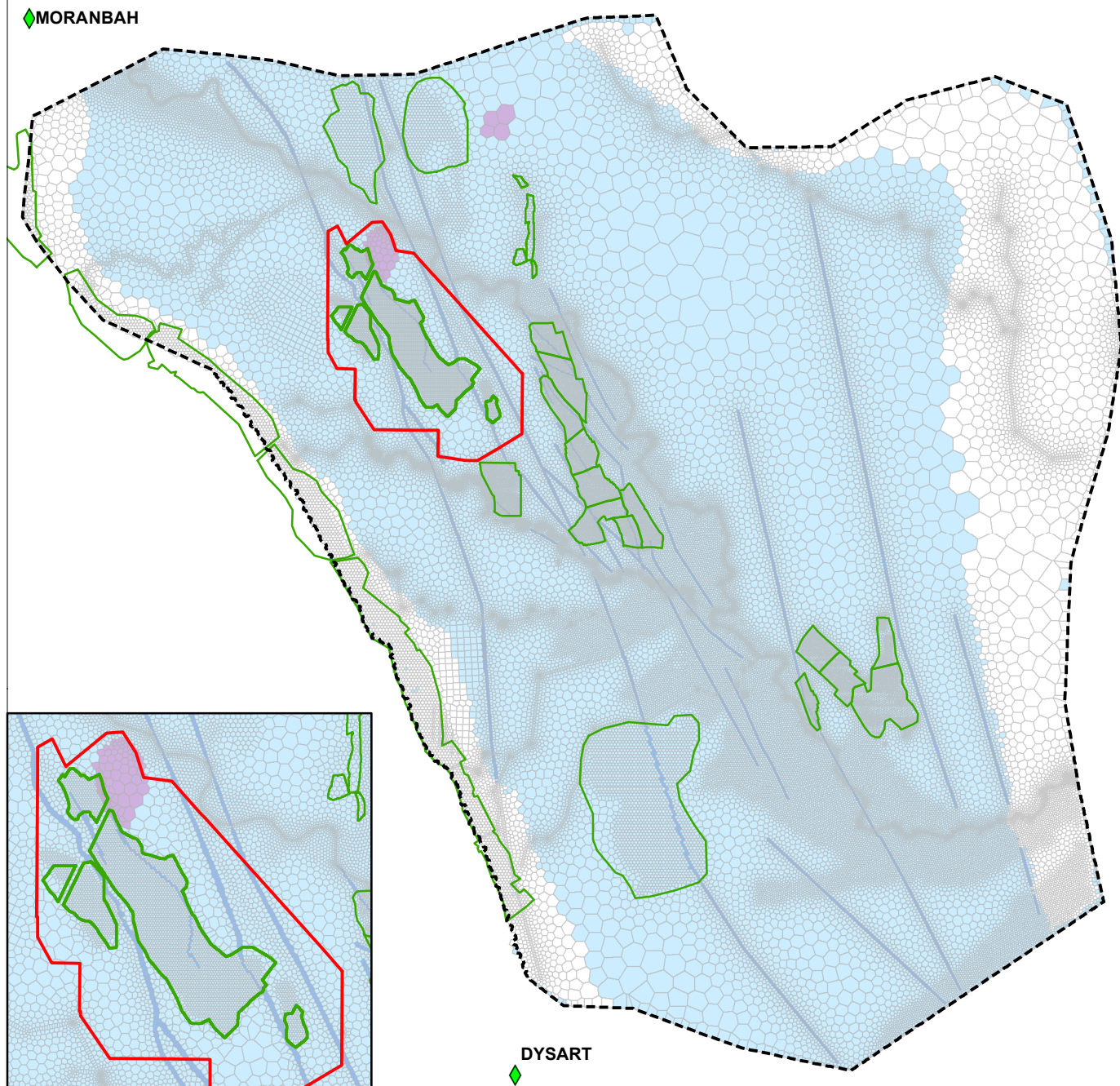
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**Hydraulic Parameter
Zone Distribution
Layer 9**

FIGURE C-9

H:\Projects\SLR\620-BNE\620-BNE\13-246 Winchester South Groundwater\SLR Data\GIS\ArcGIS\03 Modelling\Report_Figures\AppendixK_Zones\62013245 C-10 Hydraulic Zone Distribution (Layer 10).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

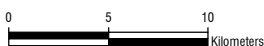
- FCCM Coal Seam
- FCCM Coal Seam Fault
- Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55



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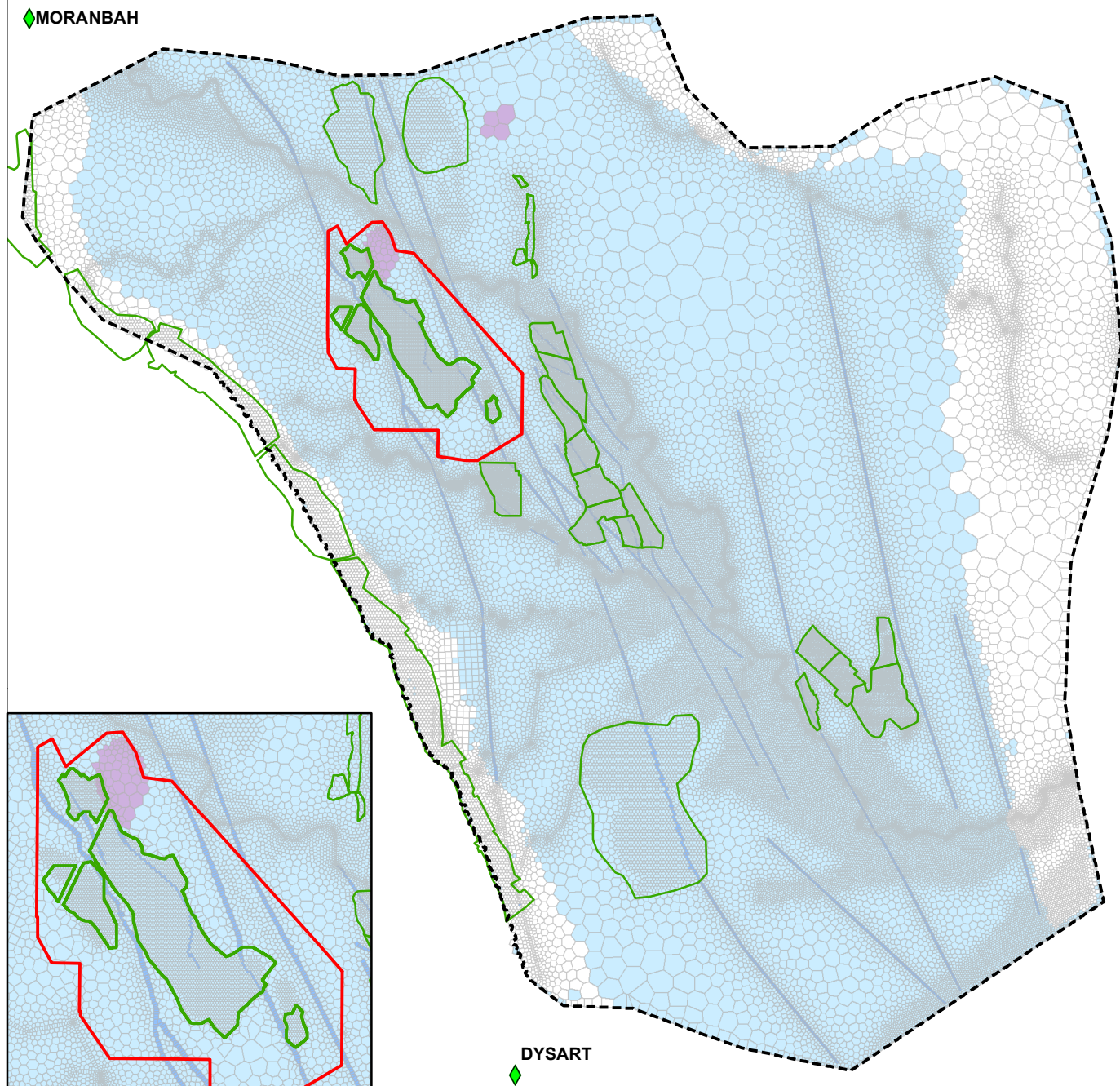
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Hydraulic Parameter Zone Distribution Layer 10

FIGURE C-10

H:\Projects\SLR\620-BNE\620-BNE\13-246 Winchester South Groundwater\GIS\Modelling\Report_Figures\AppendixK_Zones\62013245 C-11 Hydraulic Zone Distribution (Layer 11).mxd



LEGEND

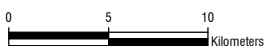
- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh
- Parameter Zone**
 - FCCM U/B
 - FCCM U/B Fault
 - Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55

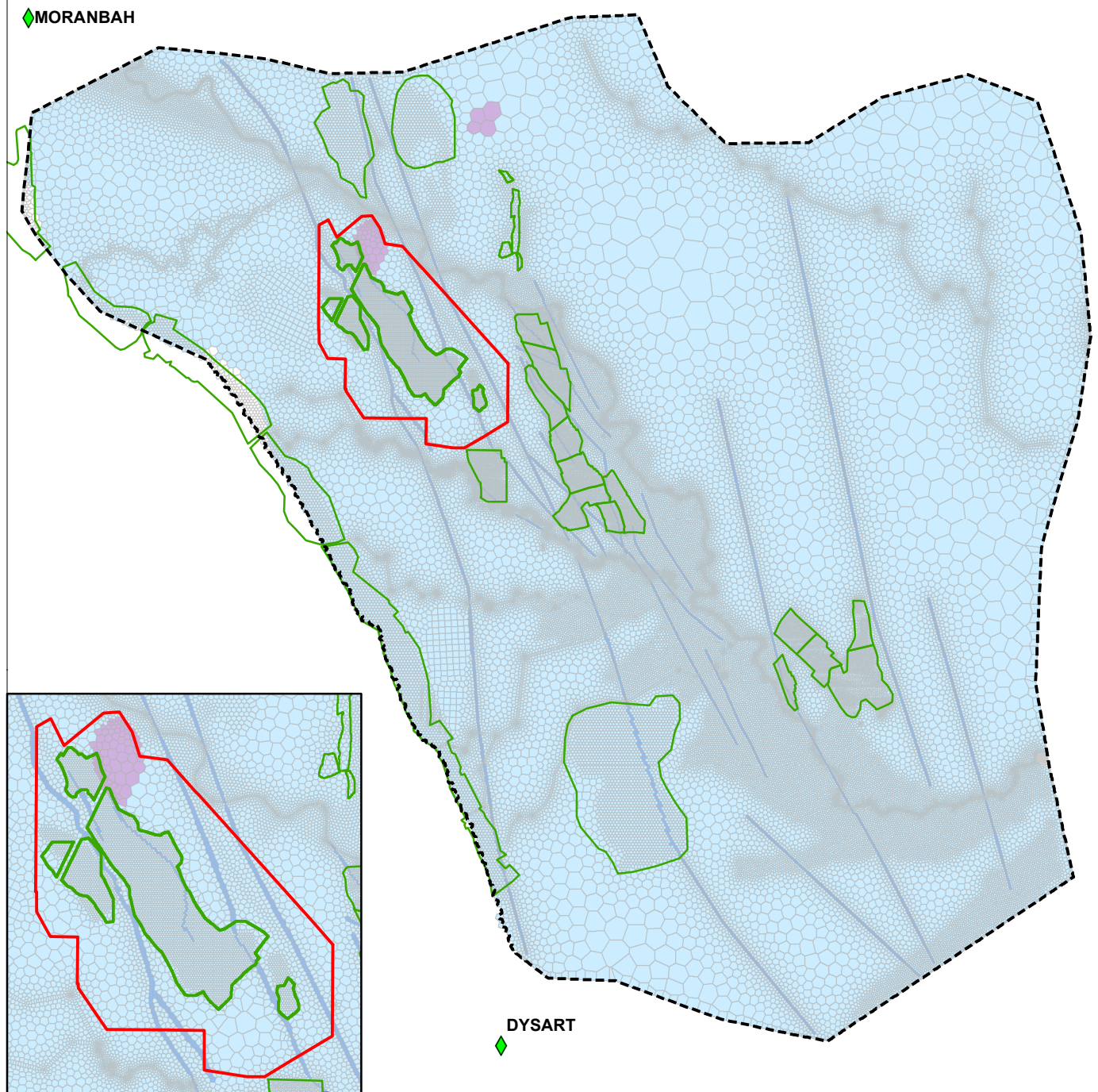


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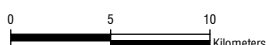
LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh
- Parameter Zone**
- MCM O/B
- MCM O/B Fault
- Intrusives

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GDA 1994 MGA Zone 55

08-Oct-2020
620.13245

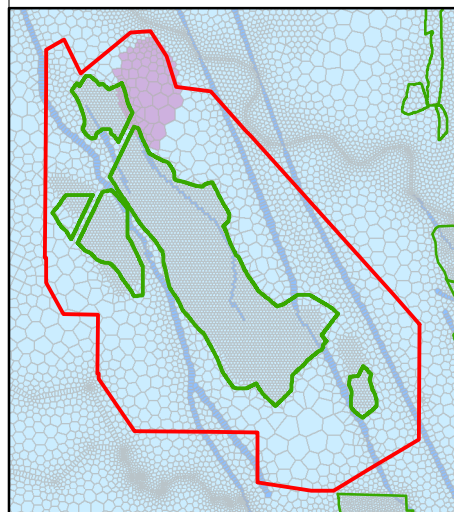
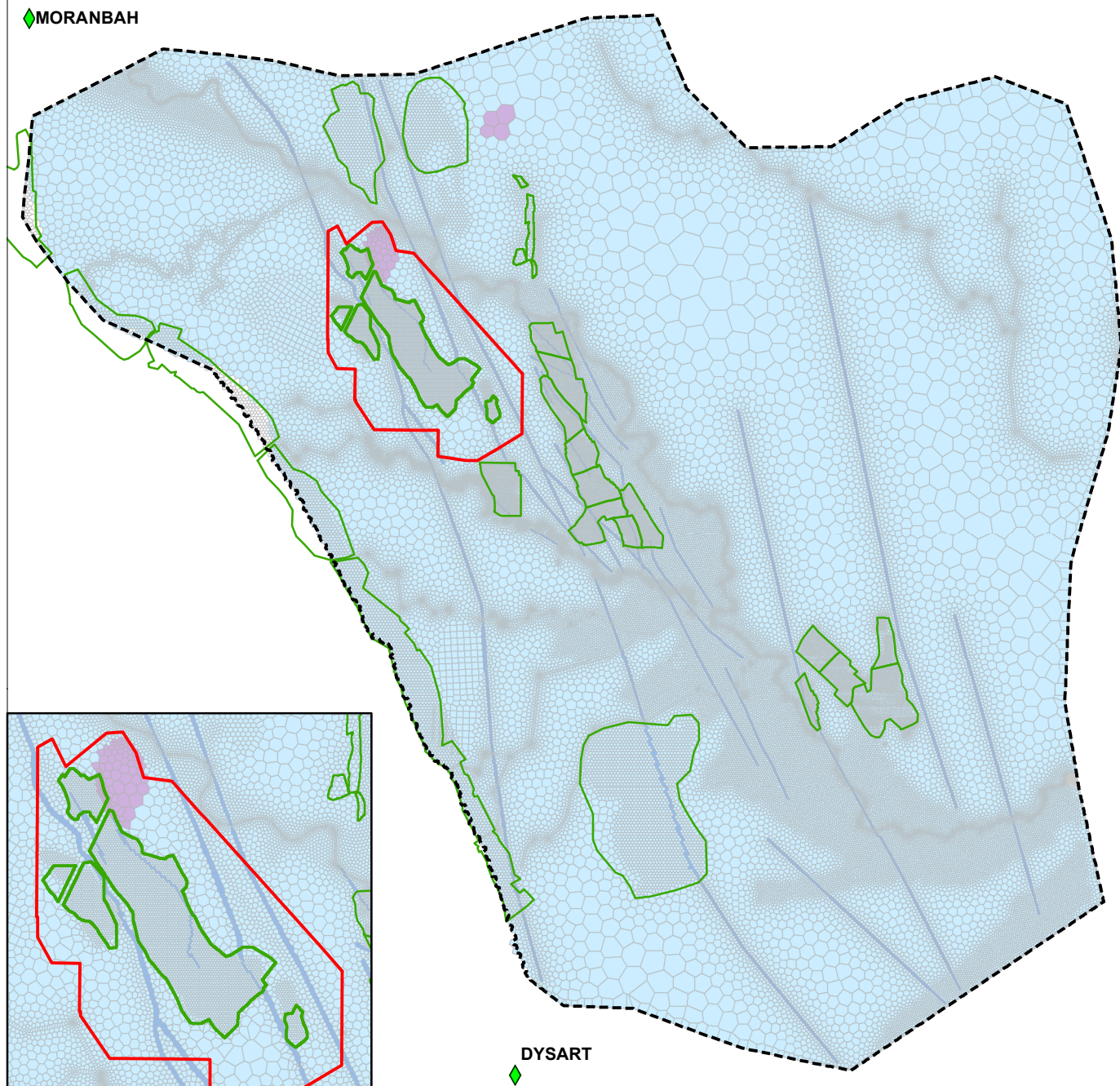
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Hydraulic Parameter Zone Distribution Layer 12

FIGURE C-12

H:\Projects\SLR\620-BNE\620-BNE\020-13245 Winchester South Groundwater\GIS\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-13 Hydraulic Zone Distribution (Layer 13).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

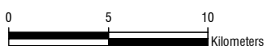
- MCM Coal Seam
- MCM Coal Seam Fault
- Intrusives

Scale: 1:380,000
GDA 1994 MGA Zone 55



08-Oct-2020
620.13245

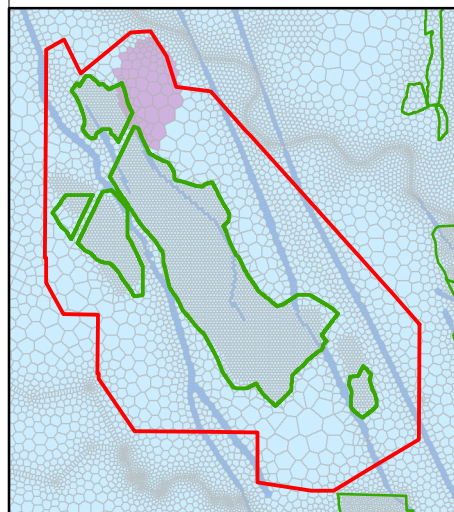
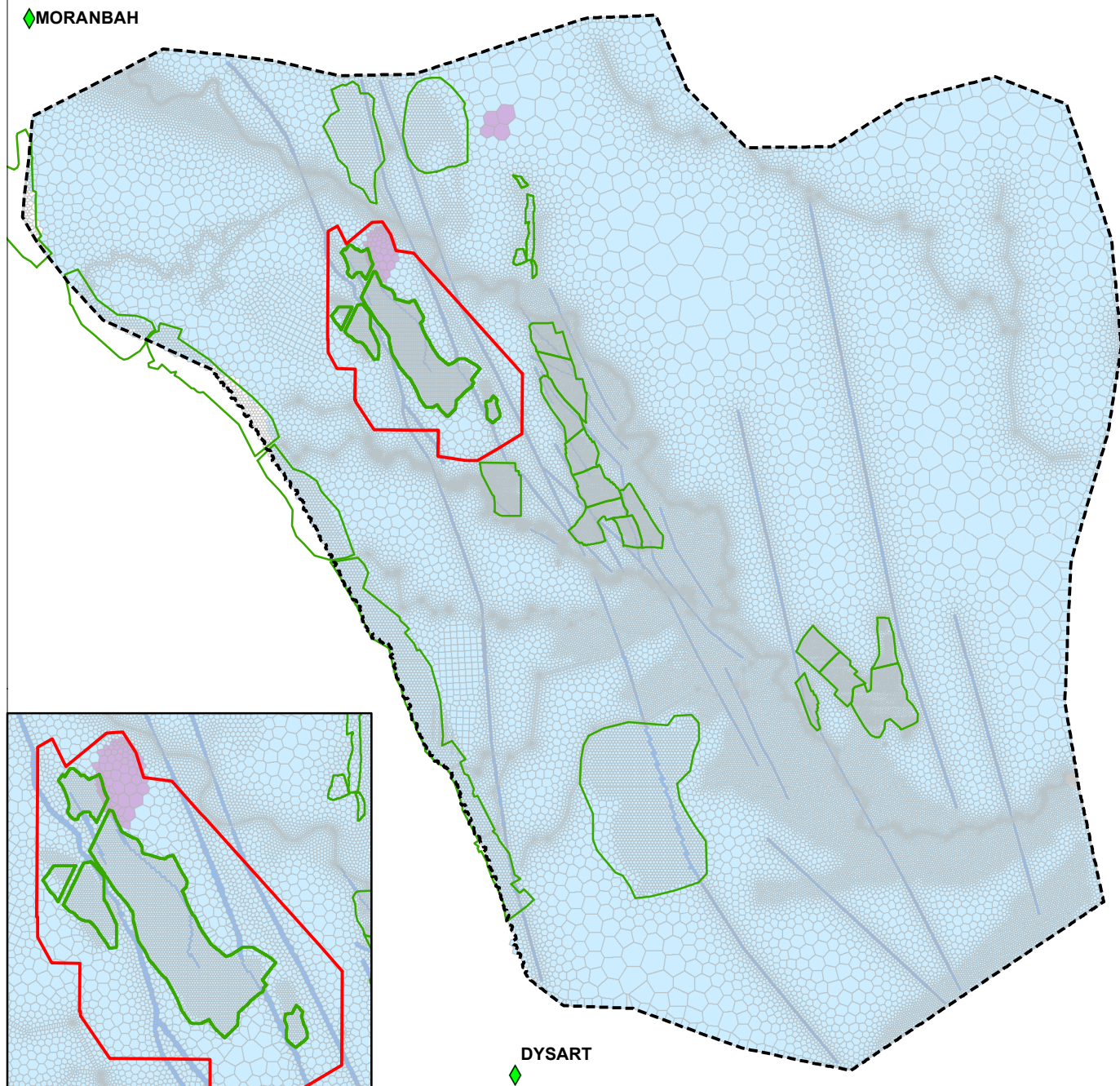
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Hydraulic Parameter Zone Distribution Layer 13

FIGURE C-13

H:\Projects\SLR\620-BNE\620-BNE\020-13245 Winchester South Groundwater\GIS\SLR Data\GIS\ArcGIS\03 Modelling\Report Figures\AppendixK_Zones\62013245 C-14 Hydraulic Zone Distribution (Layer 14).mxd



LEGEND

- Project Mining Lease Boundary
- Mine Outline
- Model Boundary
- Model Mesh

Parameter Zone

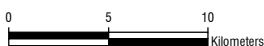
- MCM U/B
- MCM U/B Fault
- Intrusives

Scale:1:380,000
GDA 1994 MGA Zone 55



08-Oct-2020
620.13245

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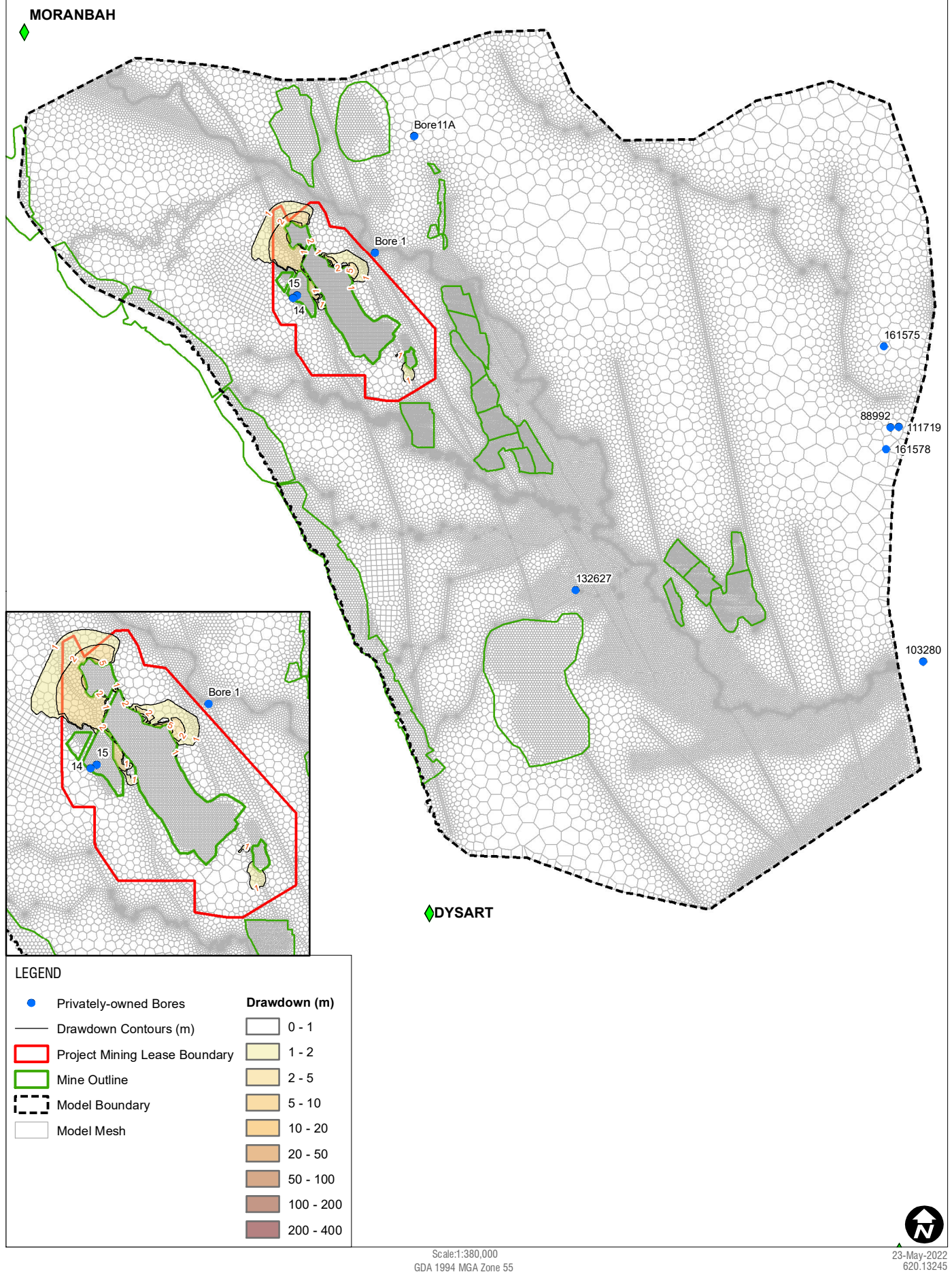


**Hydraulic Parameter
Zone Distribution
Layer 14**

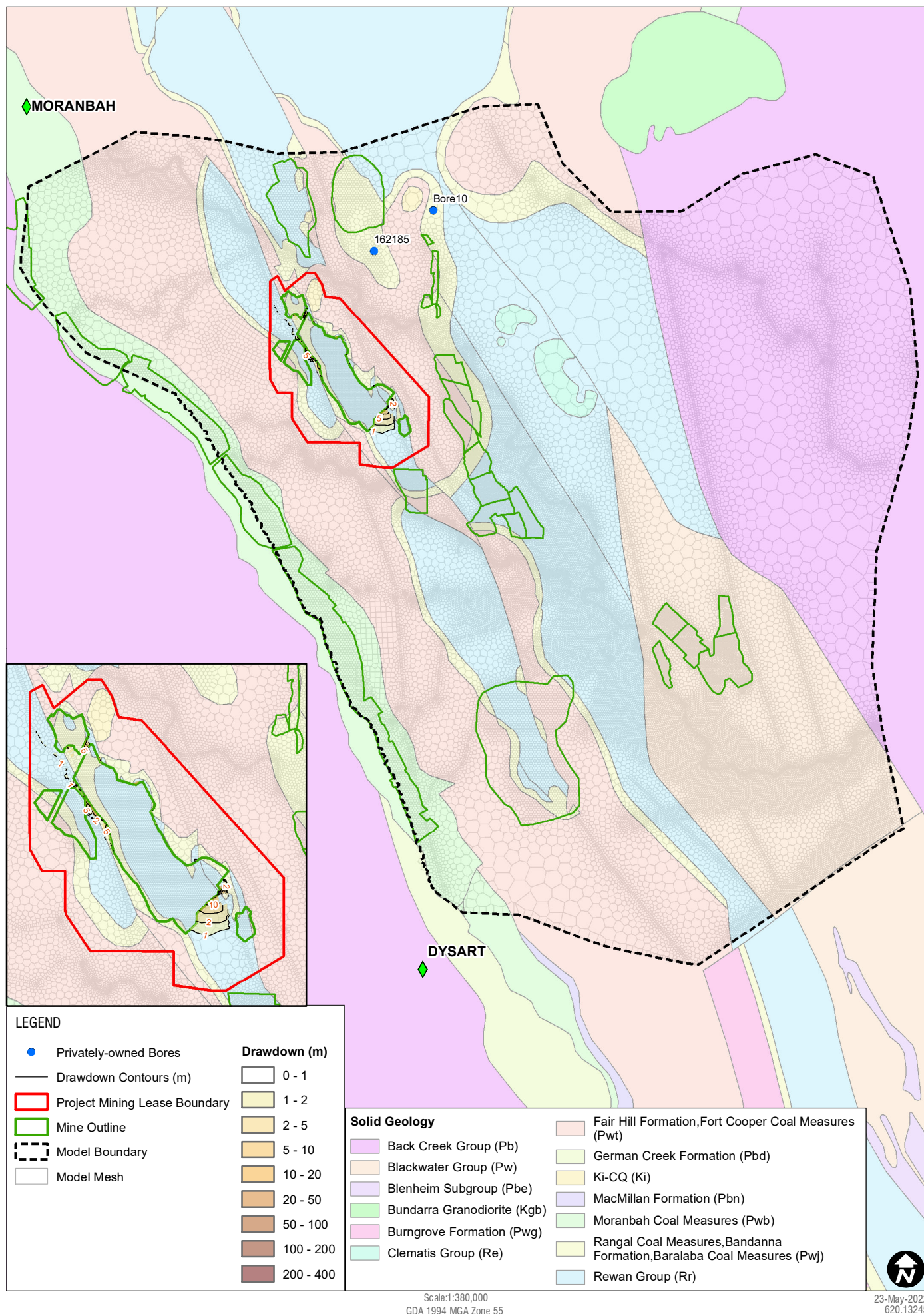
FIGURE C-14

APPENDIX D

Drawdown Progression over Life of the Project



H:\Projects-SLR\620-BNE\620-13-245 Winchester South Groundwater\05 SLR Data\01 CAD\GIS\ArcGIS\03 Modelling\Report Figures\Appendices\Drawdowns\62013245 D-4 Incremental Drawdown in Leichhardt Seam (Layer 5) 2033.mxd



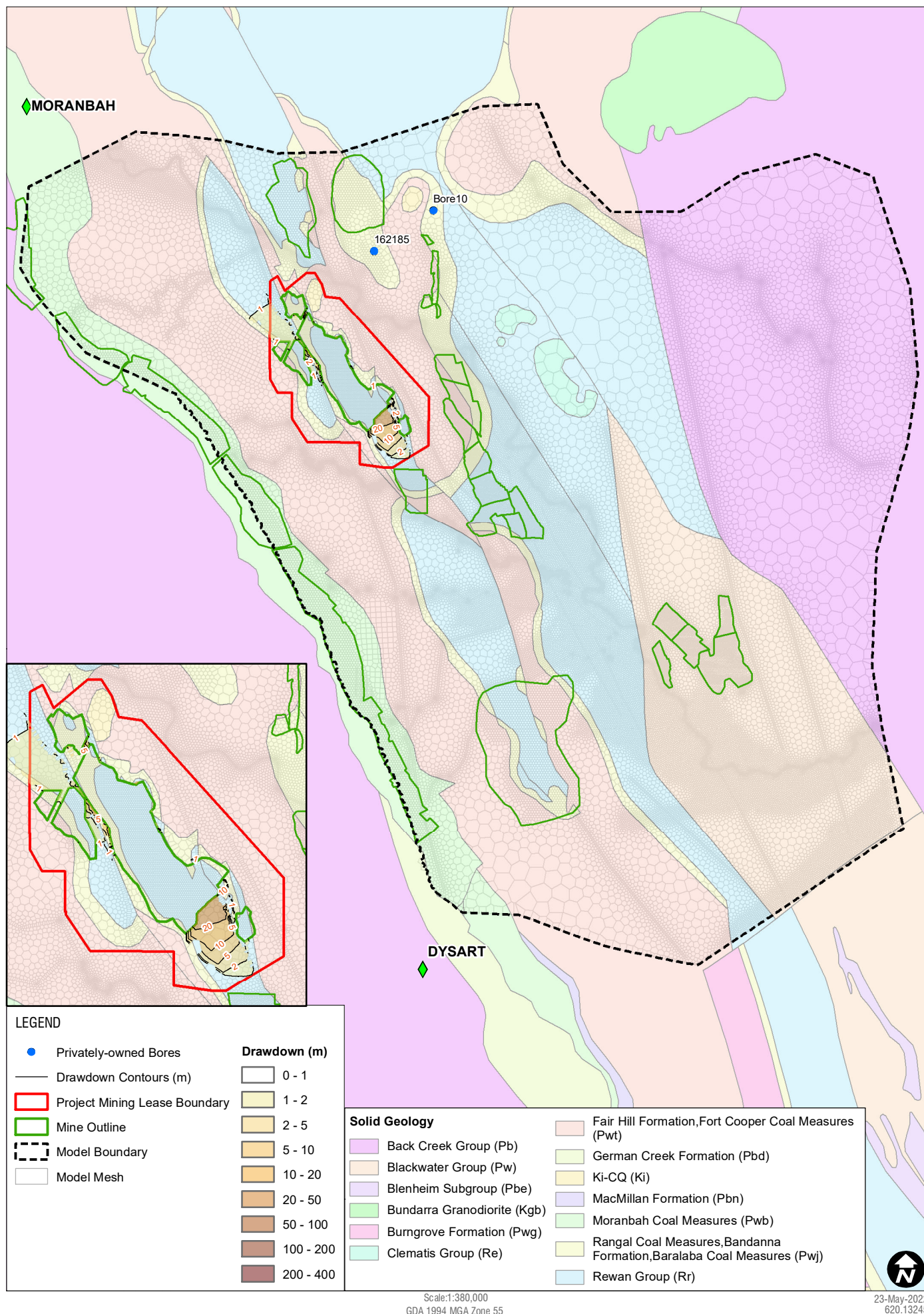
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**Incremental Drawdown in
Leichhardt Seam (Layer 5) 2033**

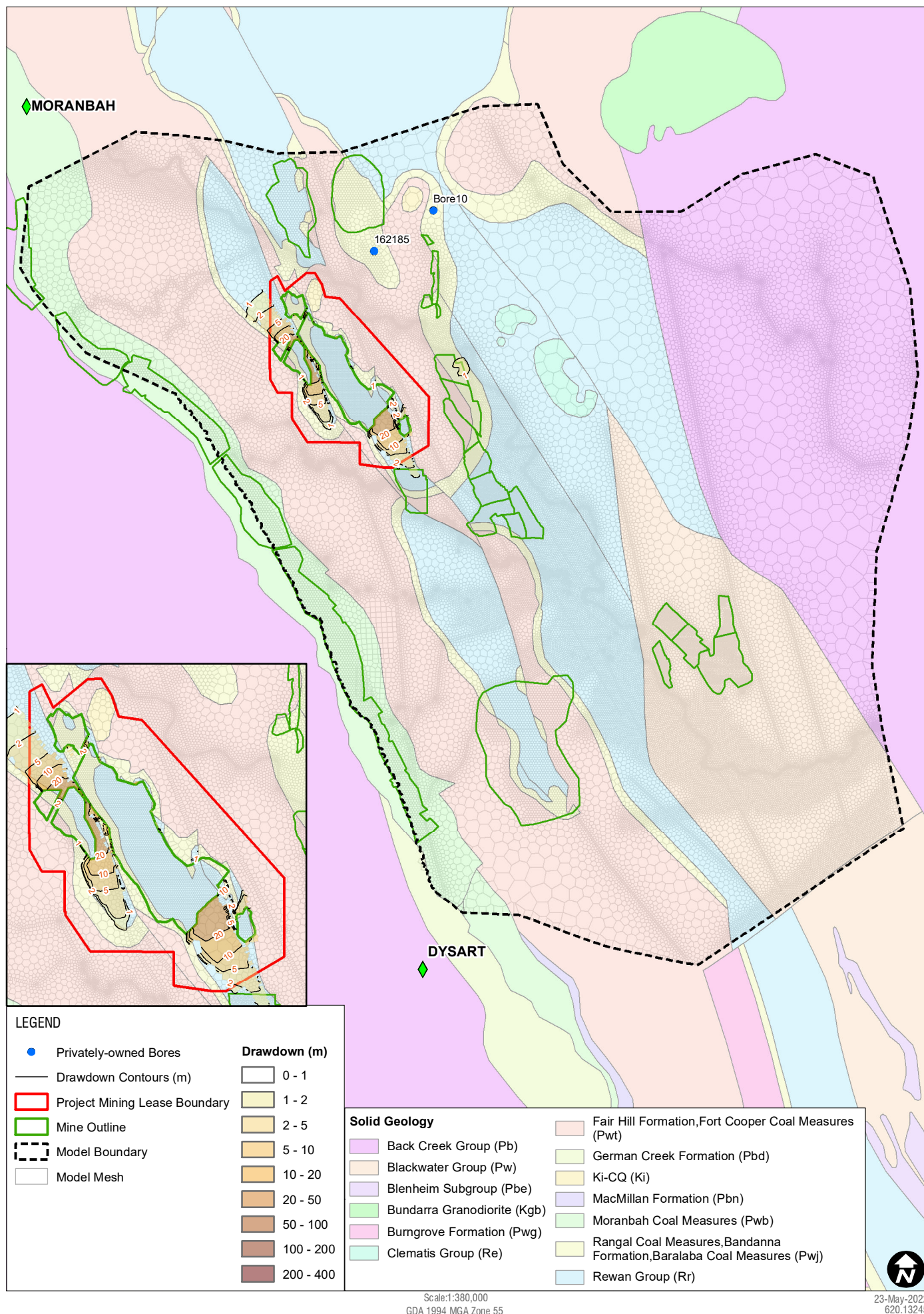
FIGURE D-4

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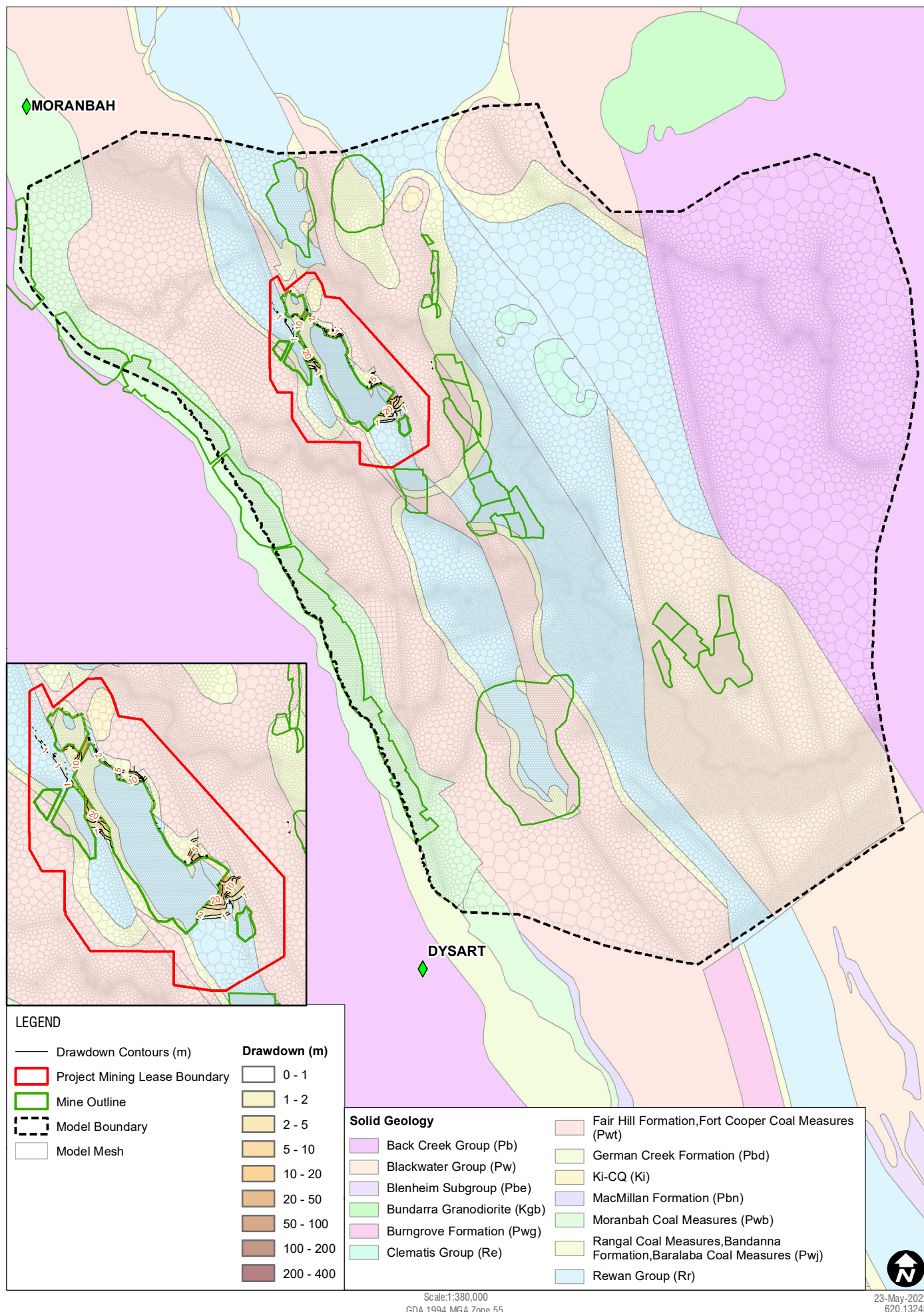
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**Incremental Drawdown in
Leichhardt Seam (Layer 5) 2053**

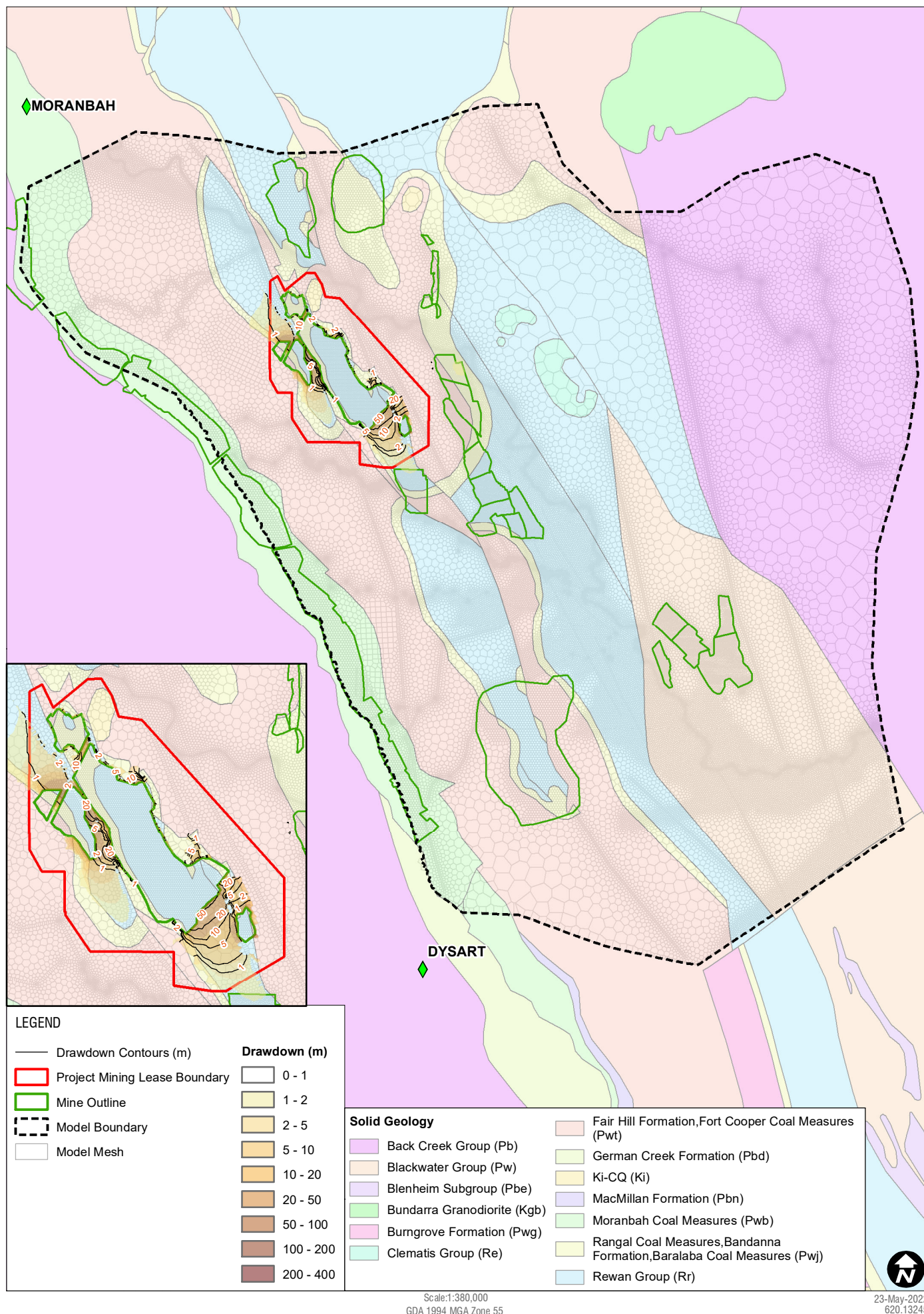
FIGURE D-6

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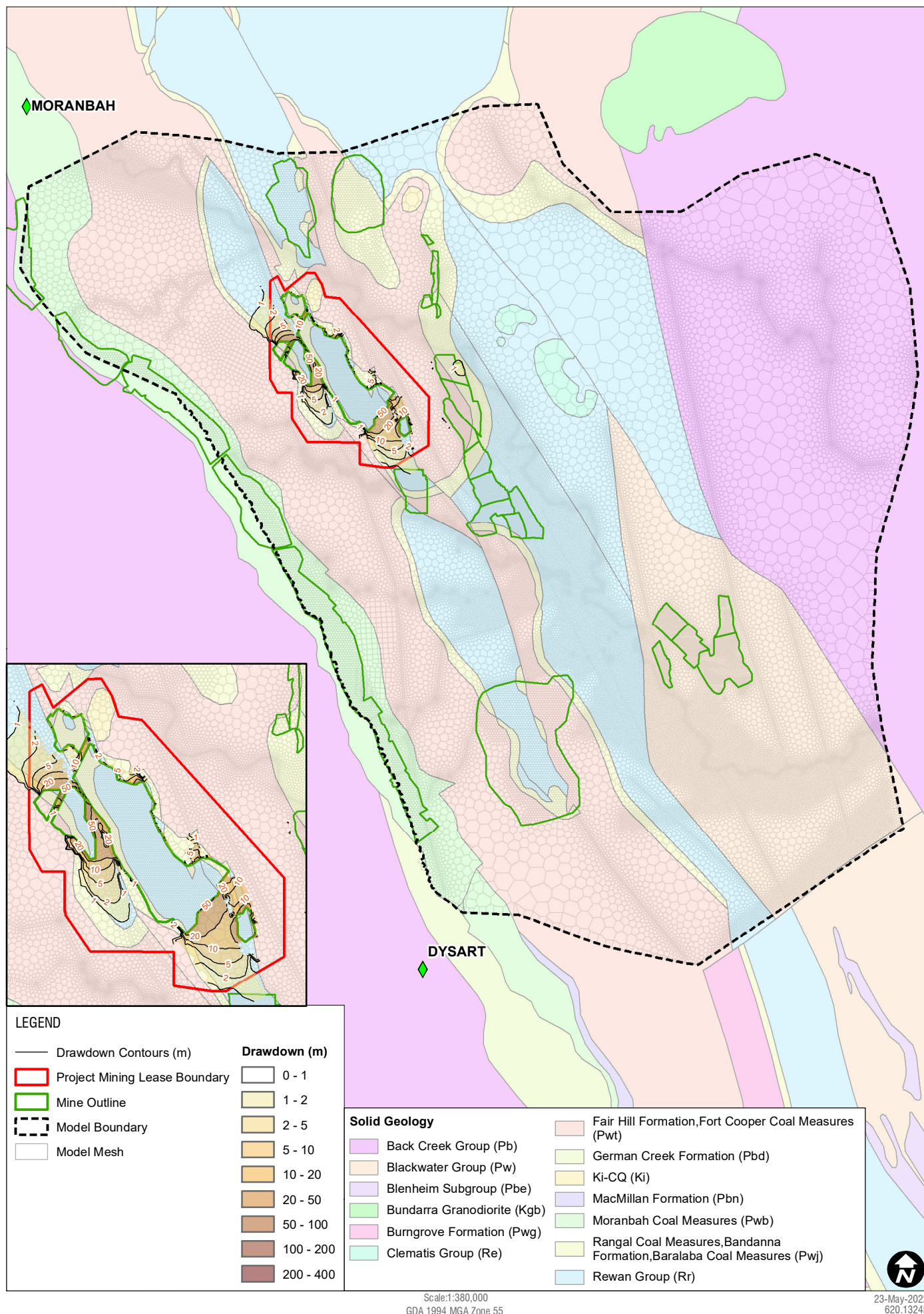
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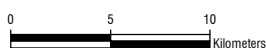
Incremental Drawdown in Vermont Seam (Layer 7) 2053

FIGURE D-9



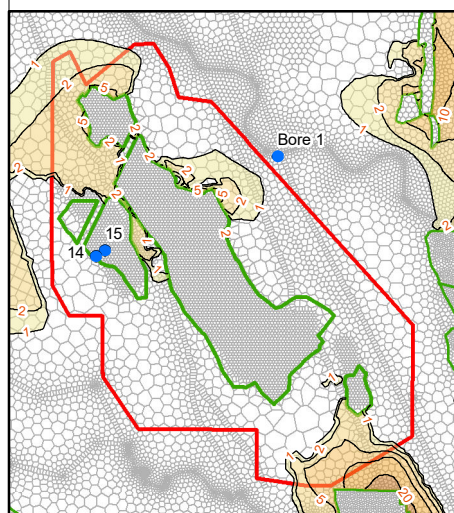
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	Drawdown Contours (m)		0 - 1
	Project Mining Lease Boundary		1 - 2
	Mine Outline		2 - 5
	Model Boundary		5 - 10
	Model Mesh		10 - 20
			20 - 50
			50 - 100
			100 - 200
			200 - 400

23-May-2022
620.13245



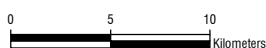
Cumulative Drawdown in Regolith (Layer 2) 2033

FIGURE D-13



	Privately-owned Bores		Drawdown (m)
	Drawdown Contours (m)		0 - 1
	Project Mining Lease Boundary		1 - 2
	Mine Outline		2 - 5
	Model Boundary		5 - 10
	Model Mesh		10 - 20
			20 - 50
			50 - 100
			100 - 200
			200 - 400

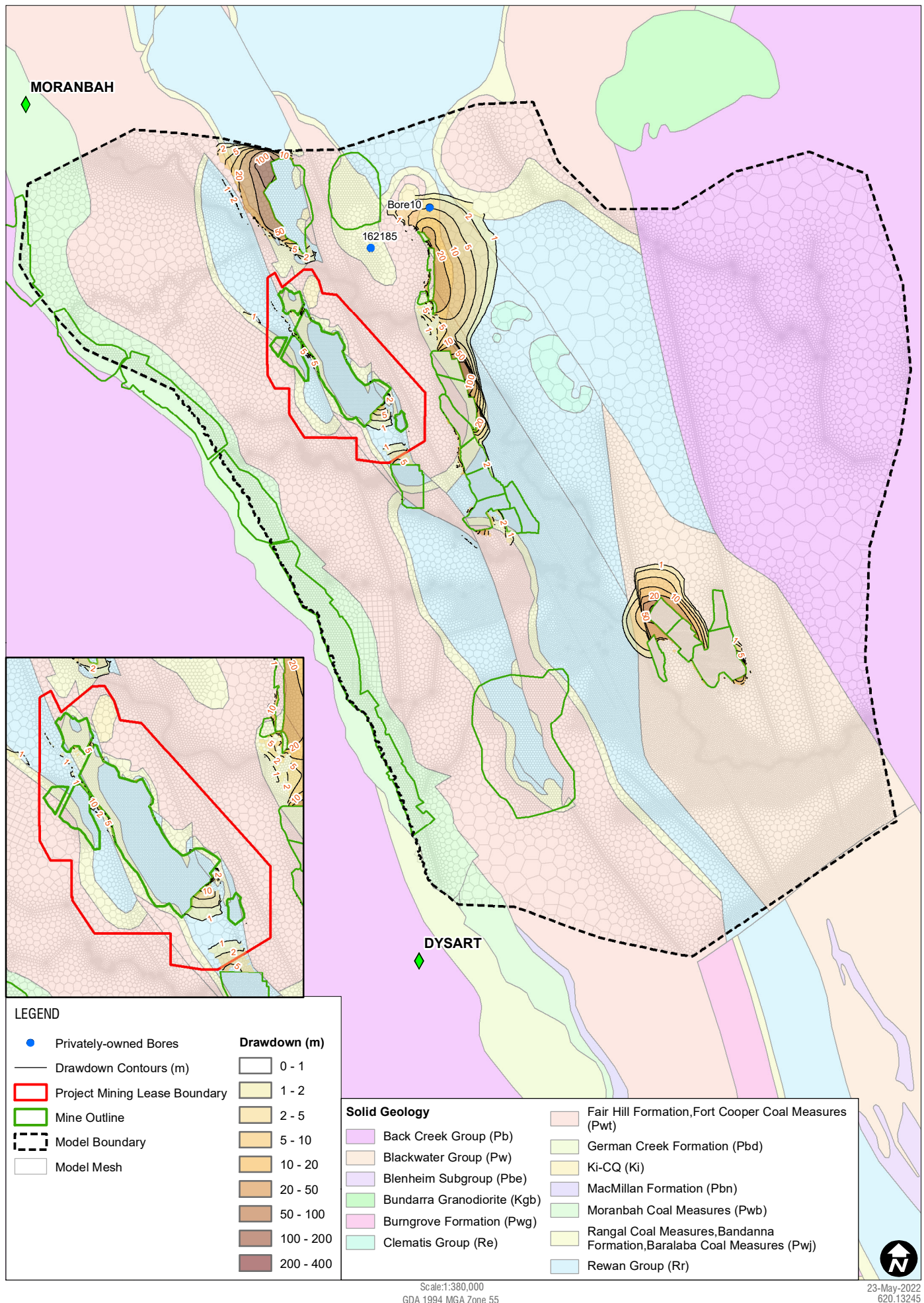
23-May-2022
620.13245



Cumulative Drawdown in Regolith (Layer 2) 2043

FIGURE D-14

H:\Projects\SLR\620-BNE\620-BNE\620-BNE\13.245 Winchester South Groundwater\05 SLR Data\01 CAD\GIS\ArcGIS\Revised Mine Plan\Appendix DD\62013245 D-16 Cumulative Drawdown in Leichhardt Seam (Layer 5) 2023.mxd



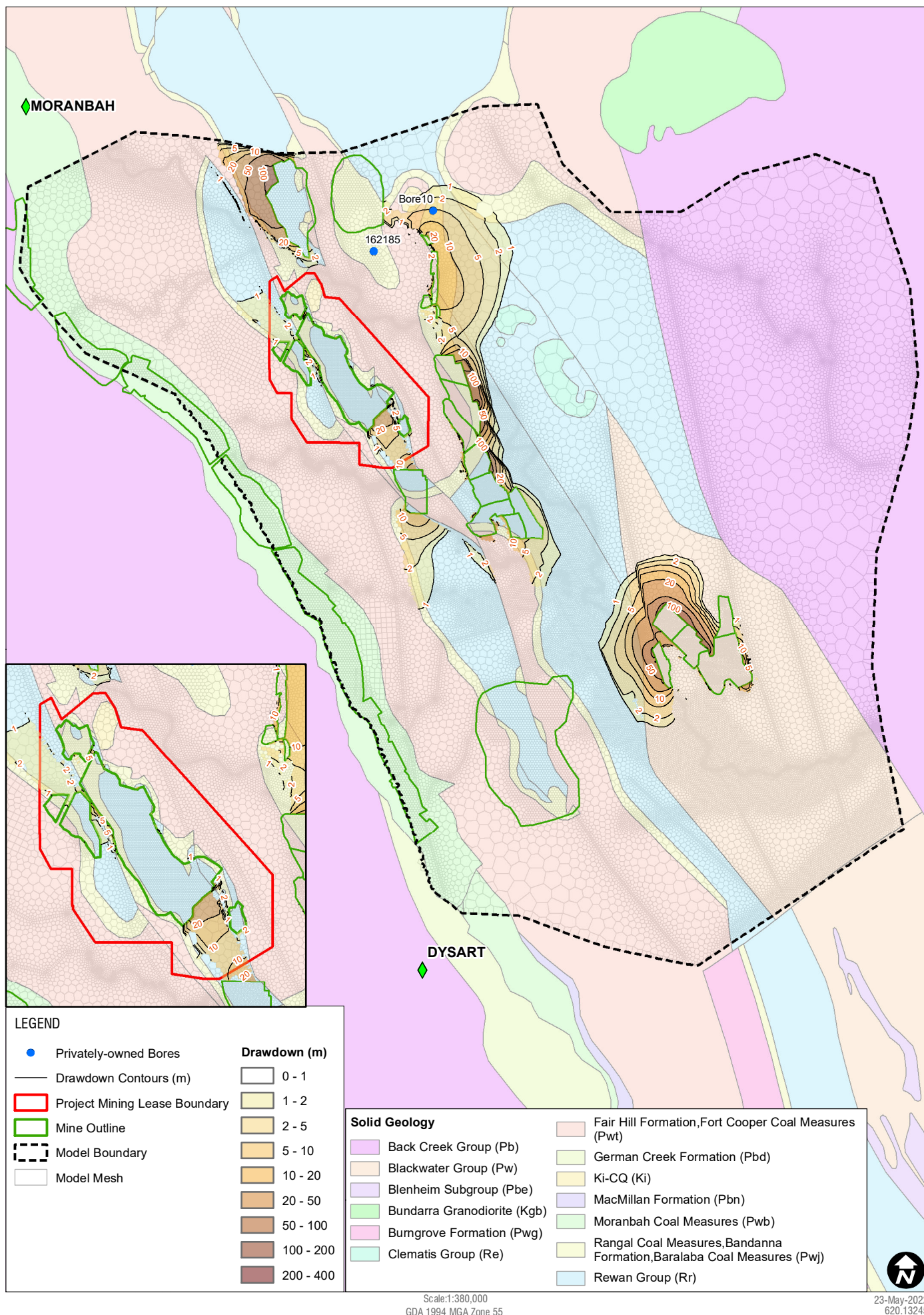
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**Cumulative Drawdown in
Leichhardt Seam (Layer 5) 2023**

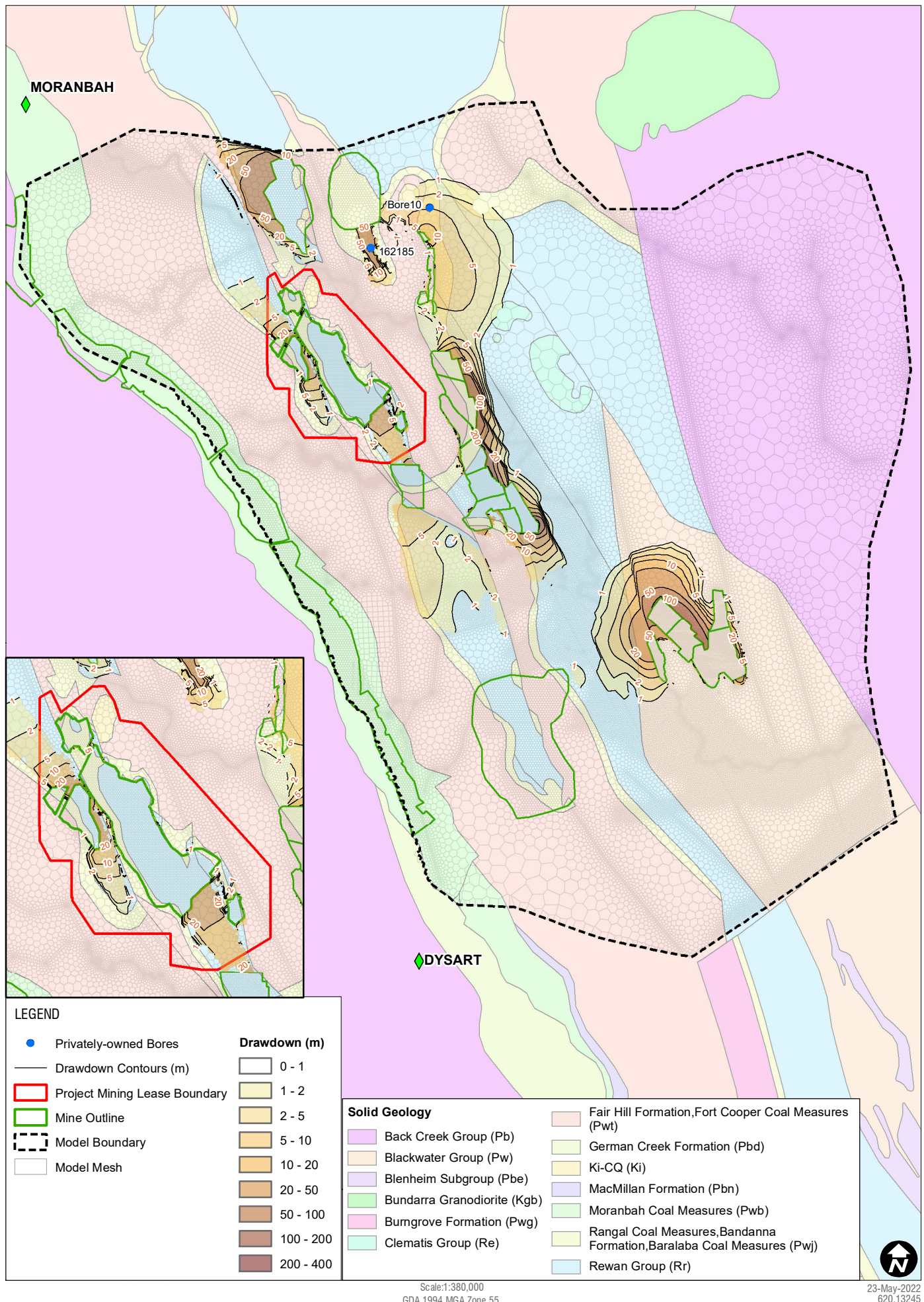
FIGURE D-16

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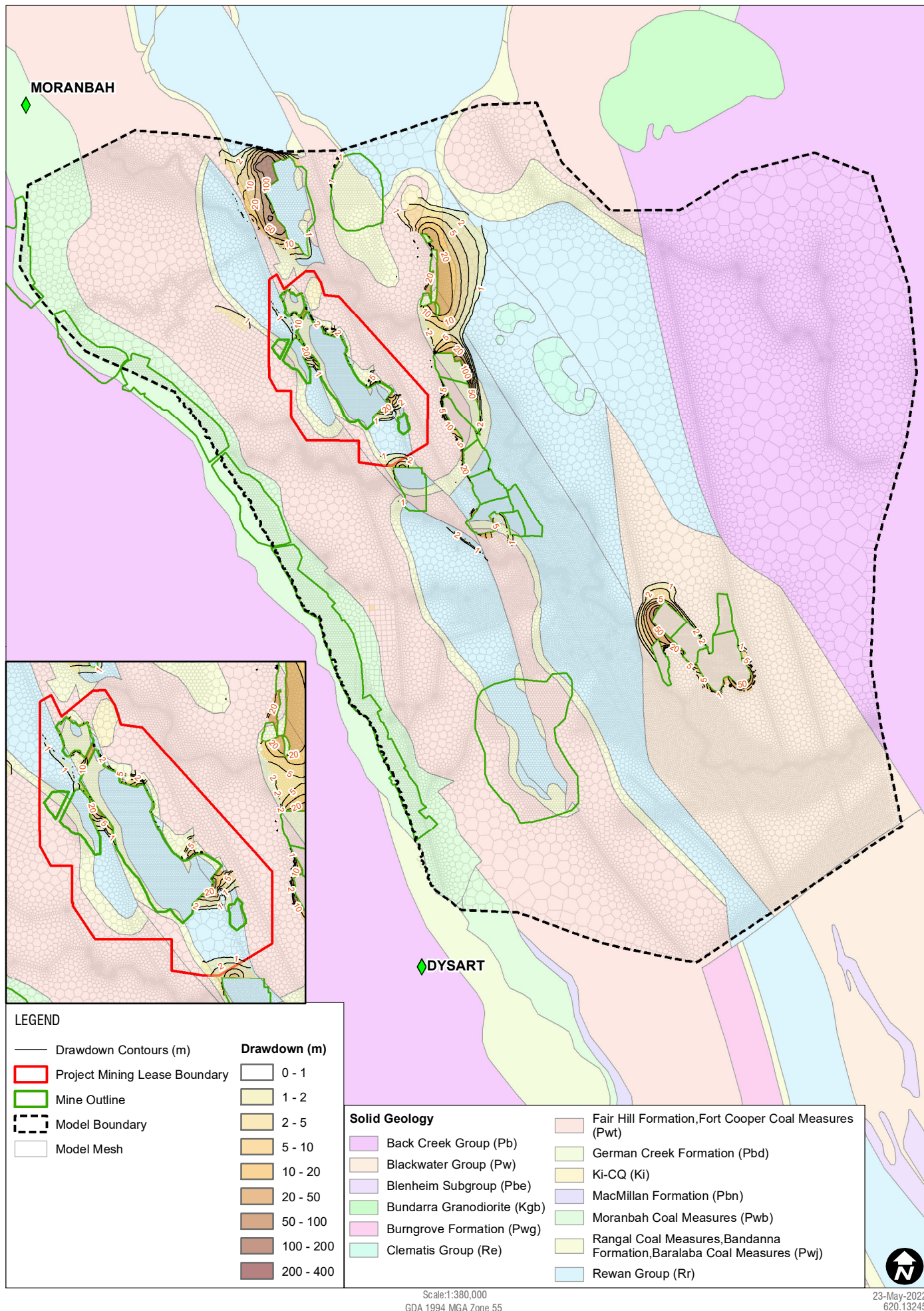
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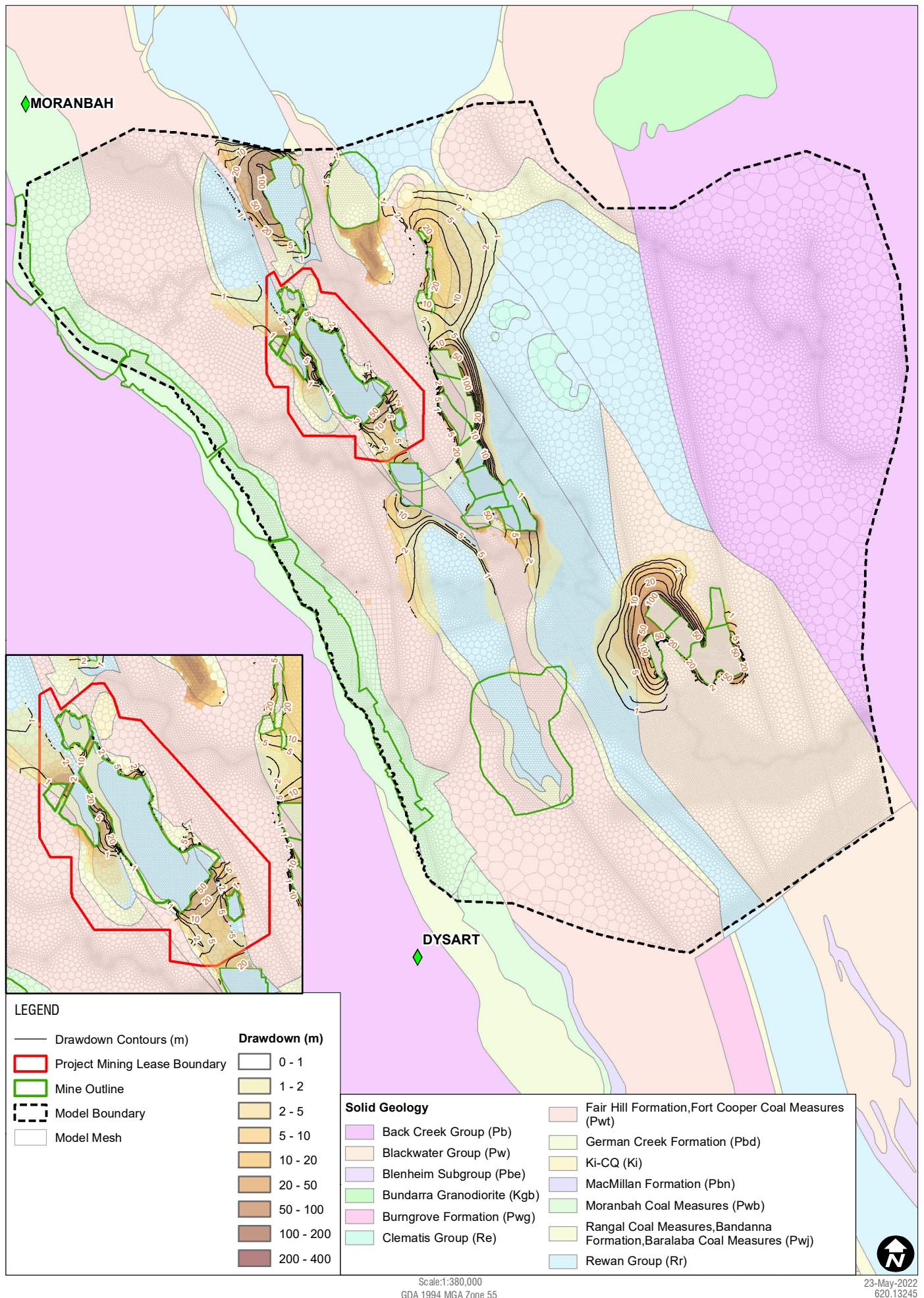
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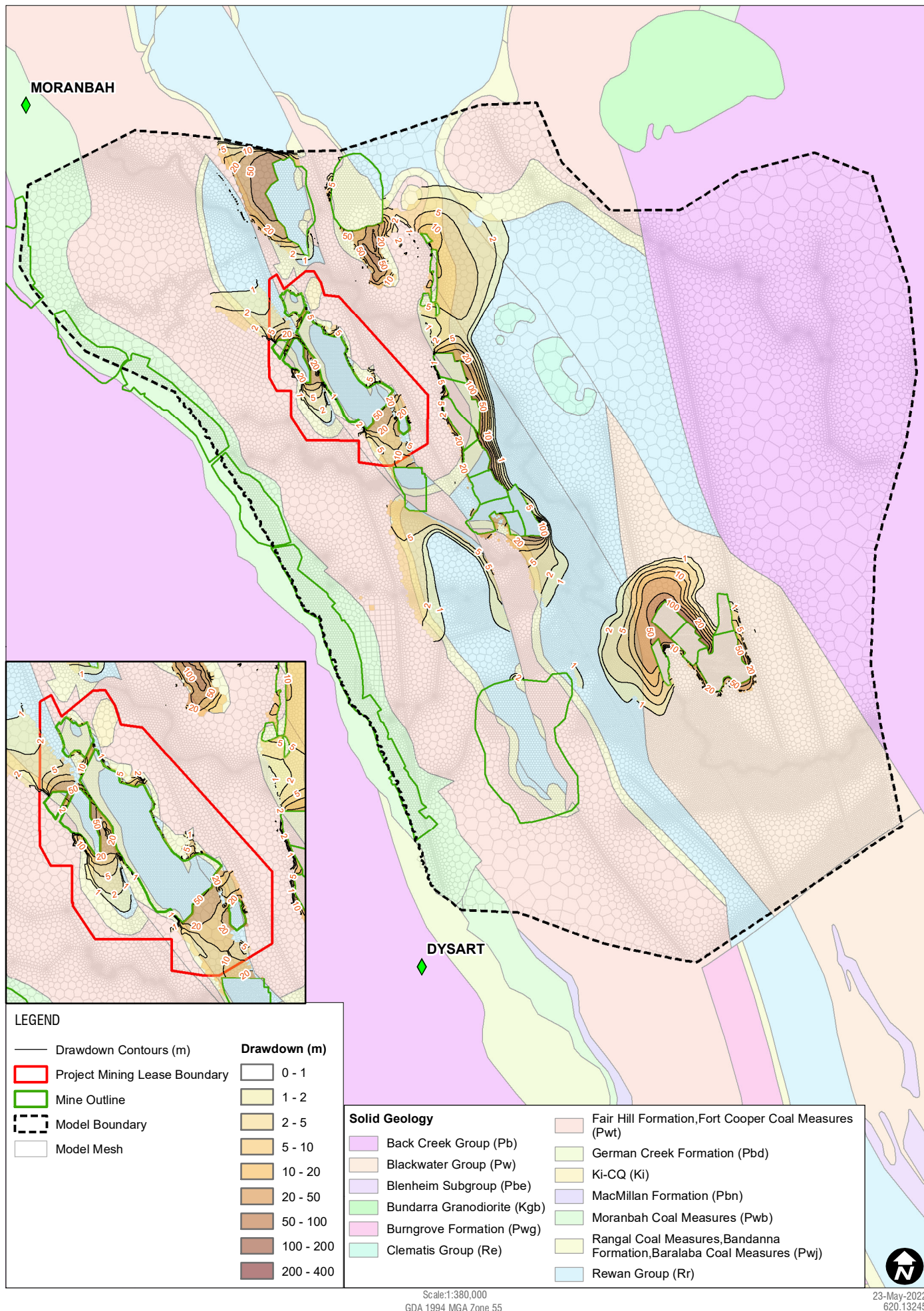
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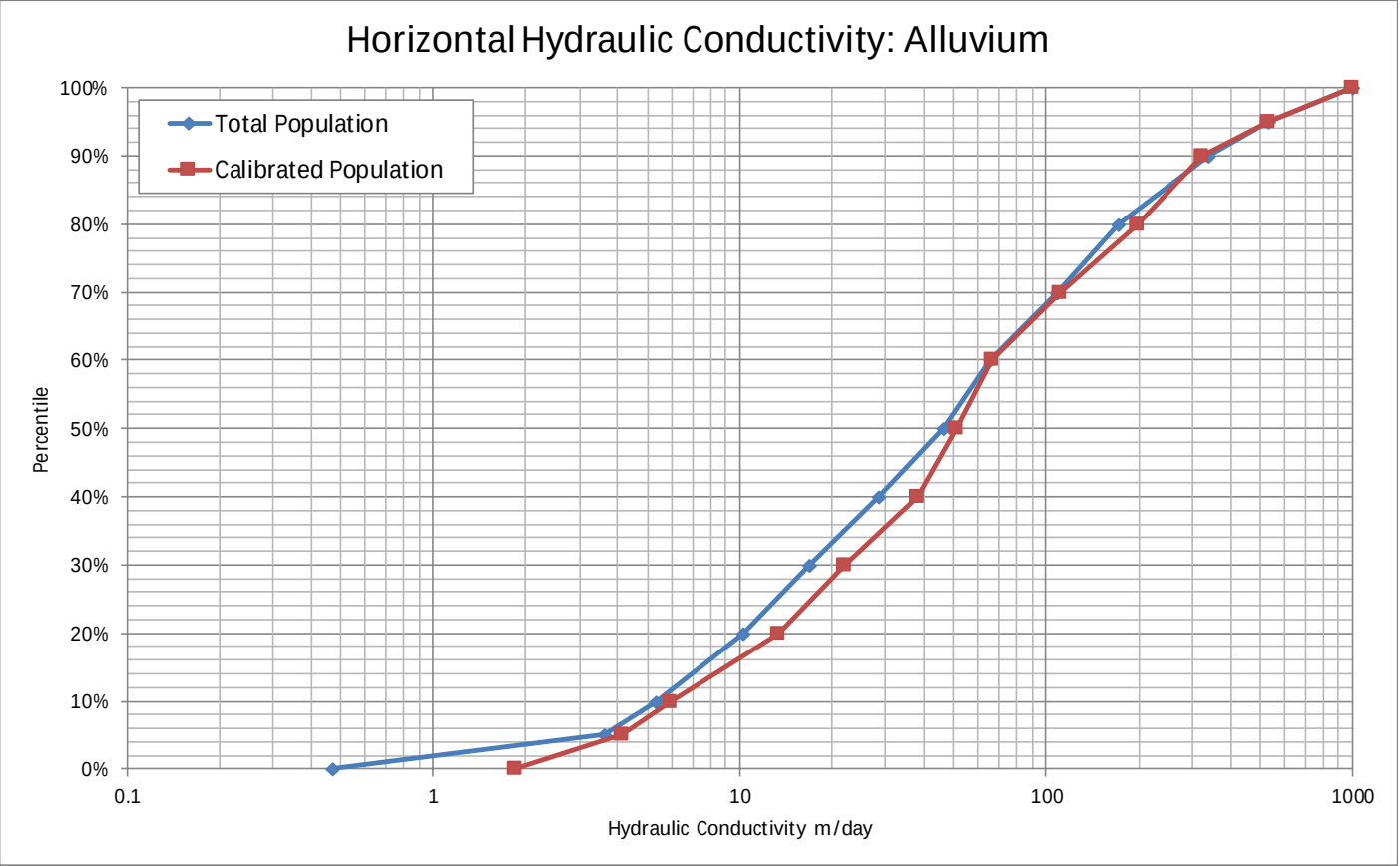
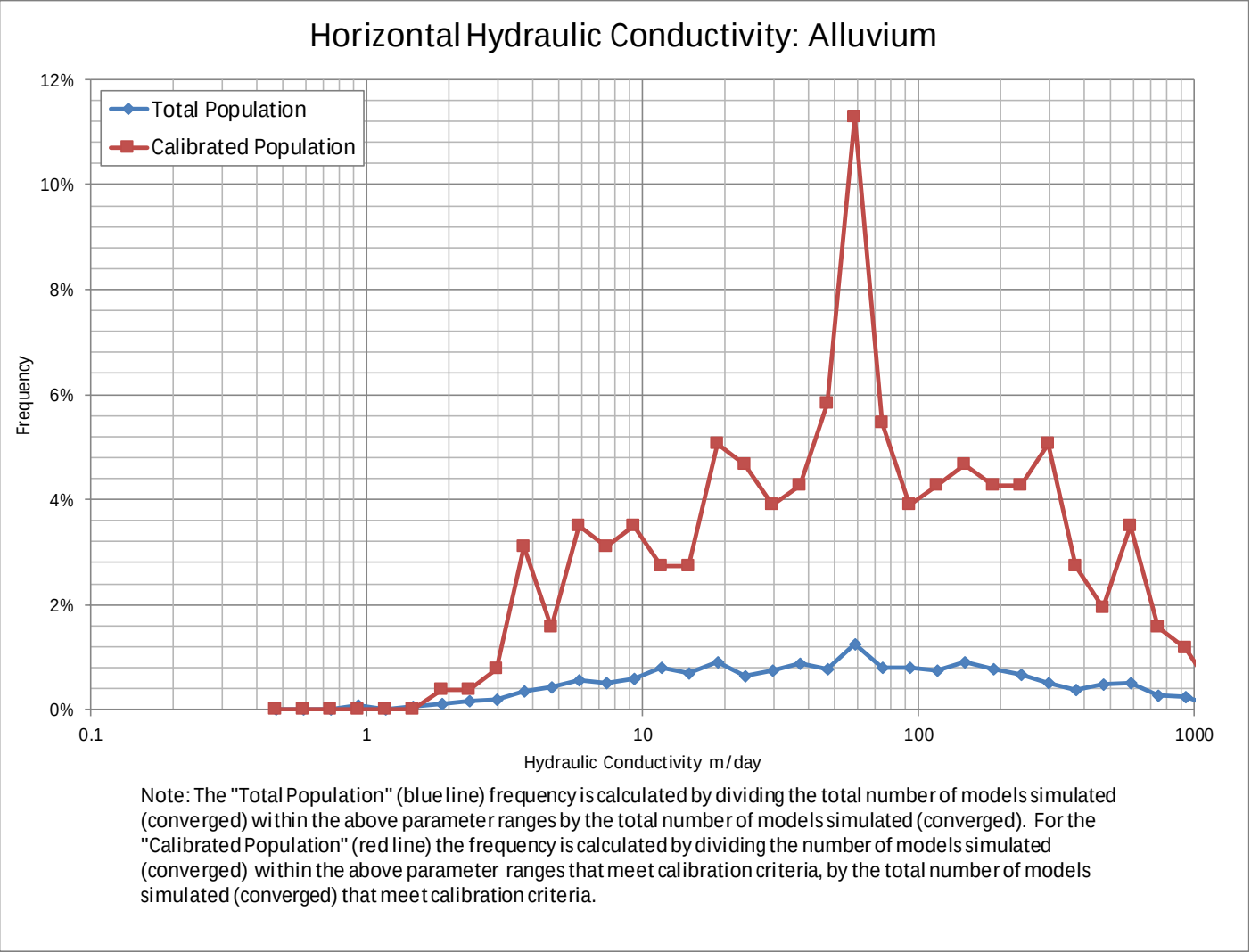
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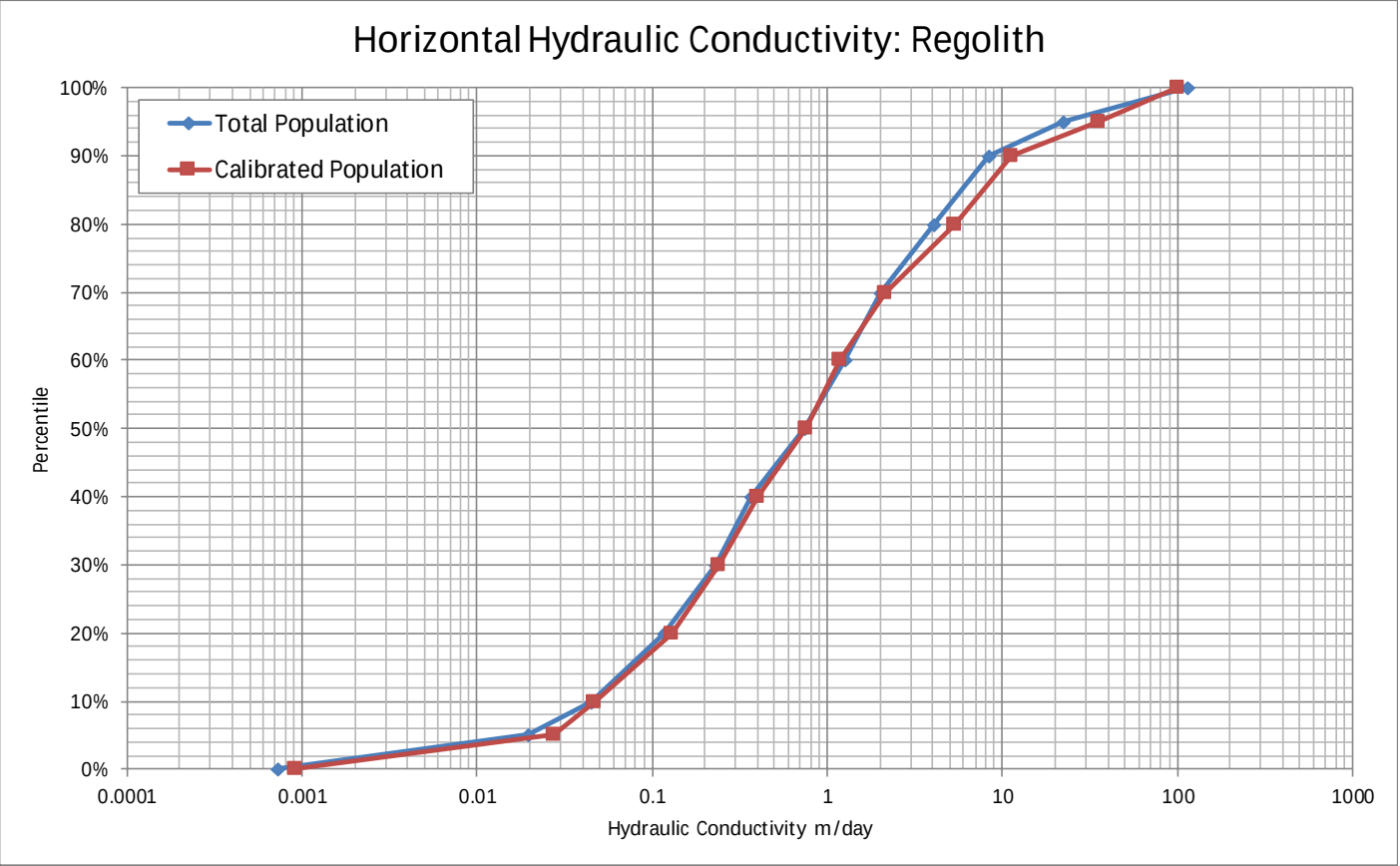
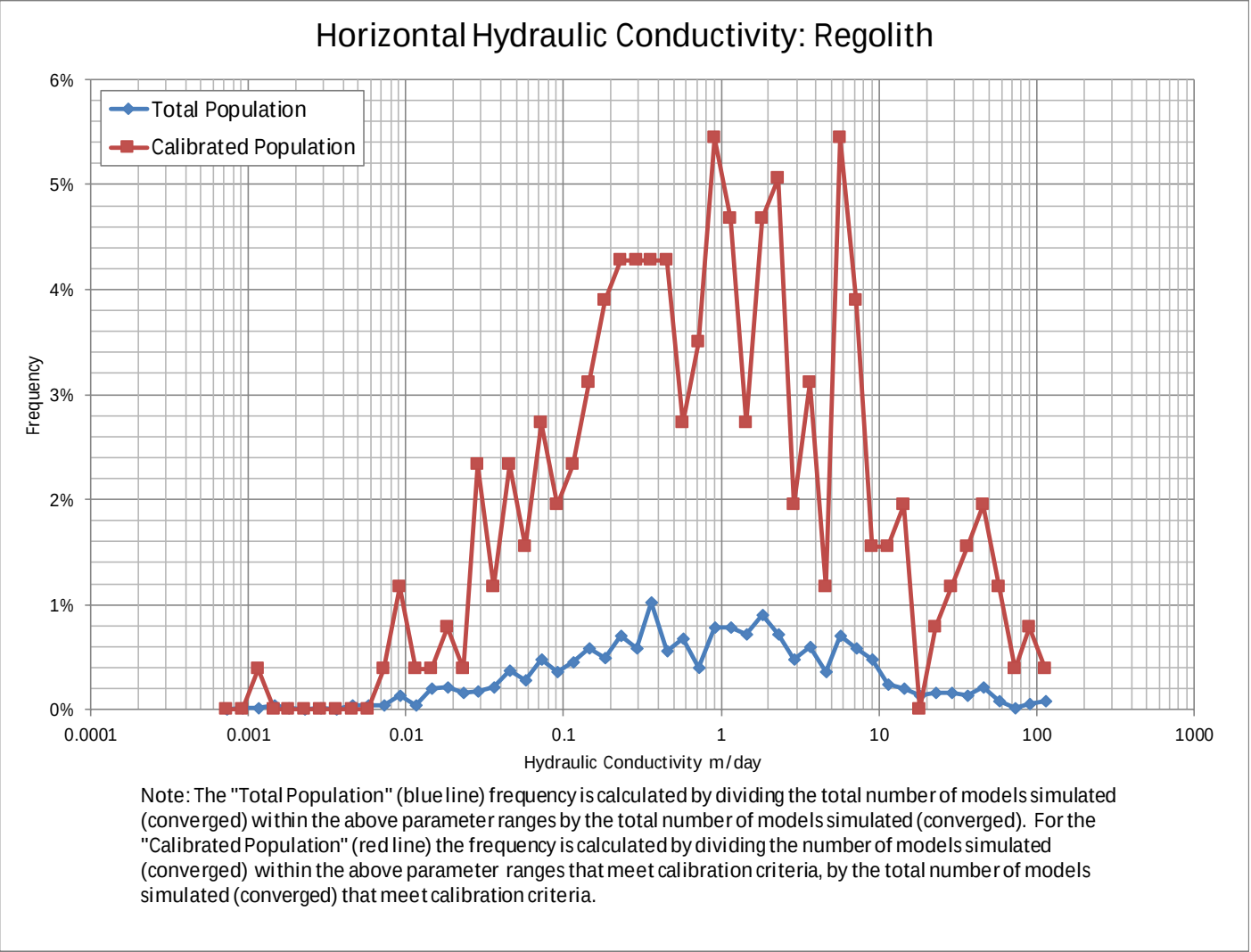


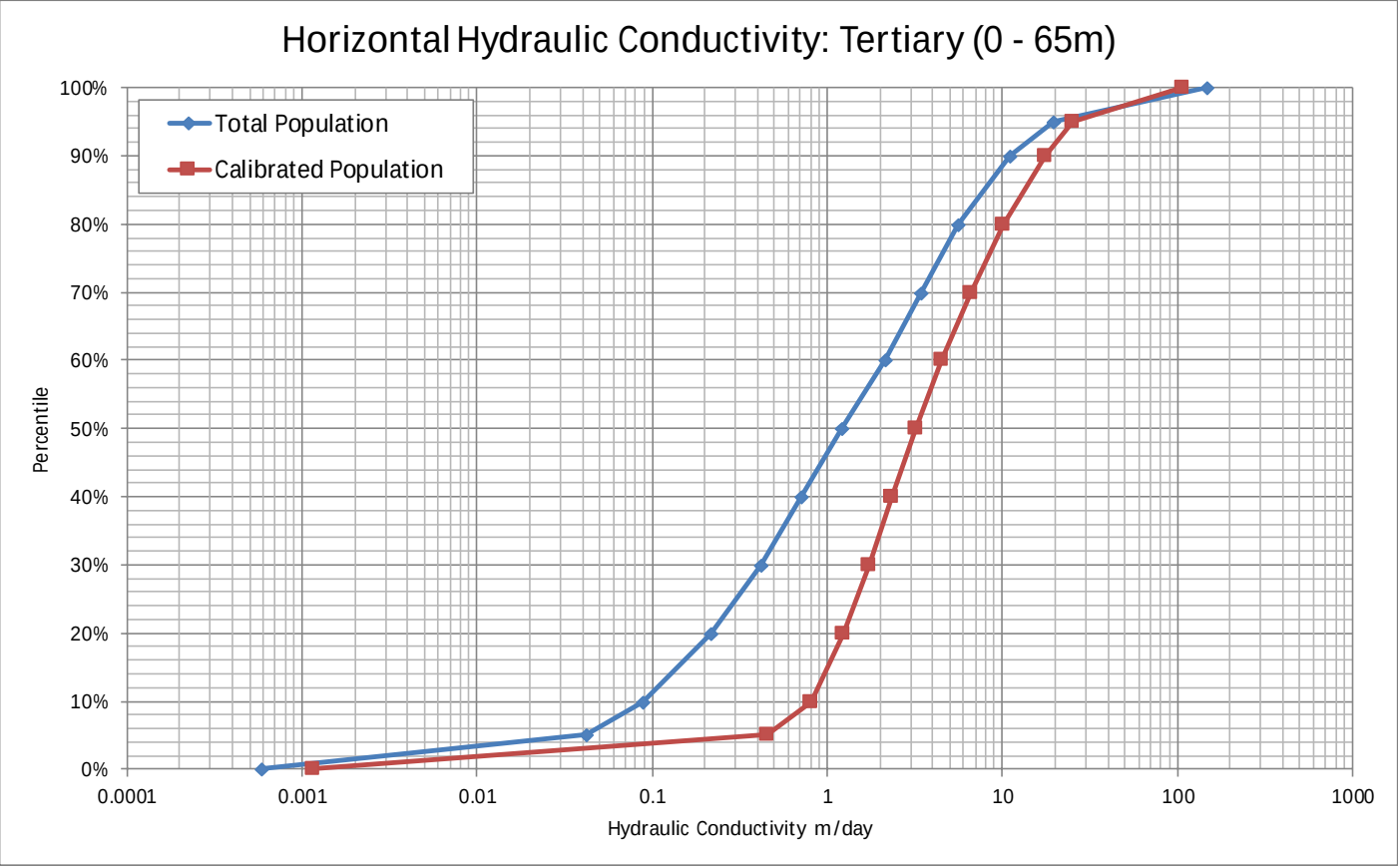
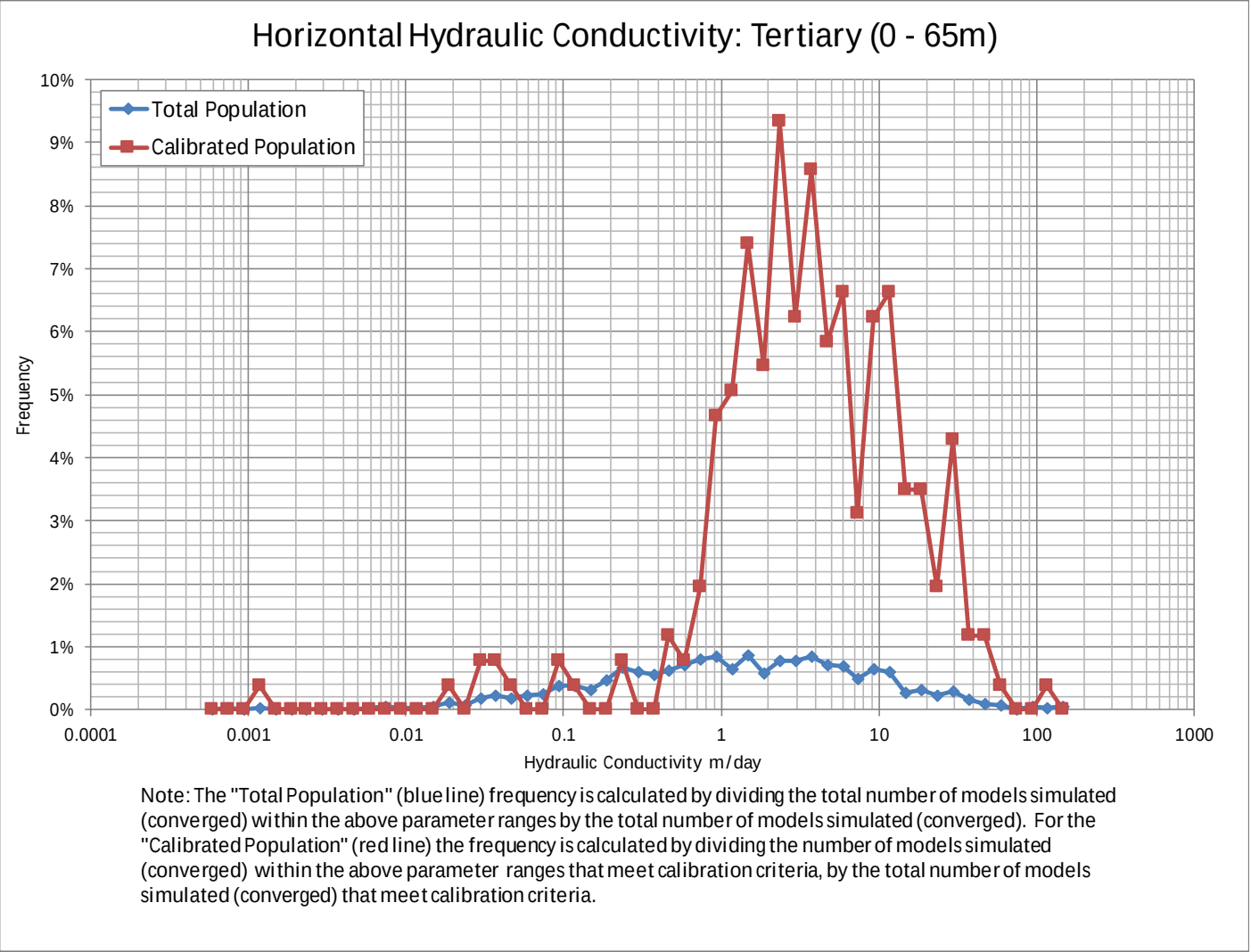
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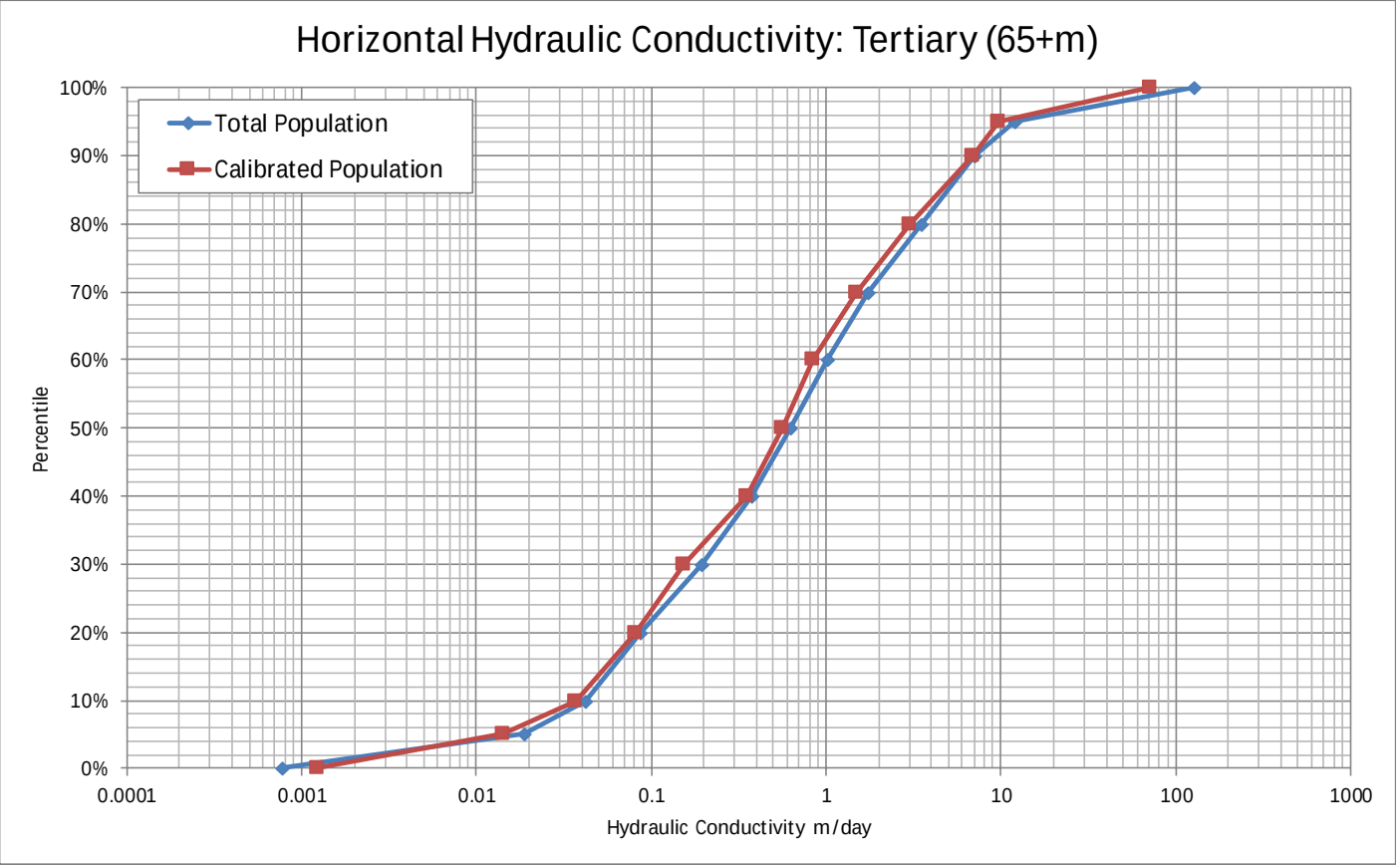
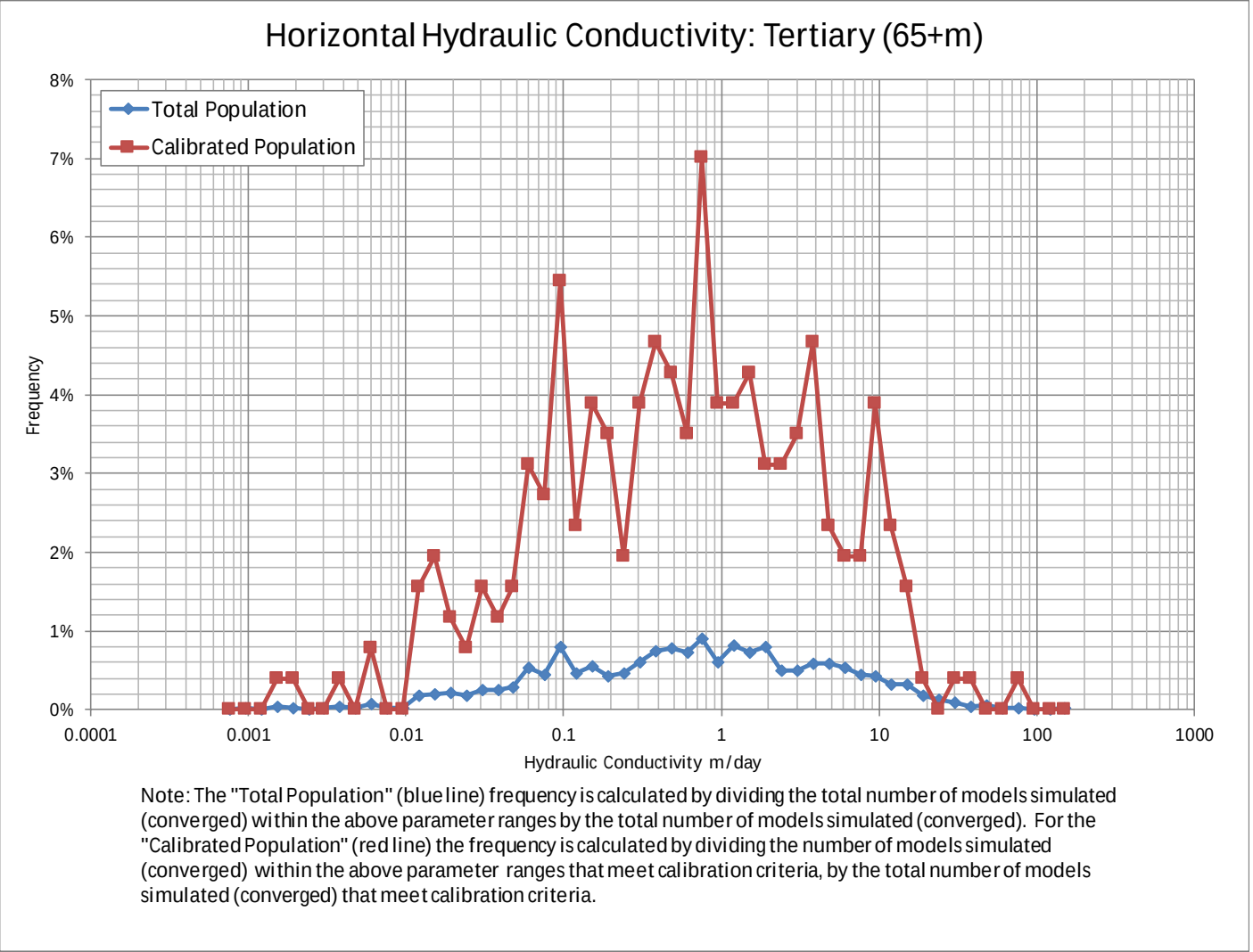
APPENDIX E

Parameter Distributions

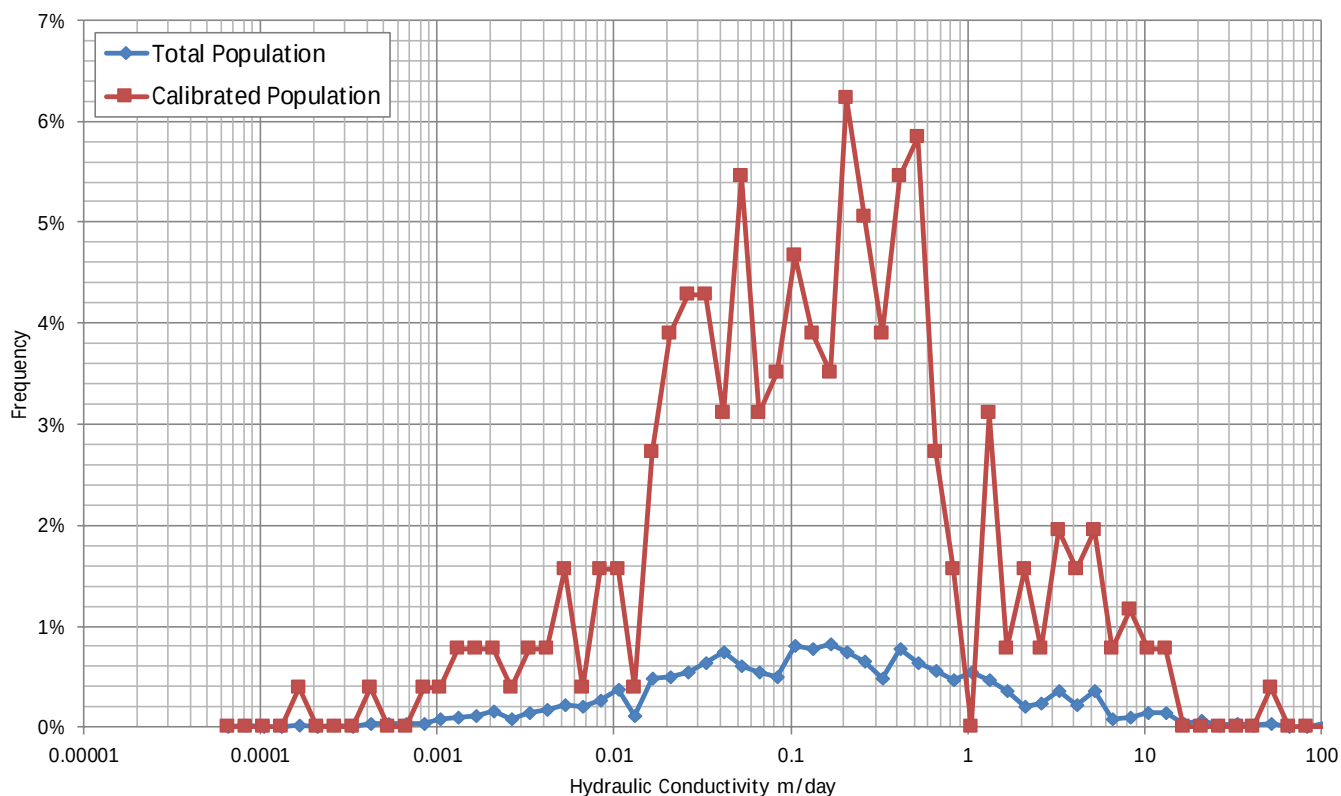






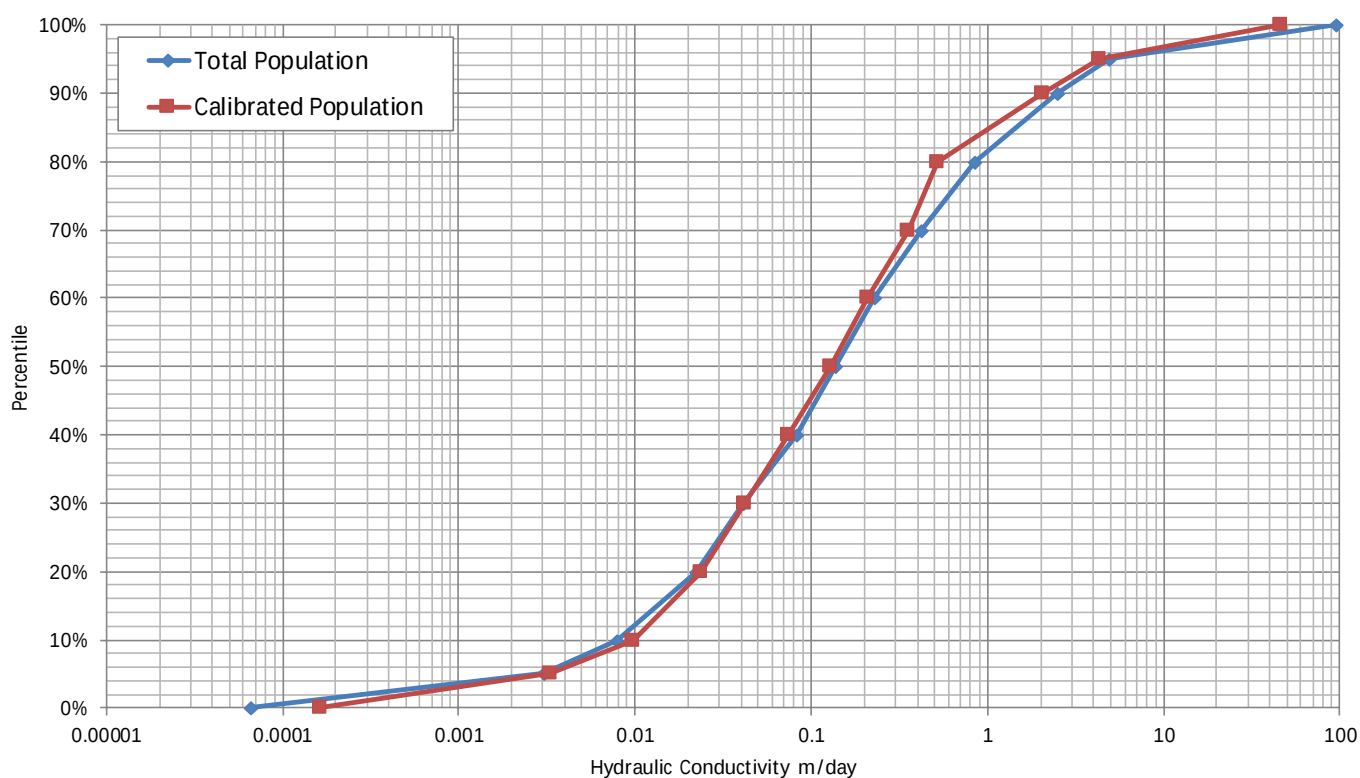


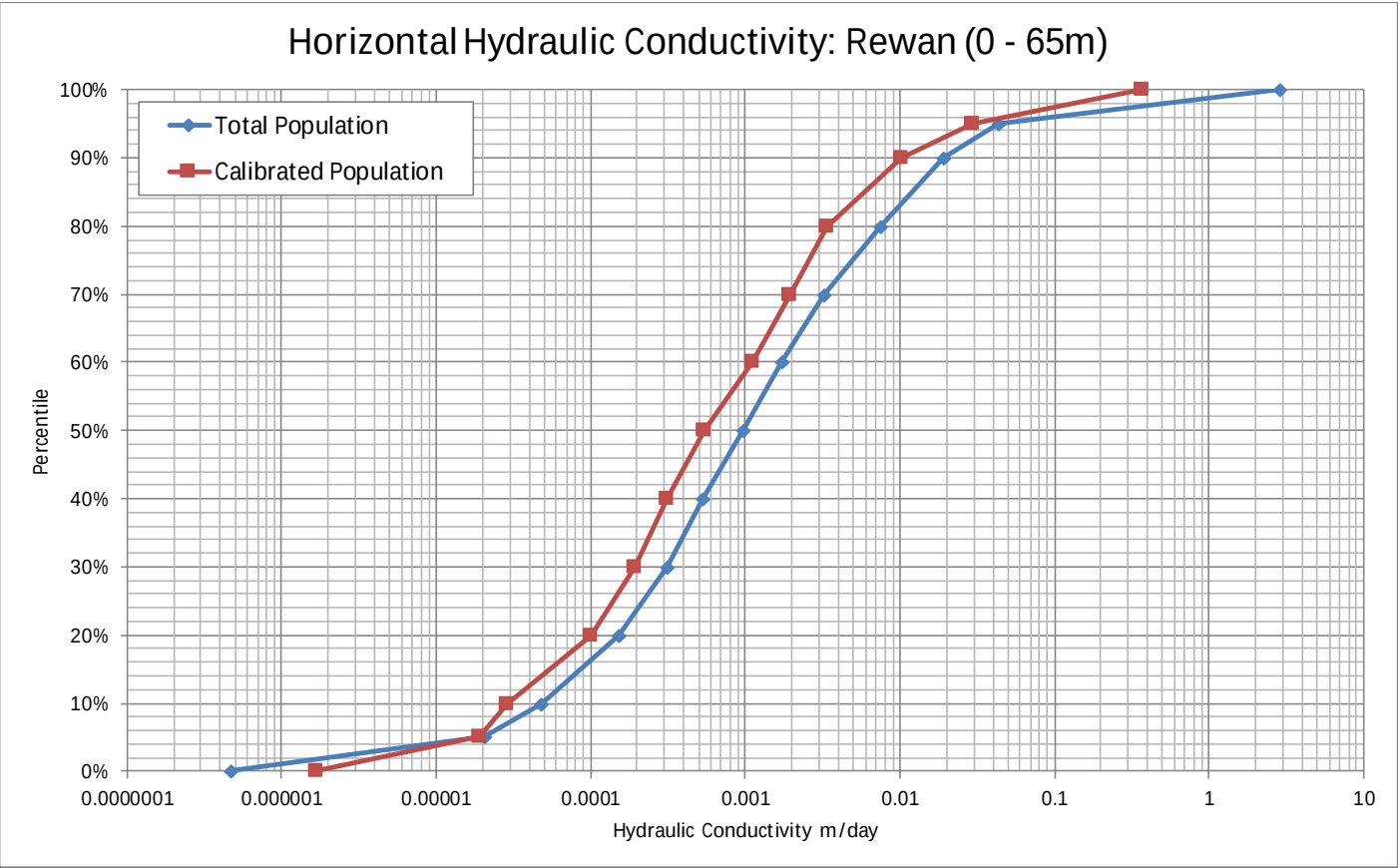
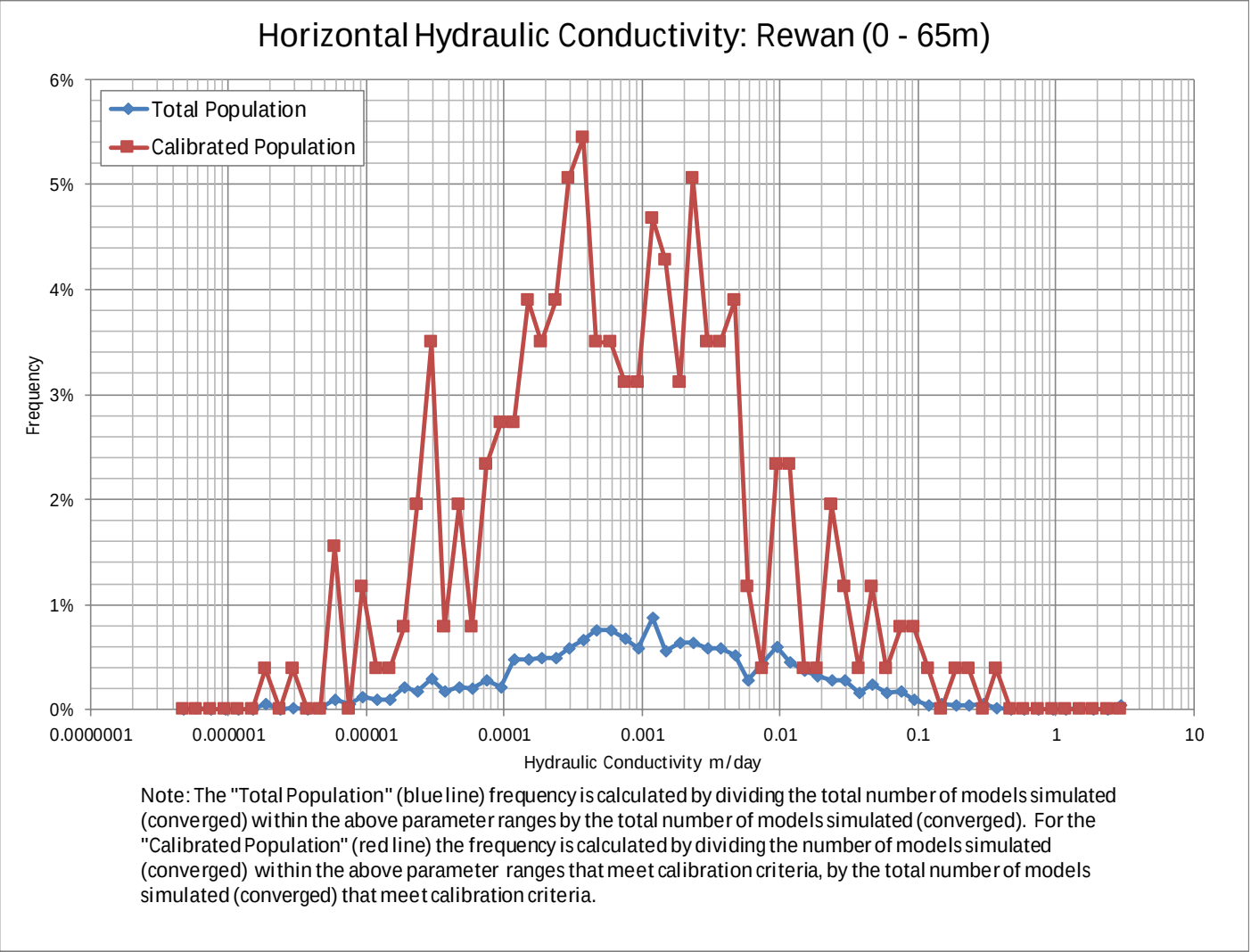
Horizontal Hydraulic Conductivity: Tertiary Basalt

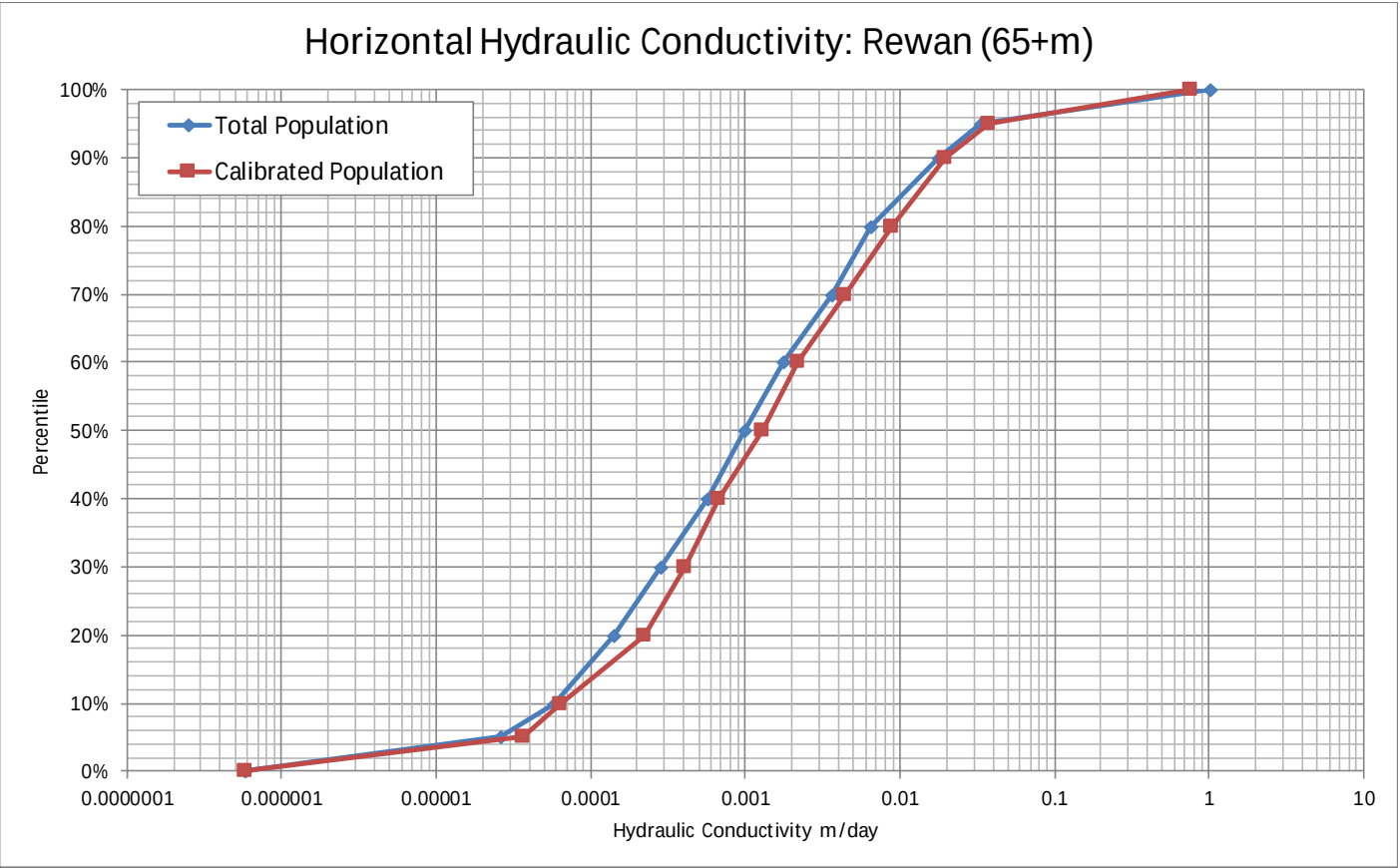
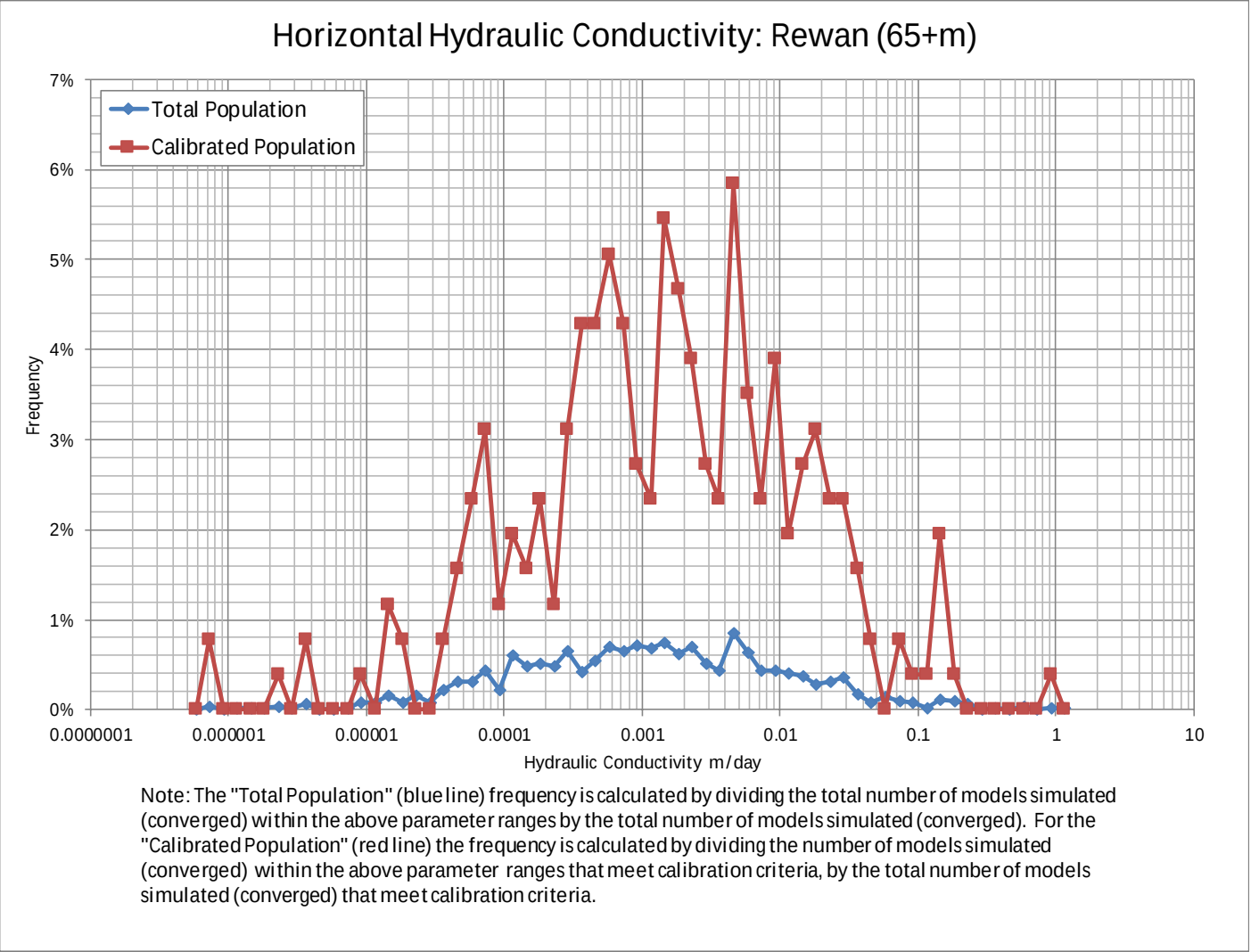


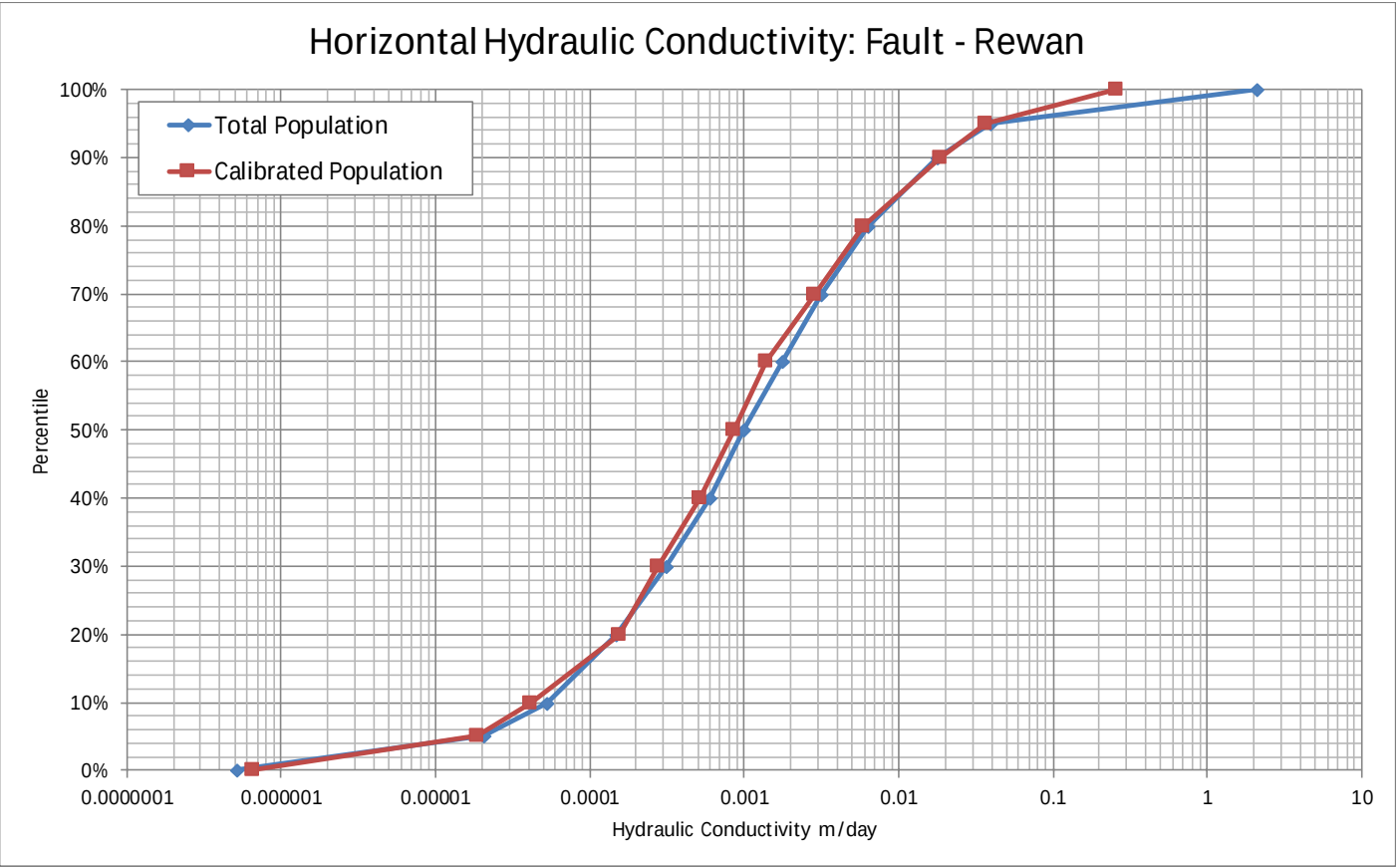
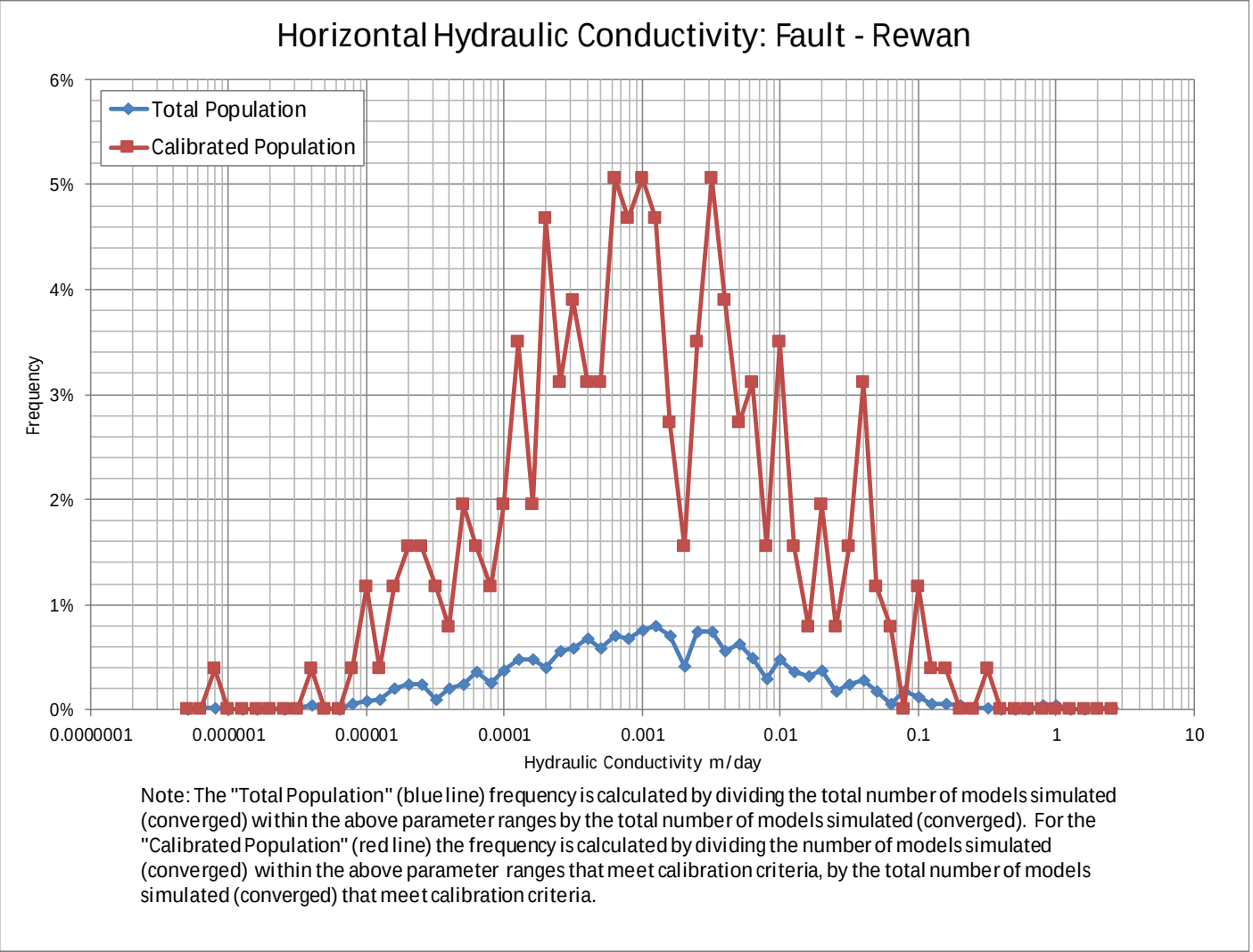
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

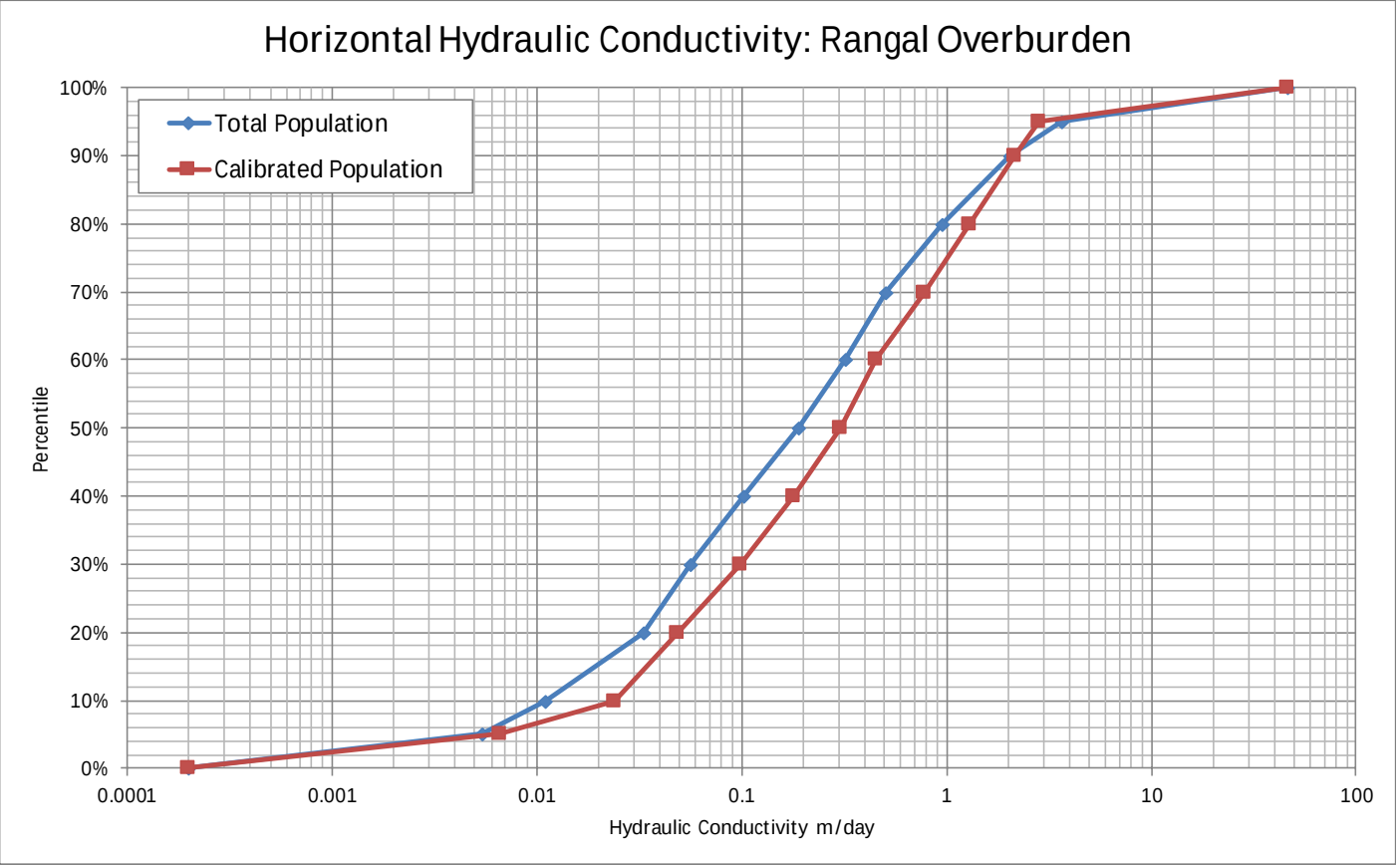
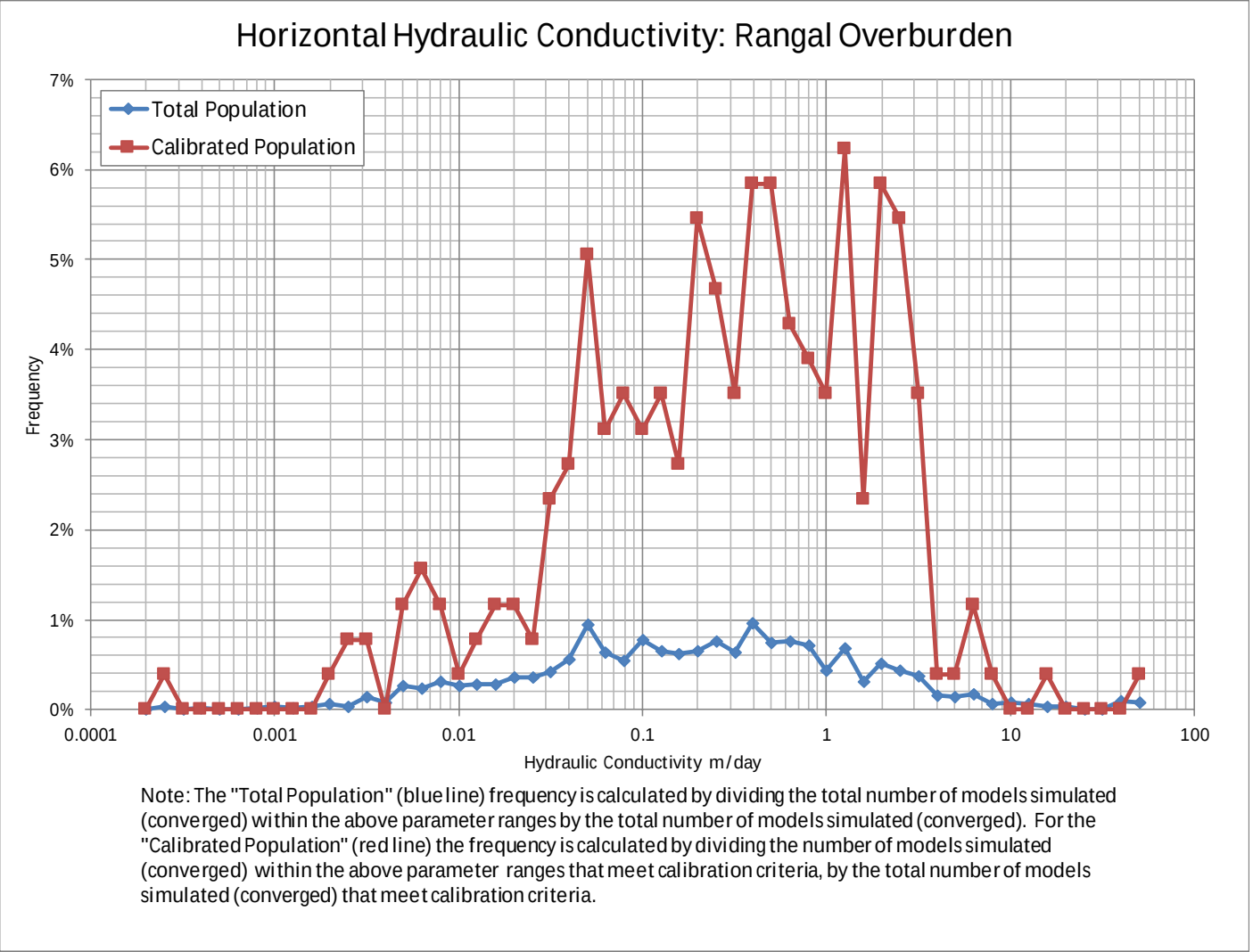
Horizontal Hydraulic Conductivity: Tertiary Basalt

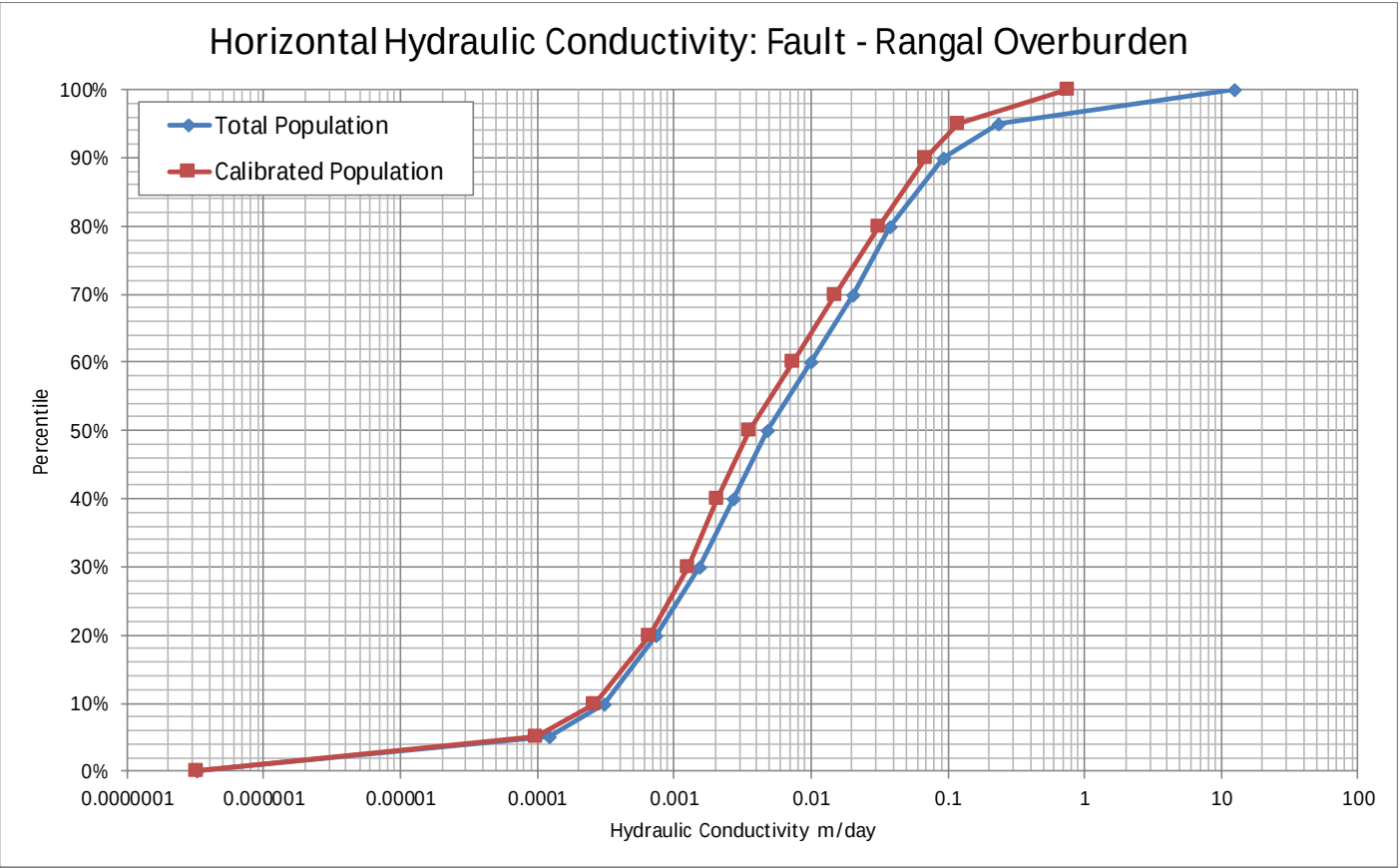
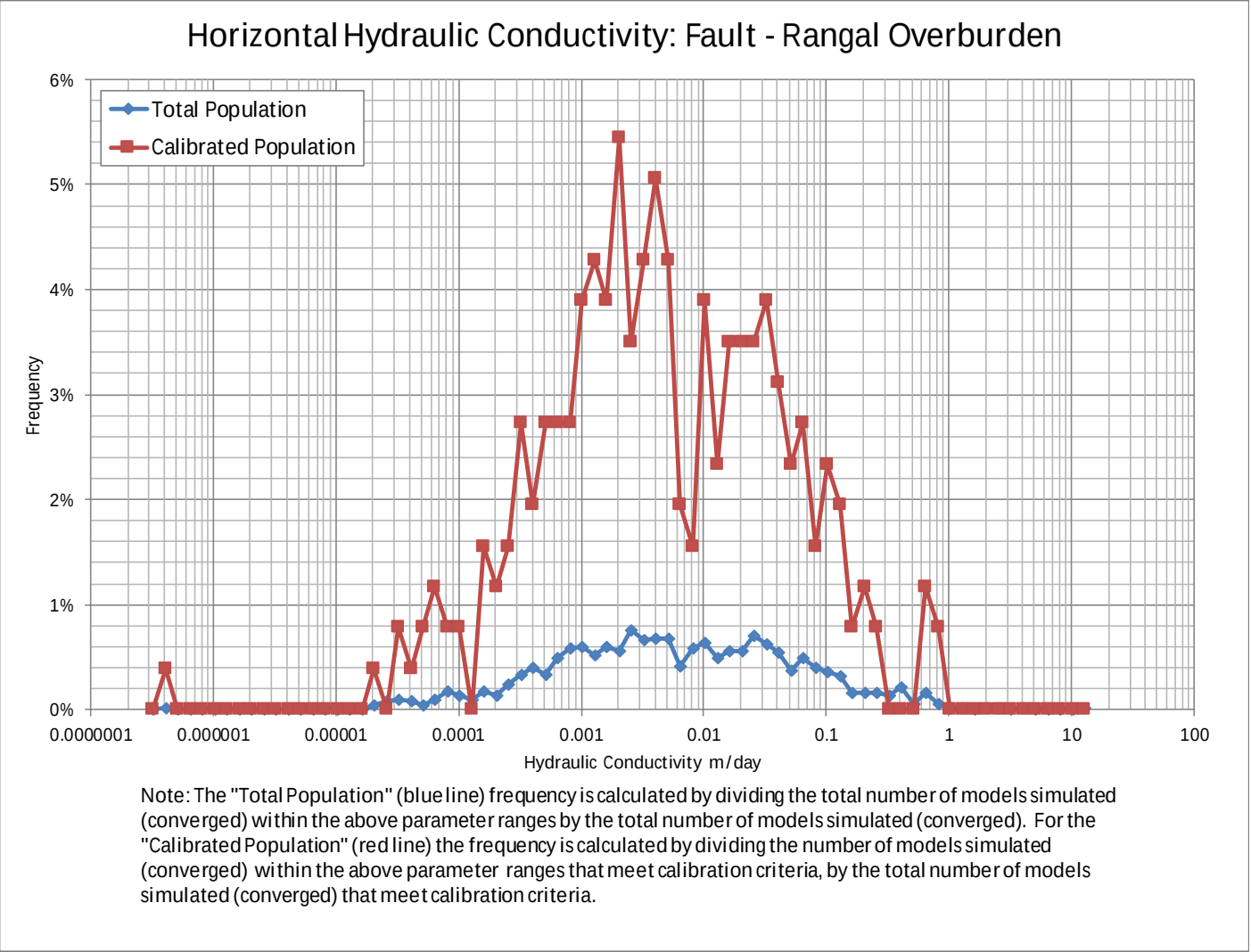


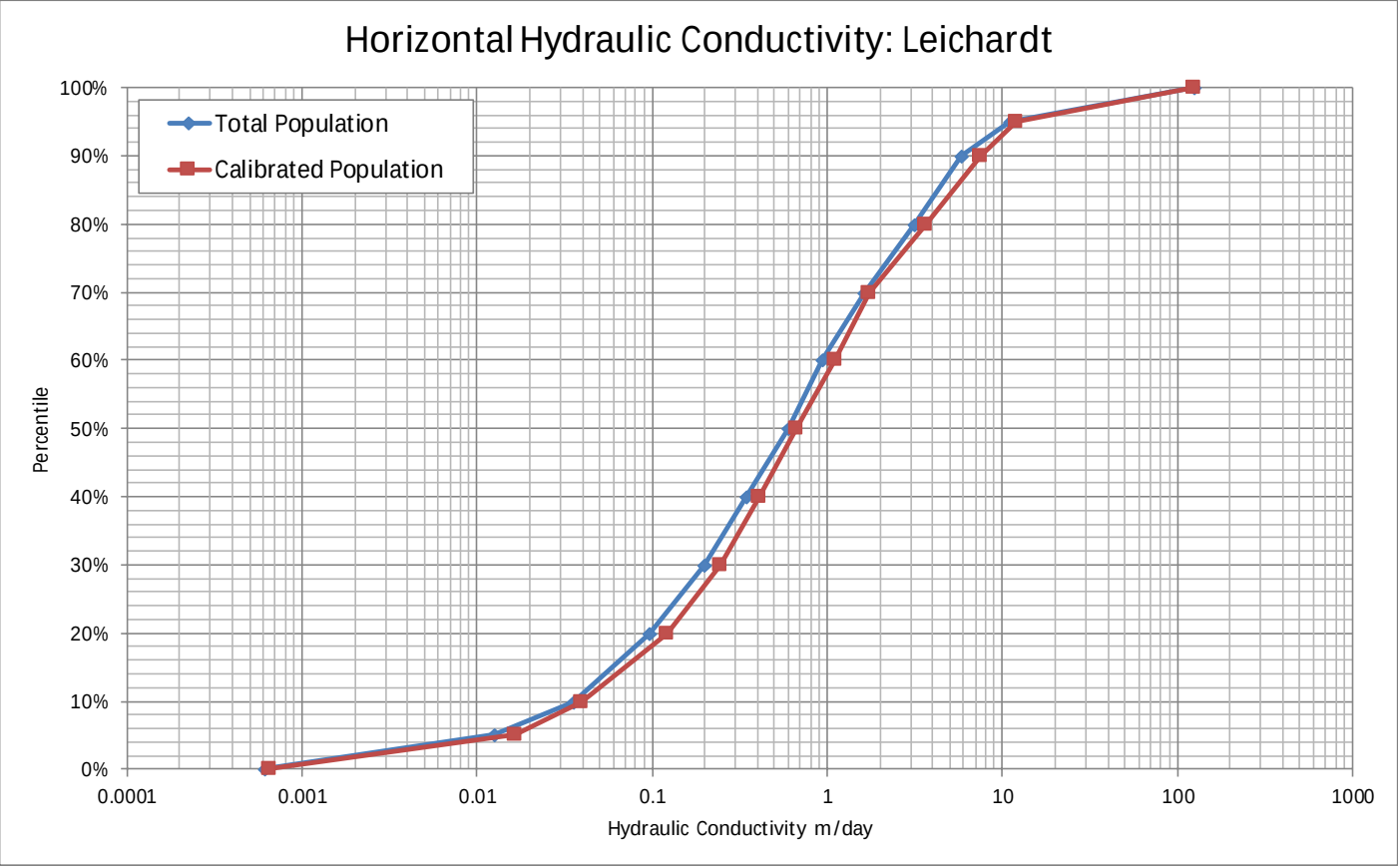
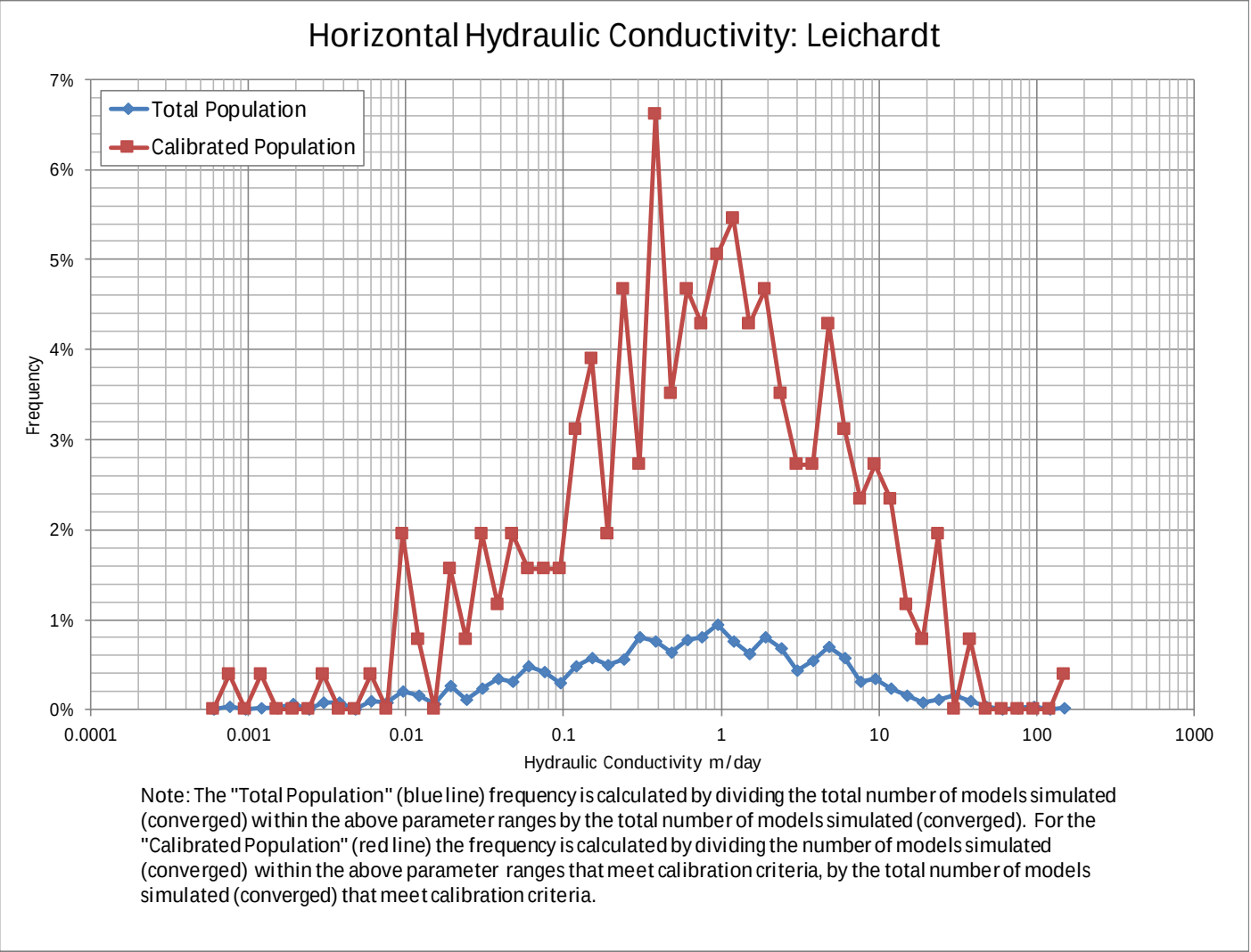




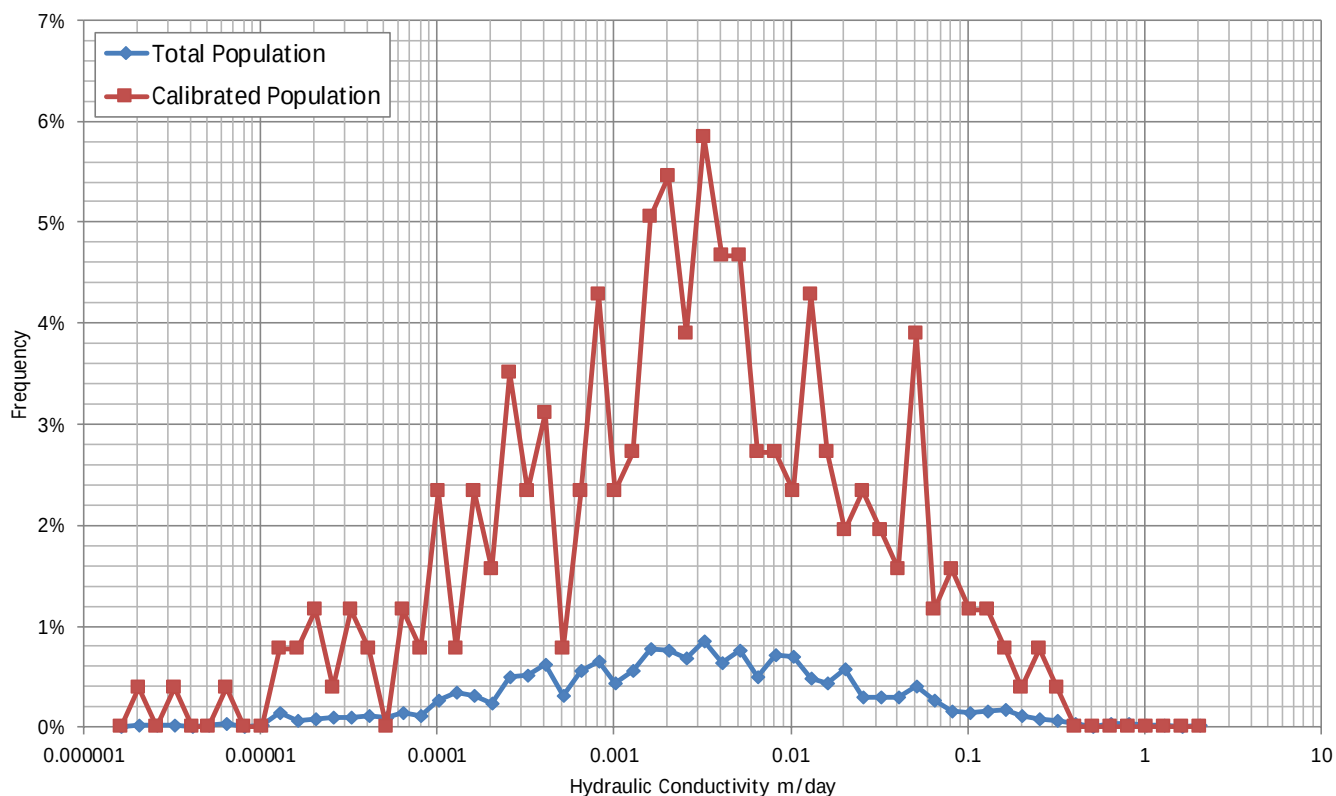






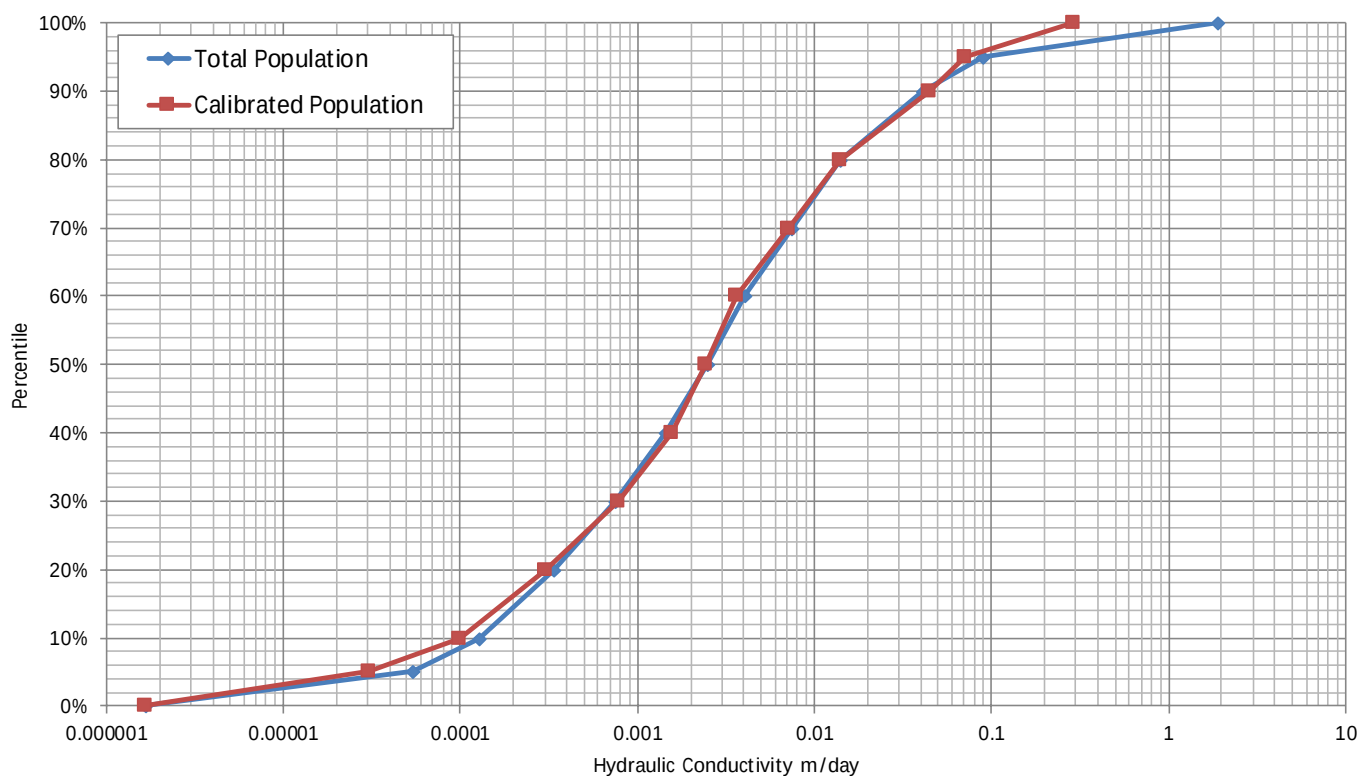


Horizontal Hydraulic Conductivity: Fault - Leichardt

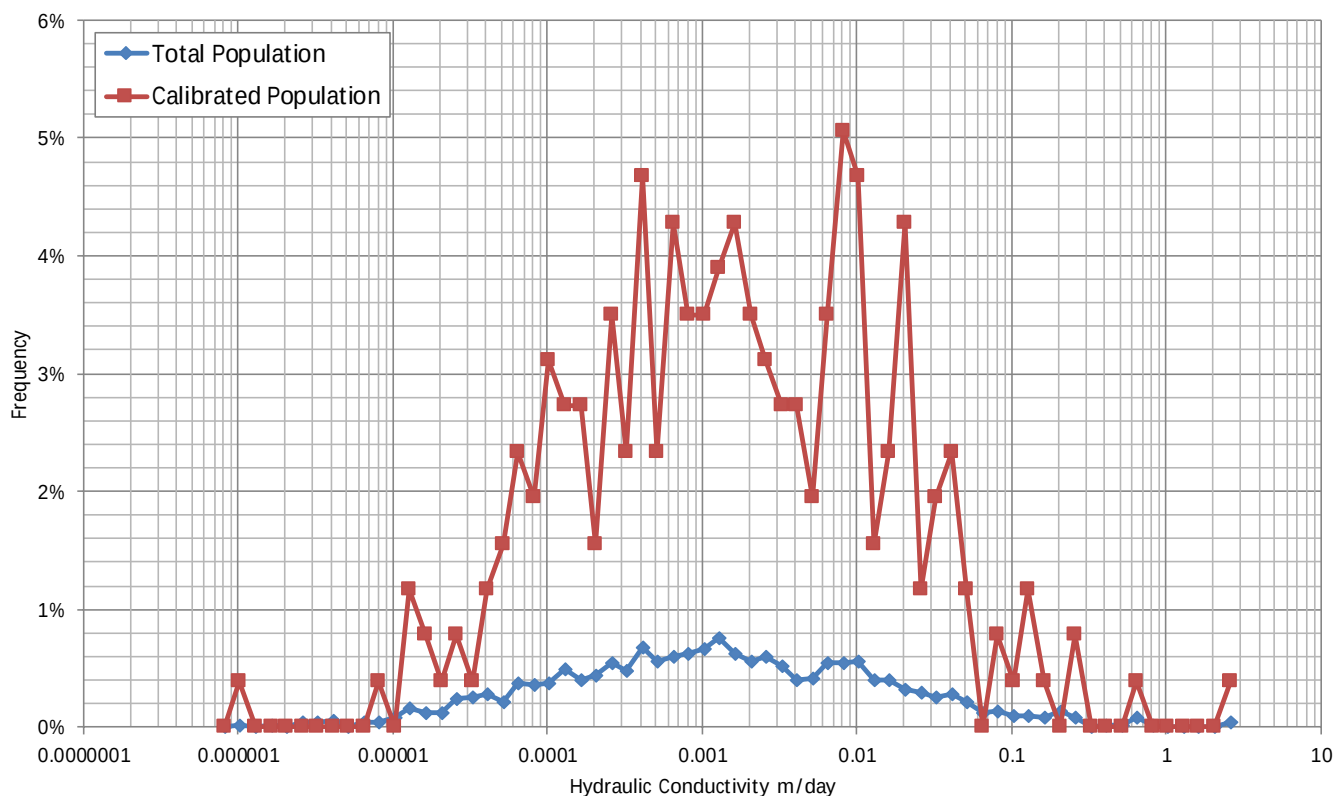


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Fault - Leichardt

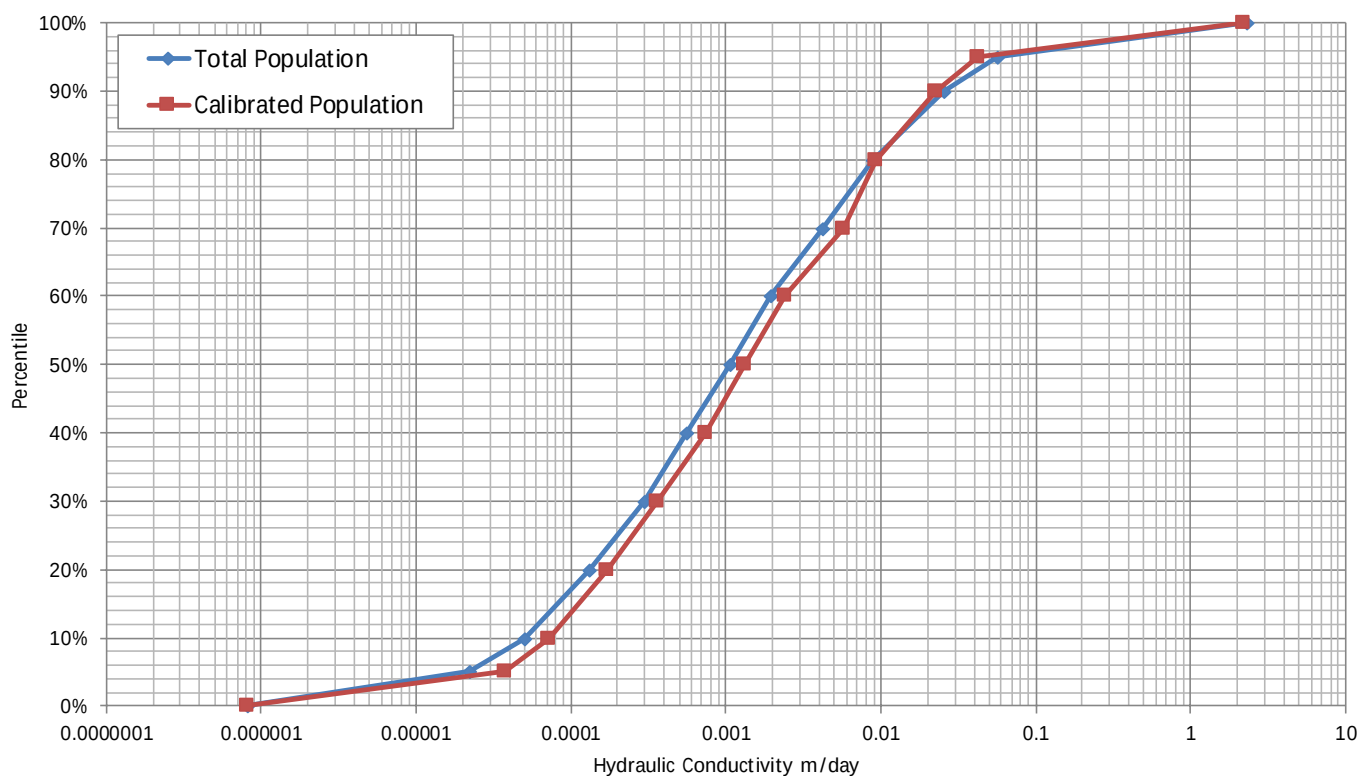


Horizontal Hydraulic Conductivity: Rangal interburden

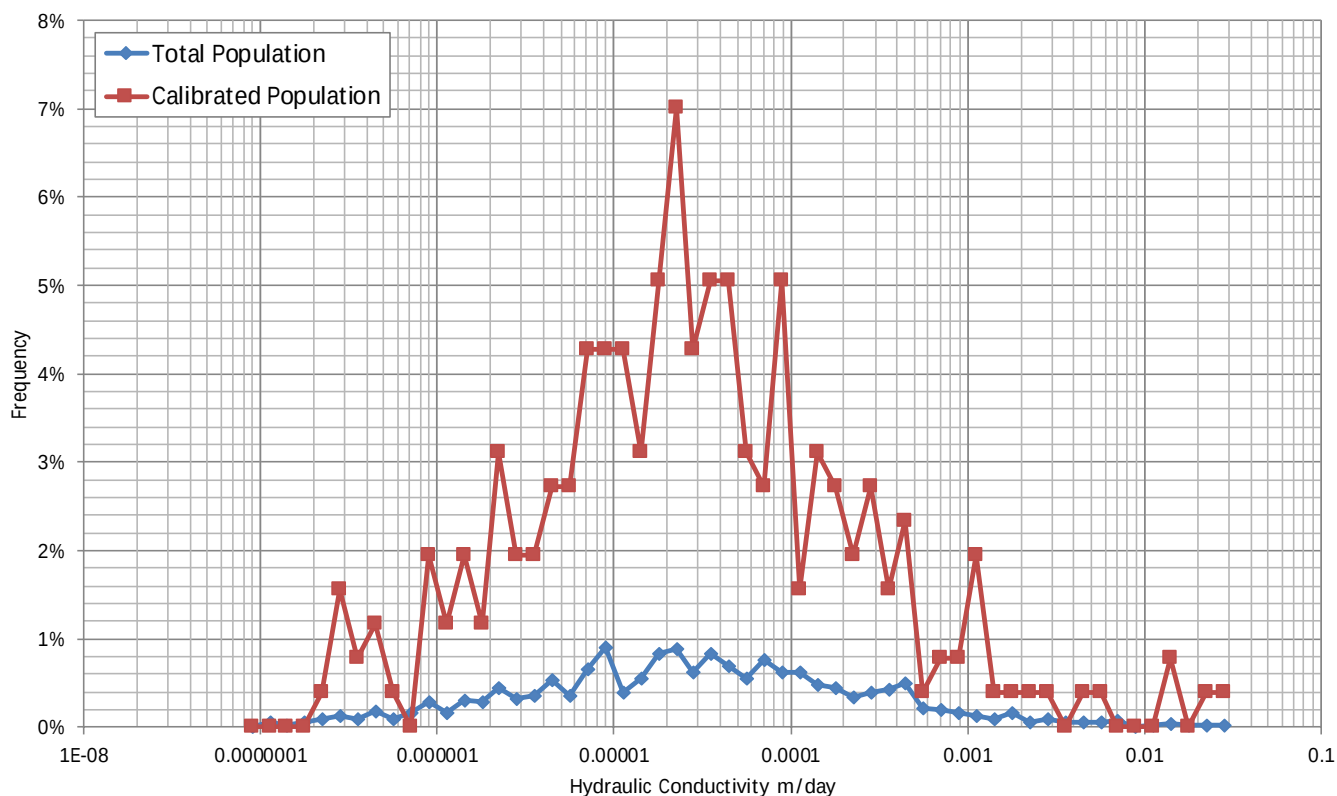


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Rangal interburden

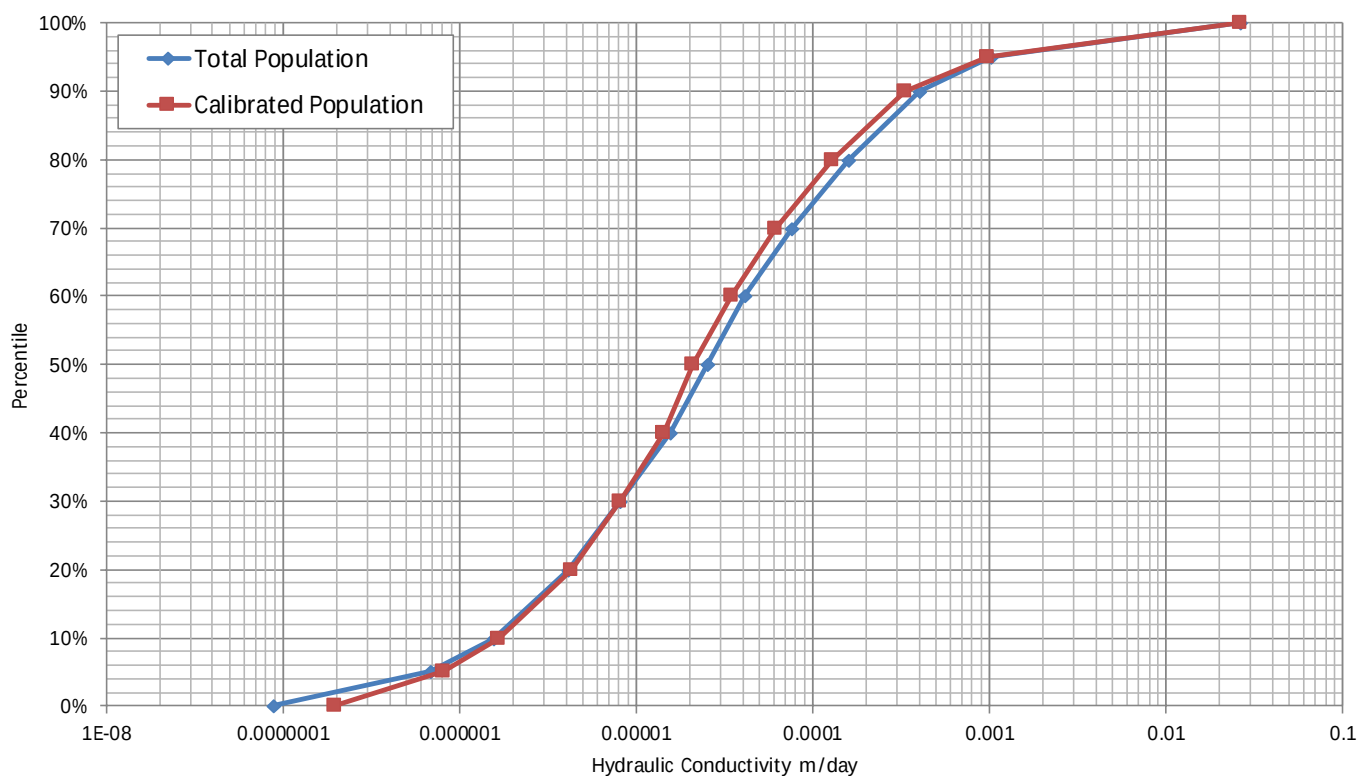


Horizontal Hydraulic Conductivity: Fault - Rangal interburden

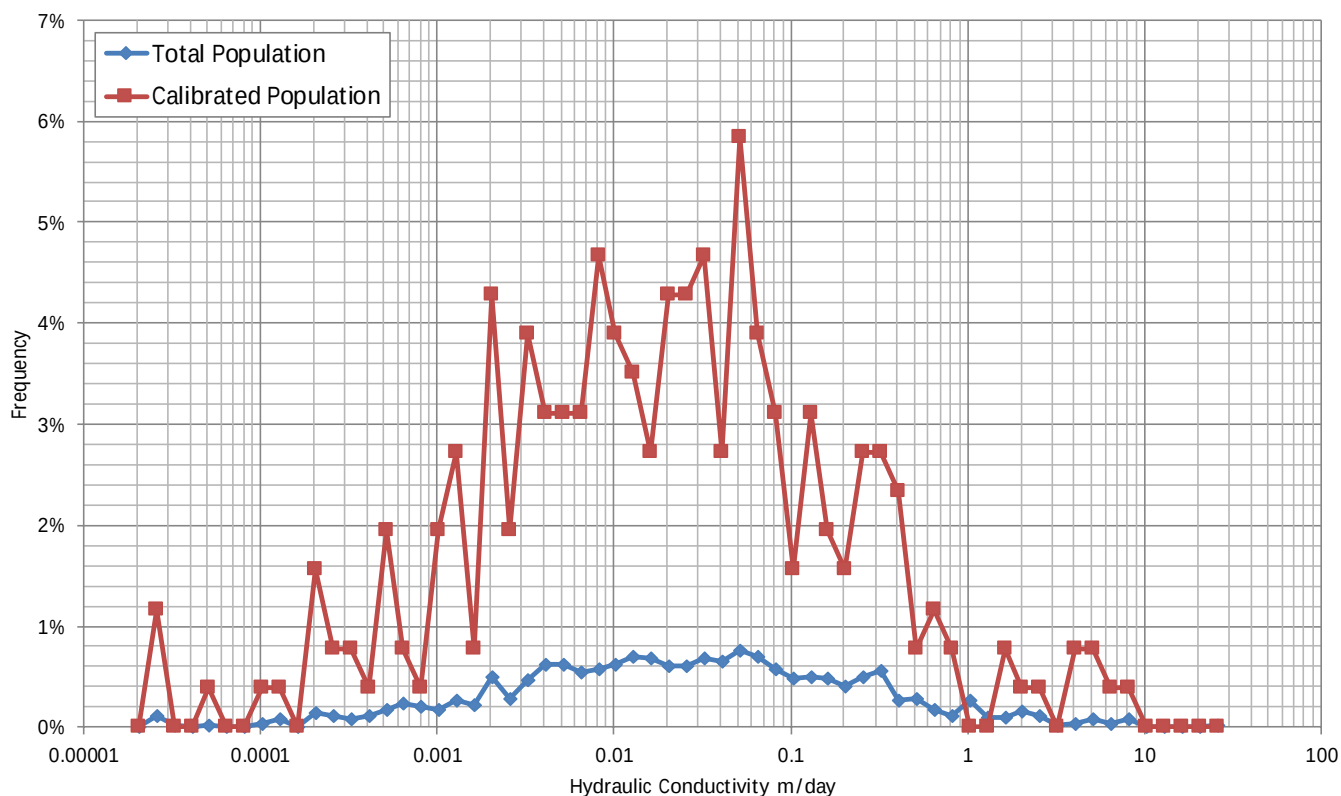


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Fault - Rangal interburden

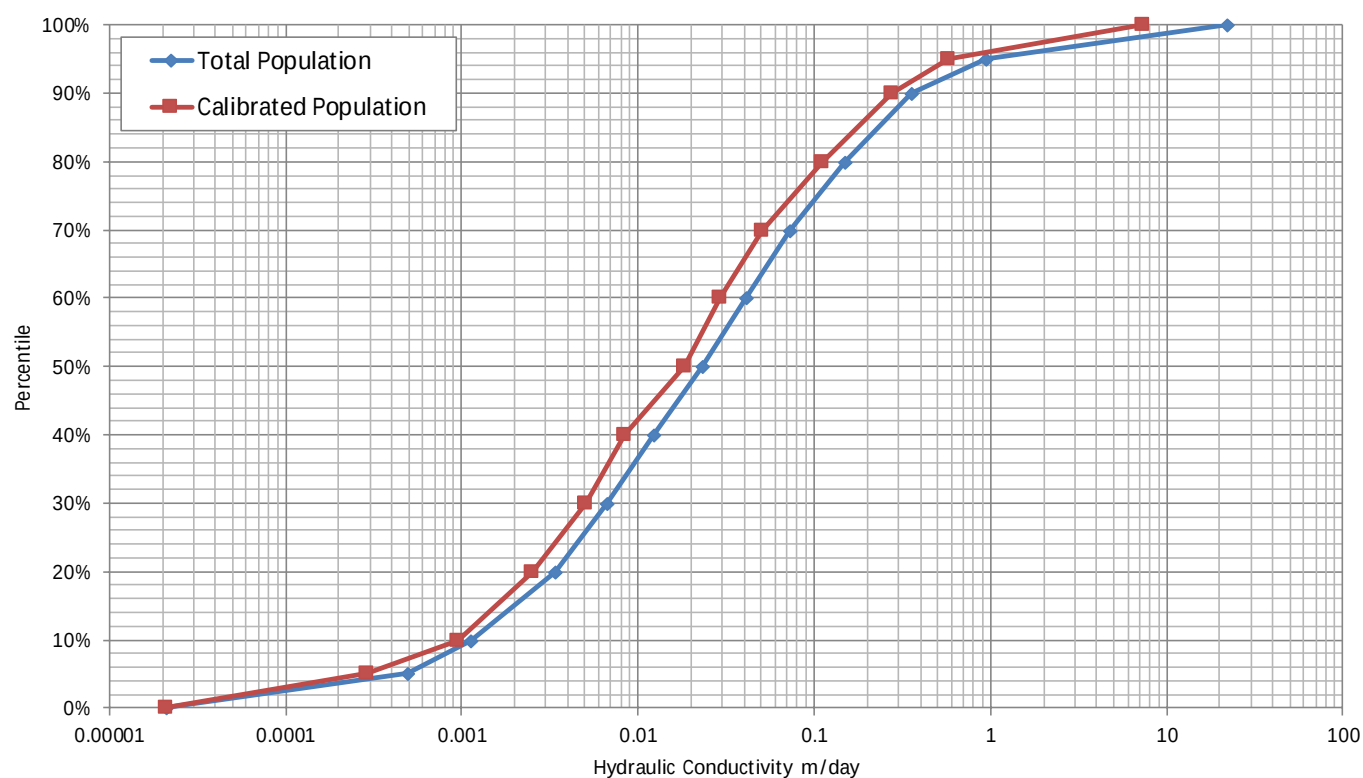


Horizontal Hydraulic Conductivity: Vermont

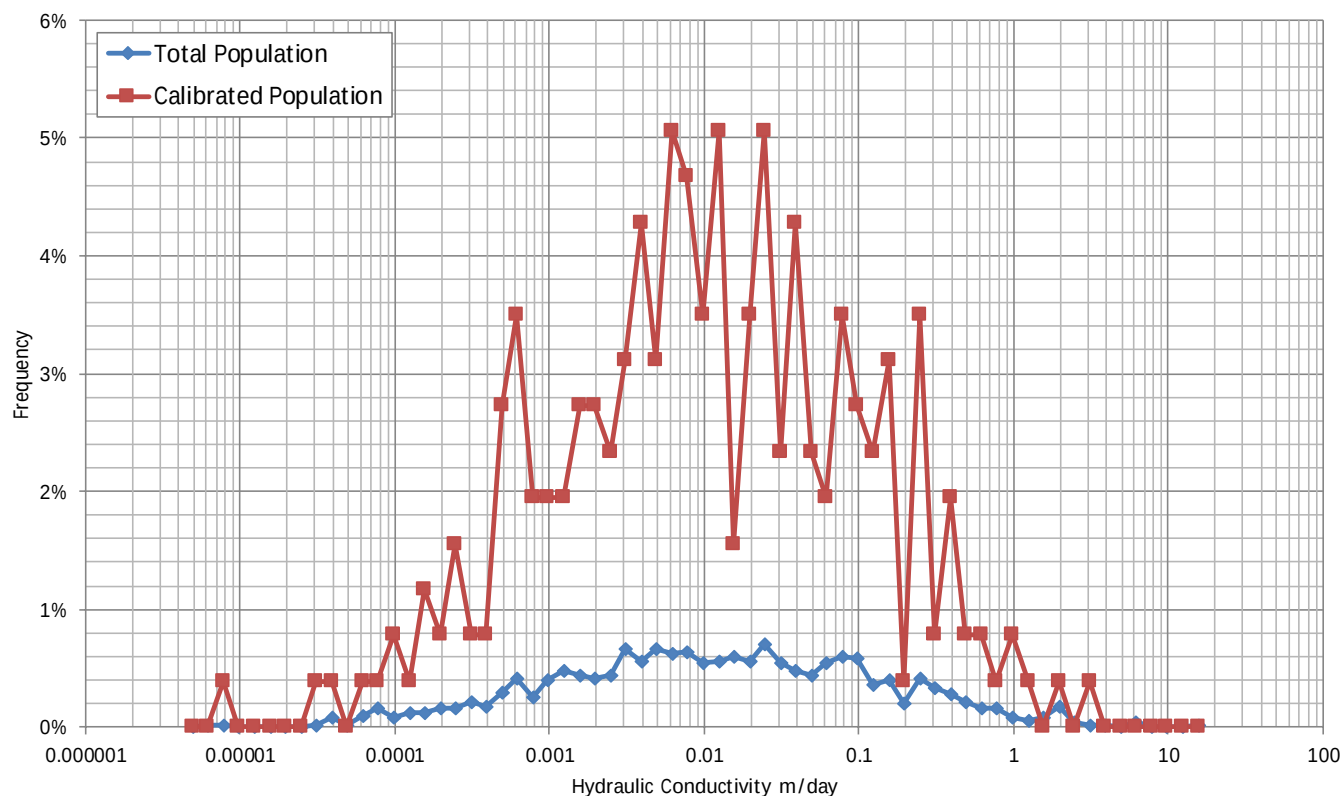


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Vermont

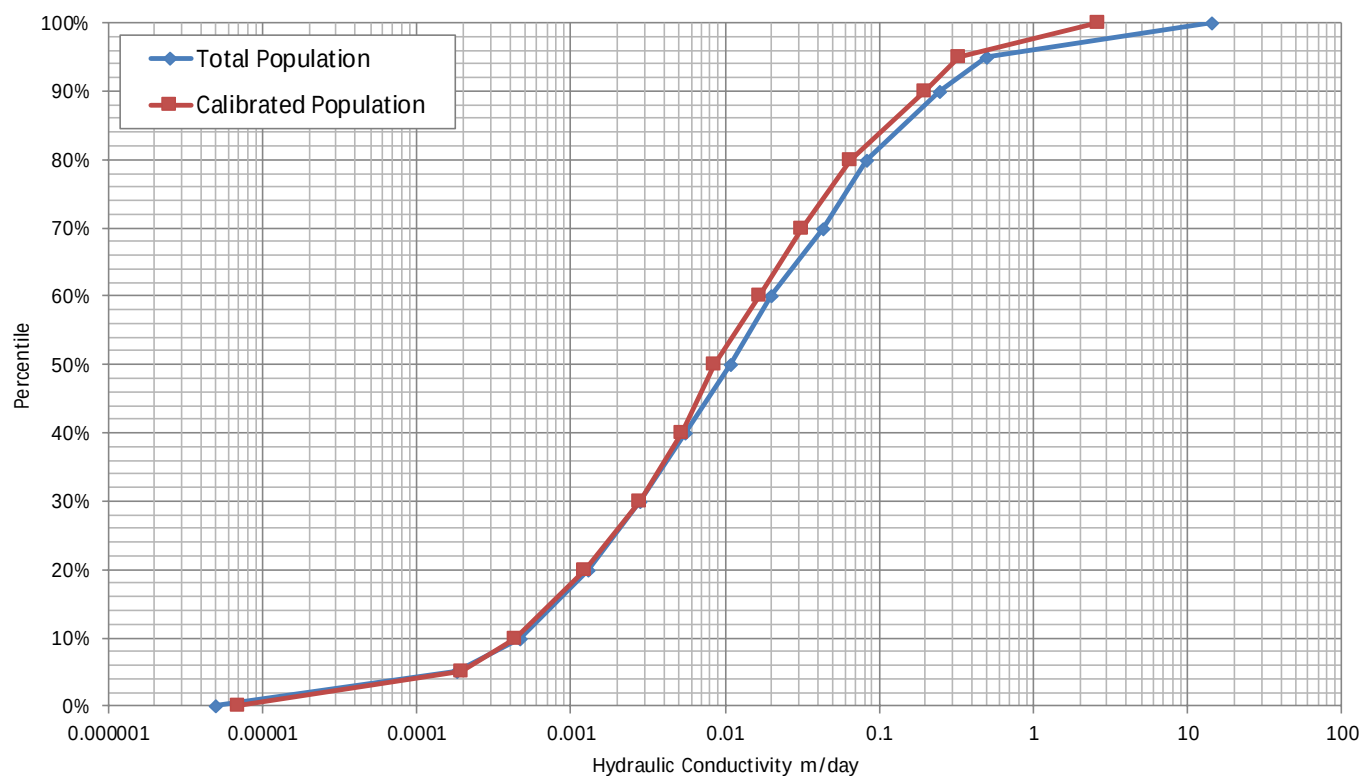


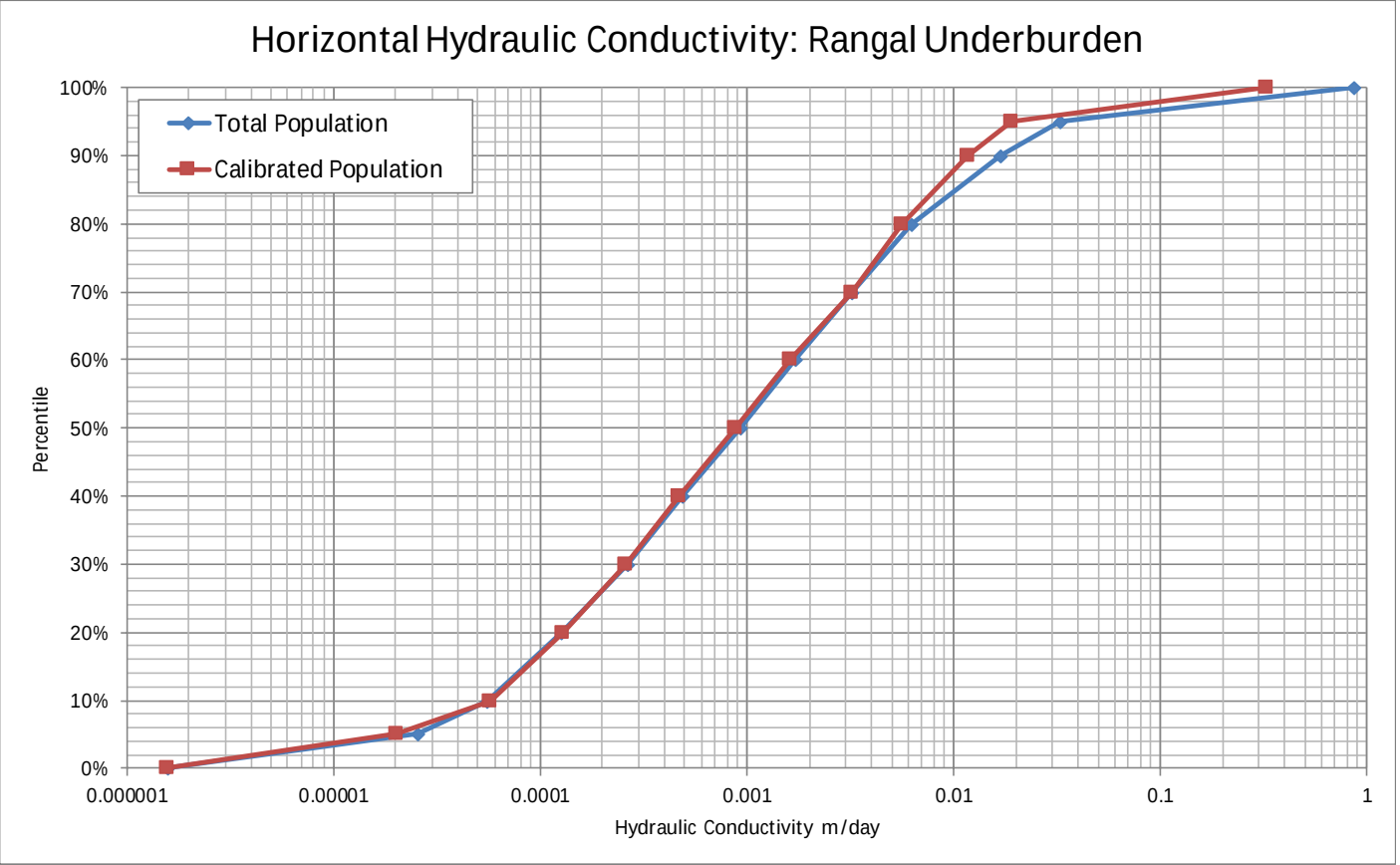
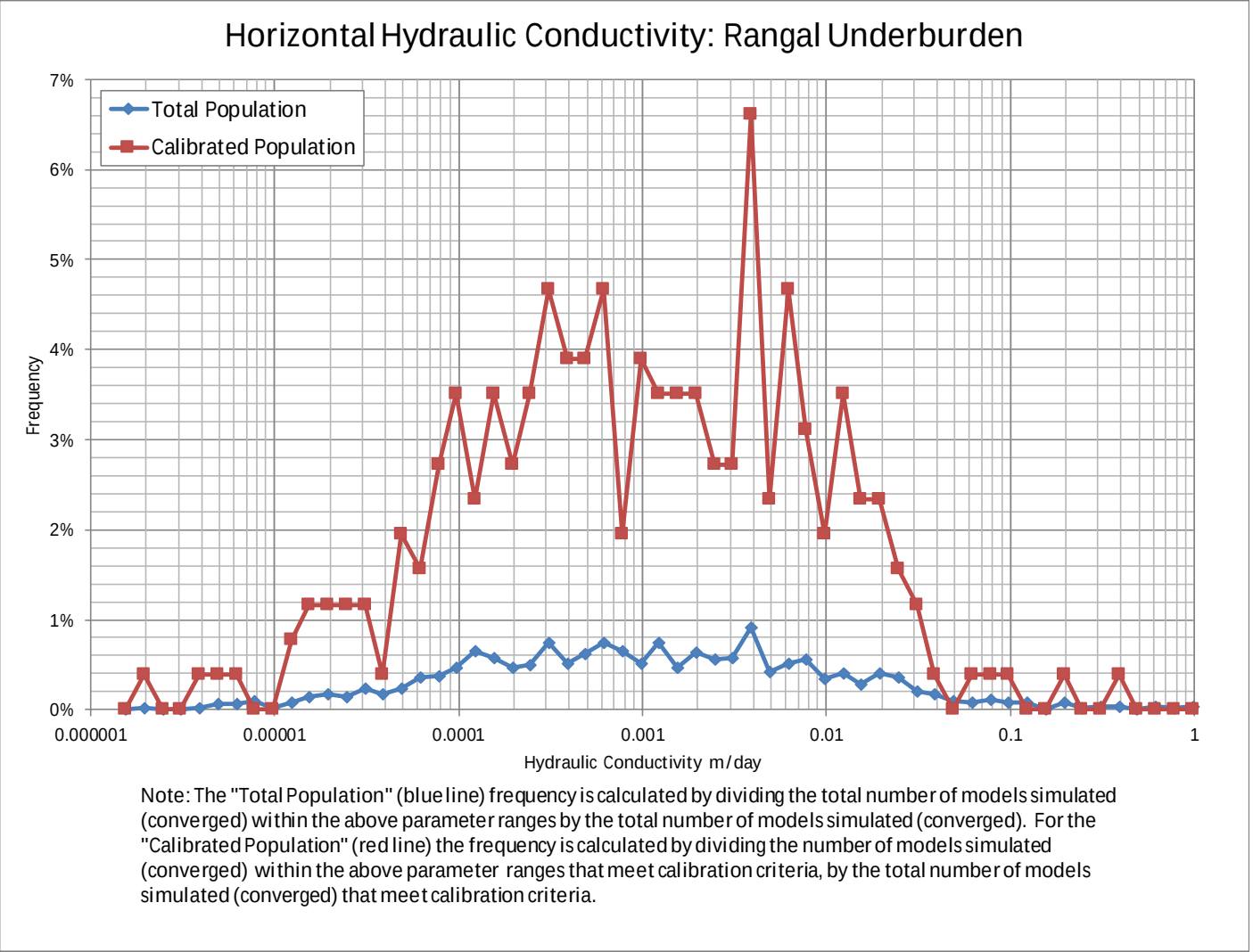
Horizontal Hydraulic Conductivity: Fault - Vermont

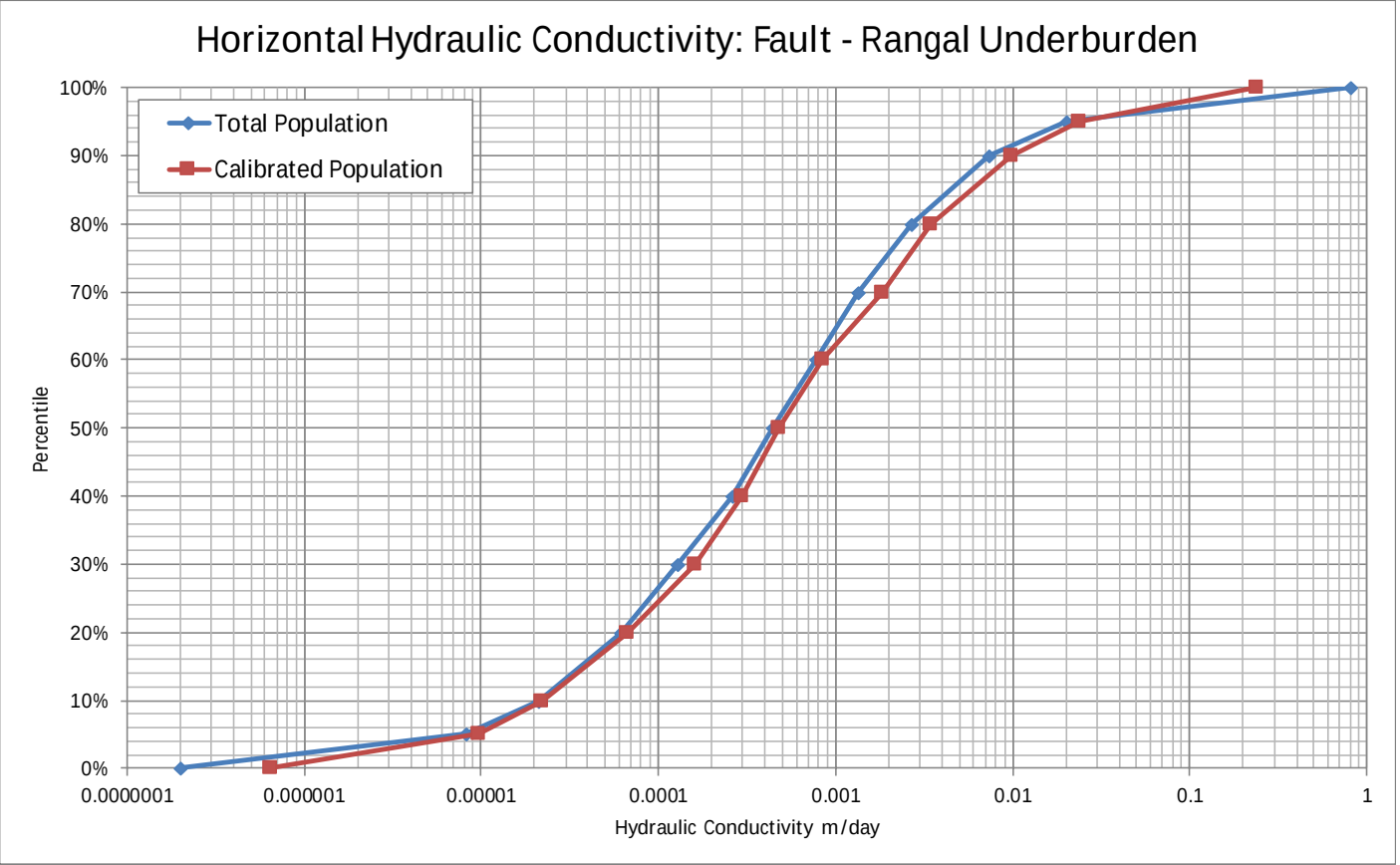
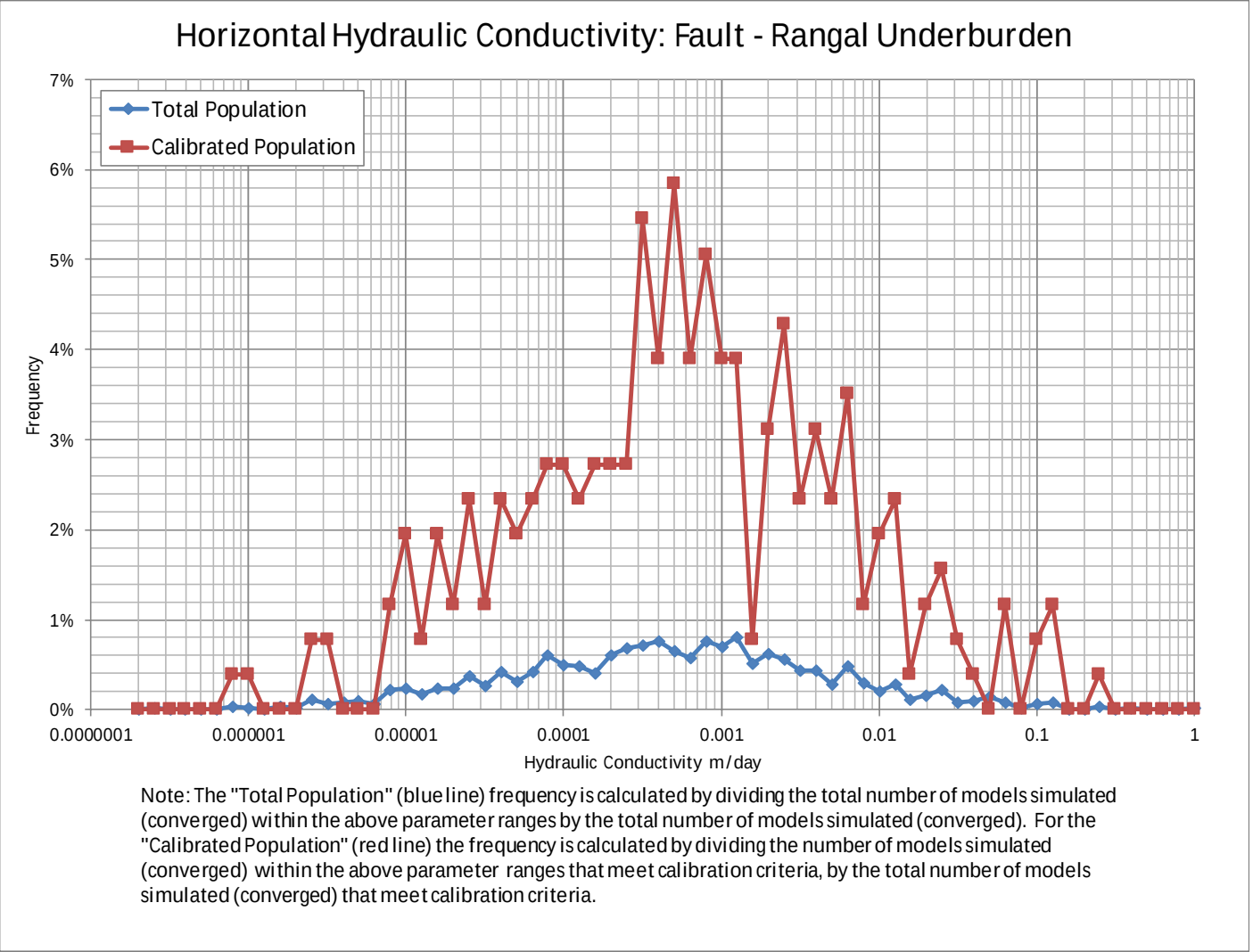


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

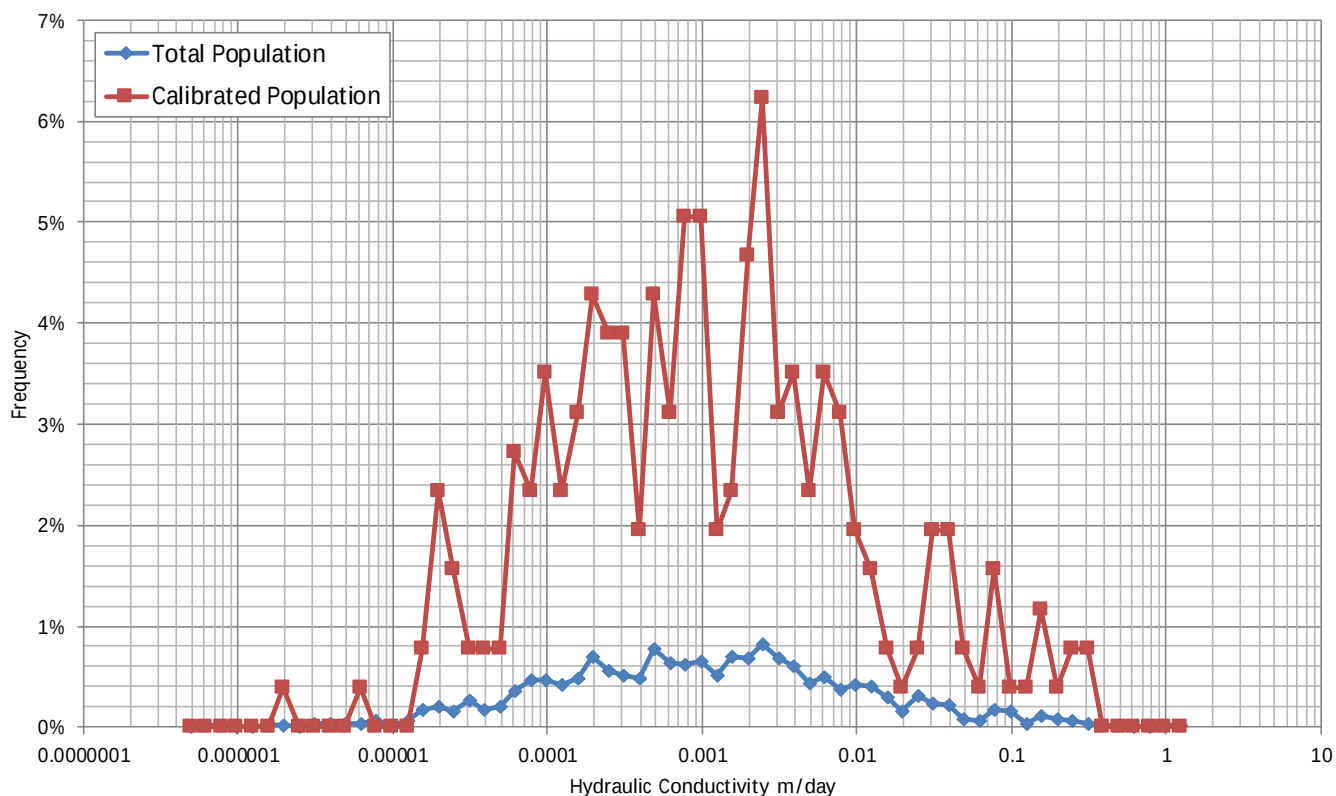
Horizontal Hydraulic Conductivity: Fault - Vermont





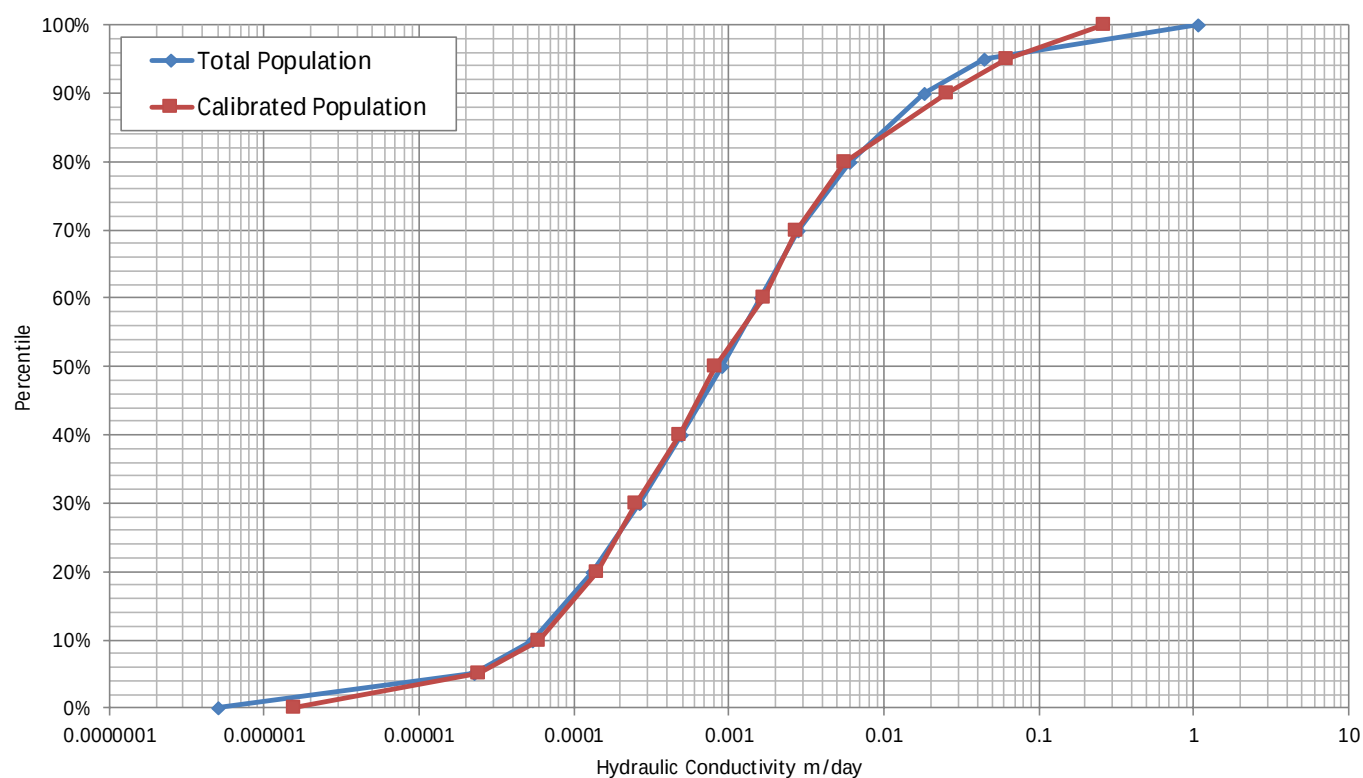


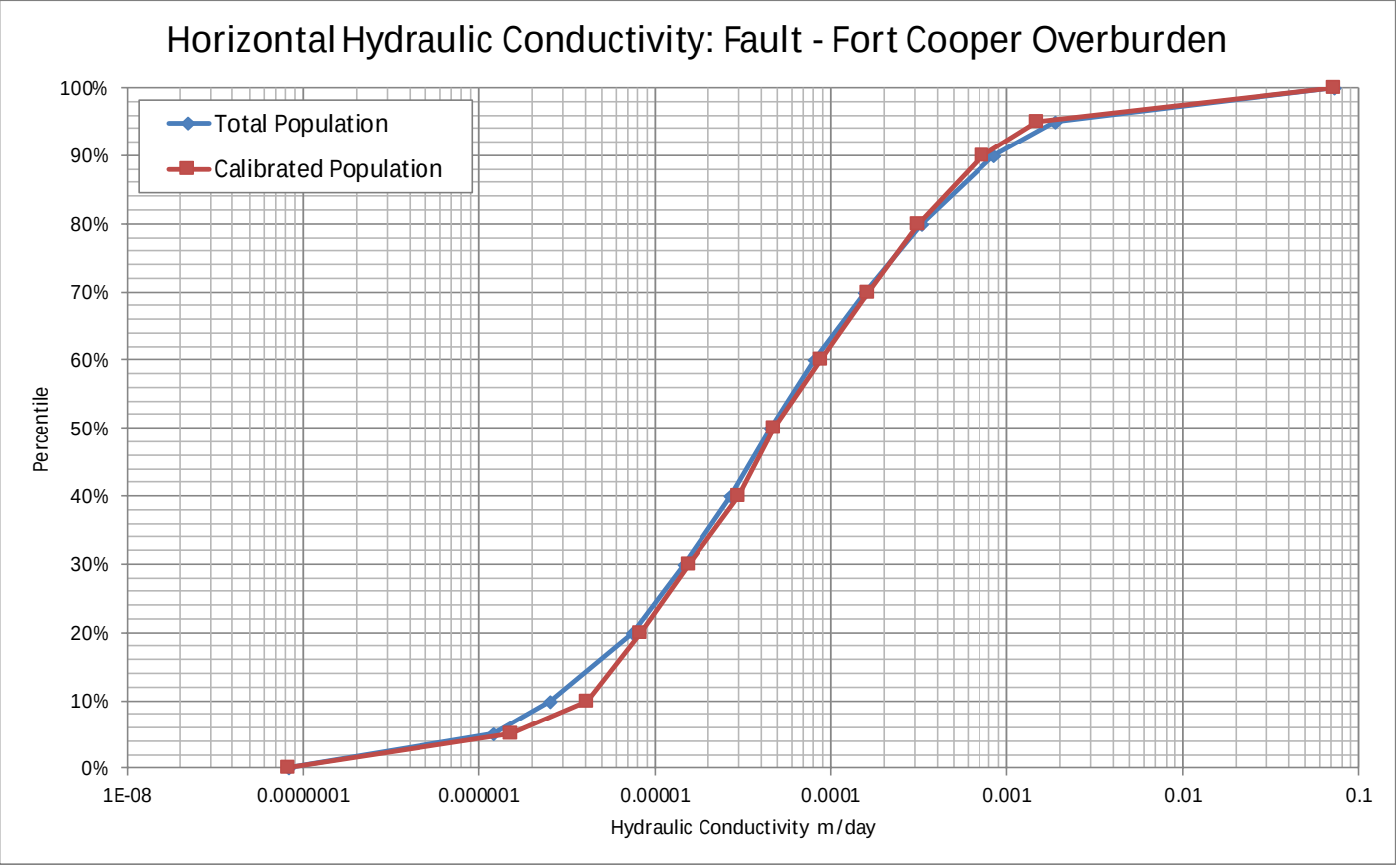
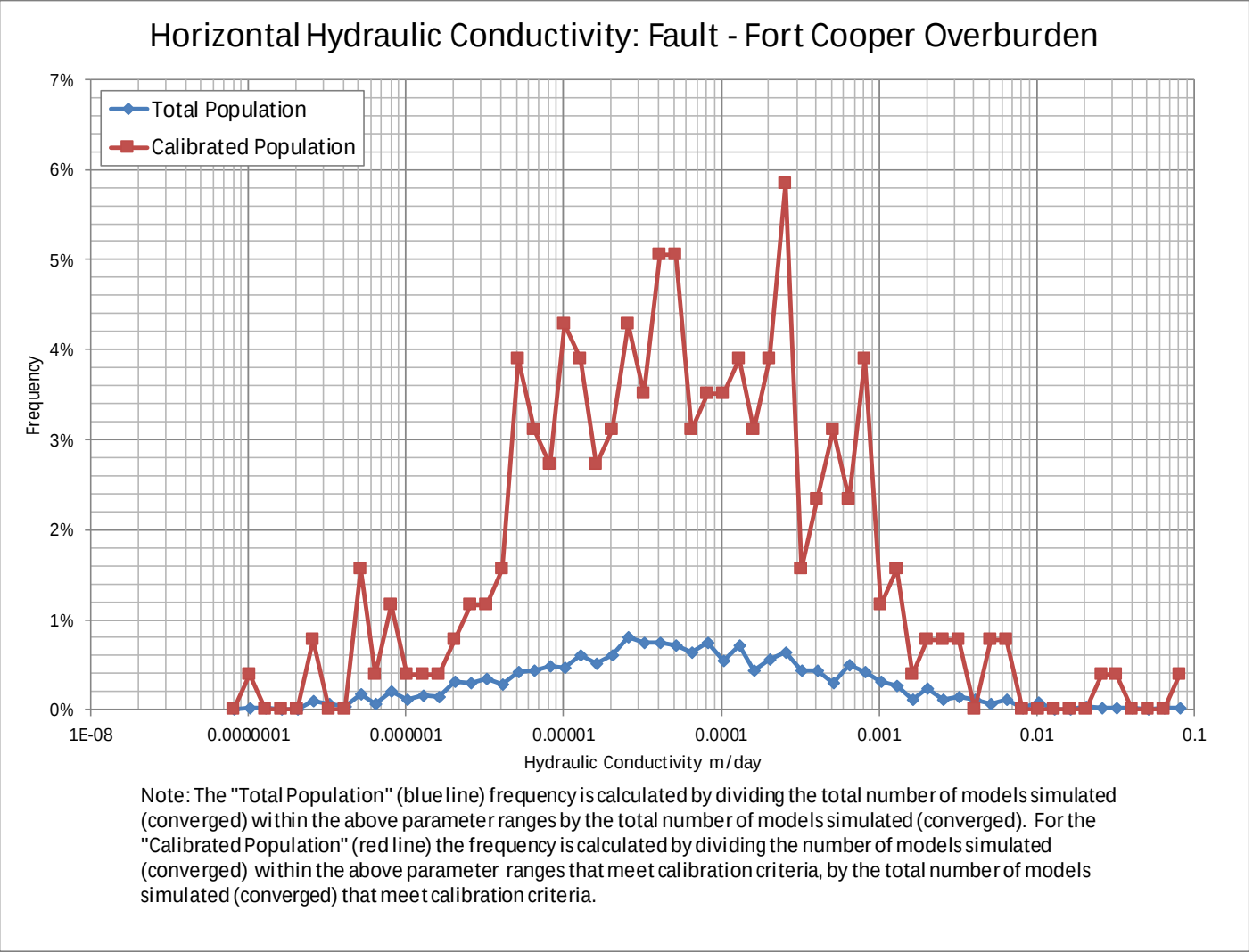
Horizontal Hydraulic Conductivity: Fort Cooper Overburden

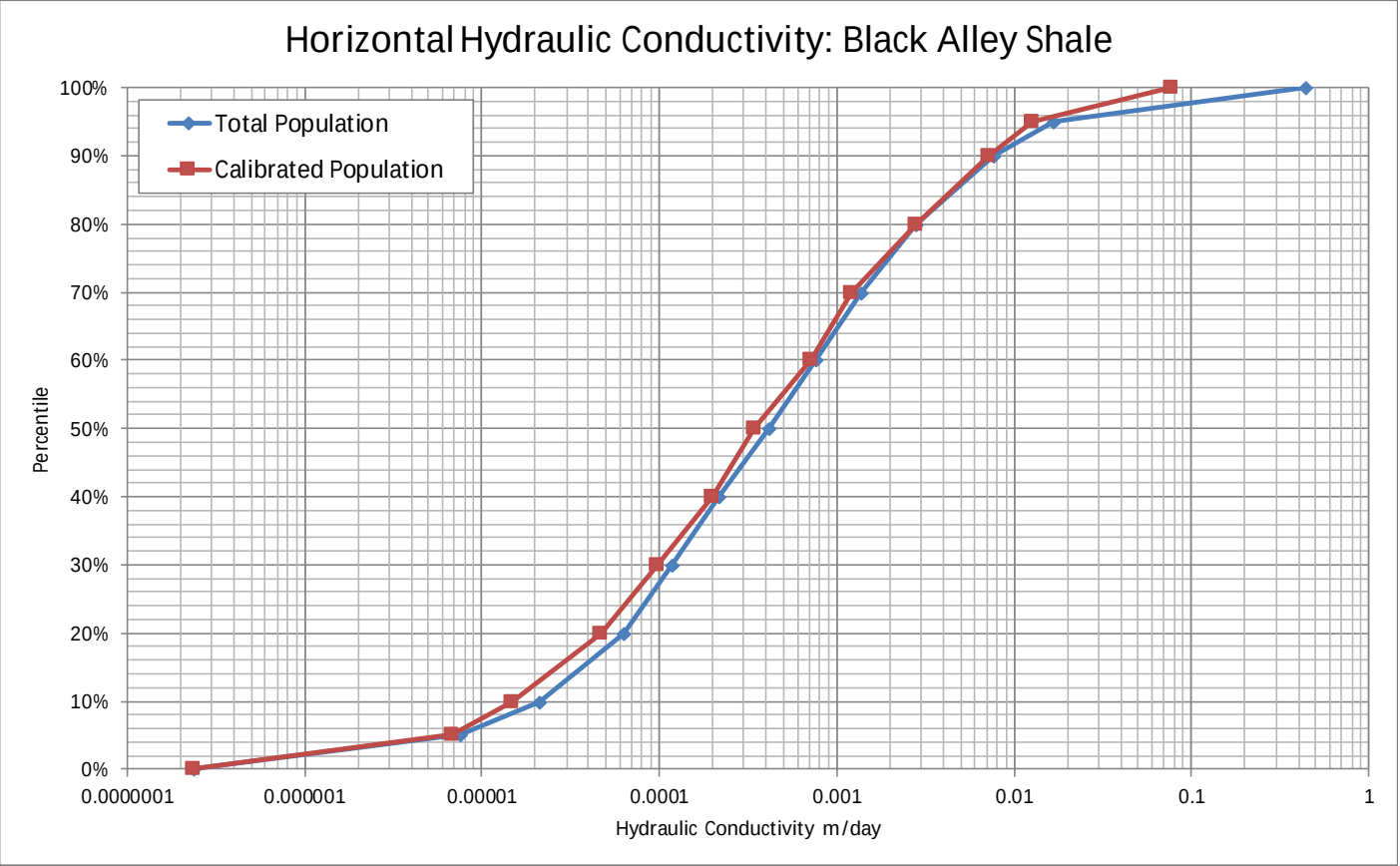
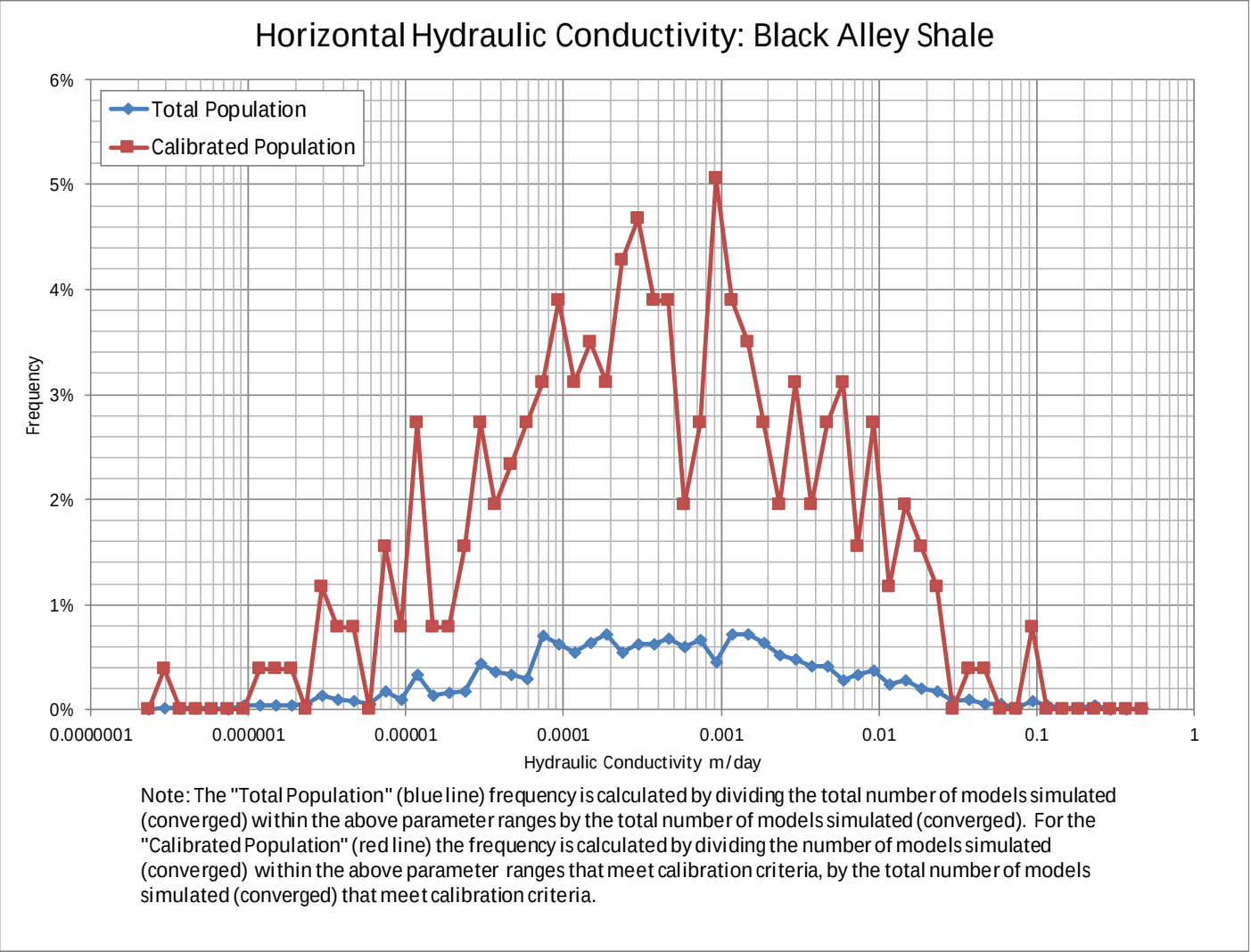


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

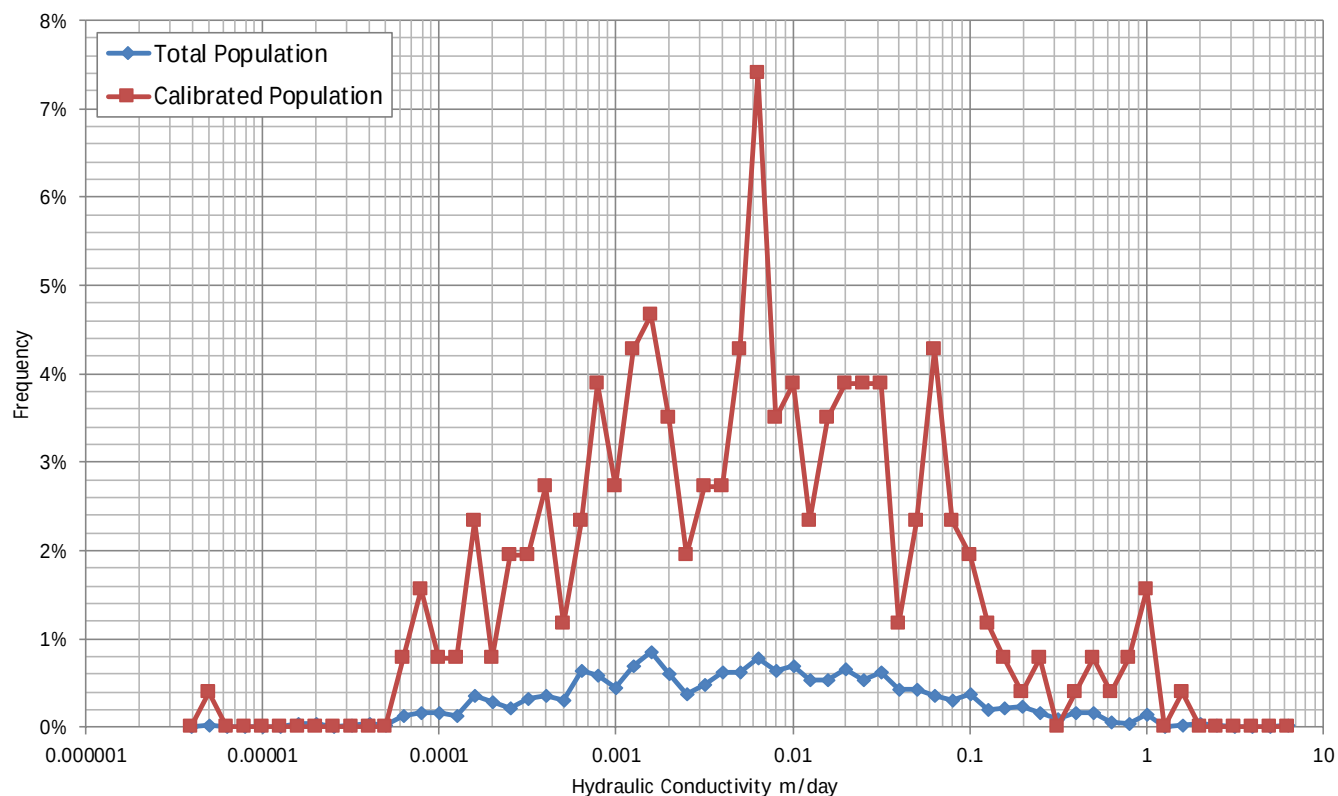
Horizontal Hydraulic Conductivity: Fort Cooper Overburden





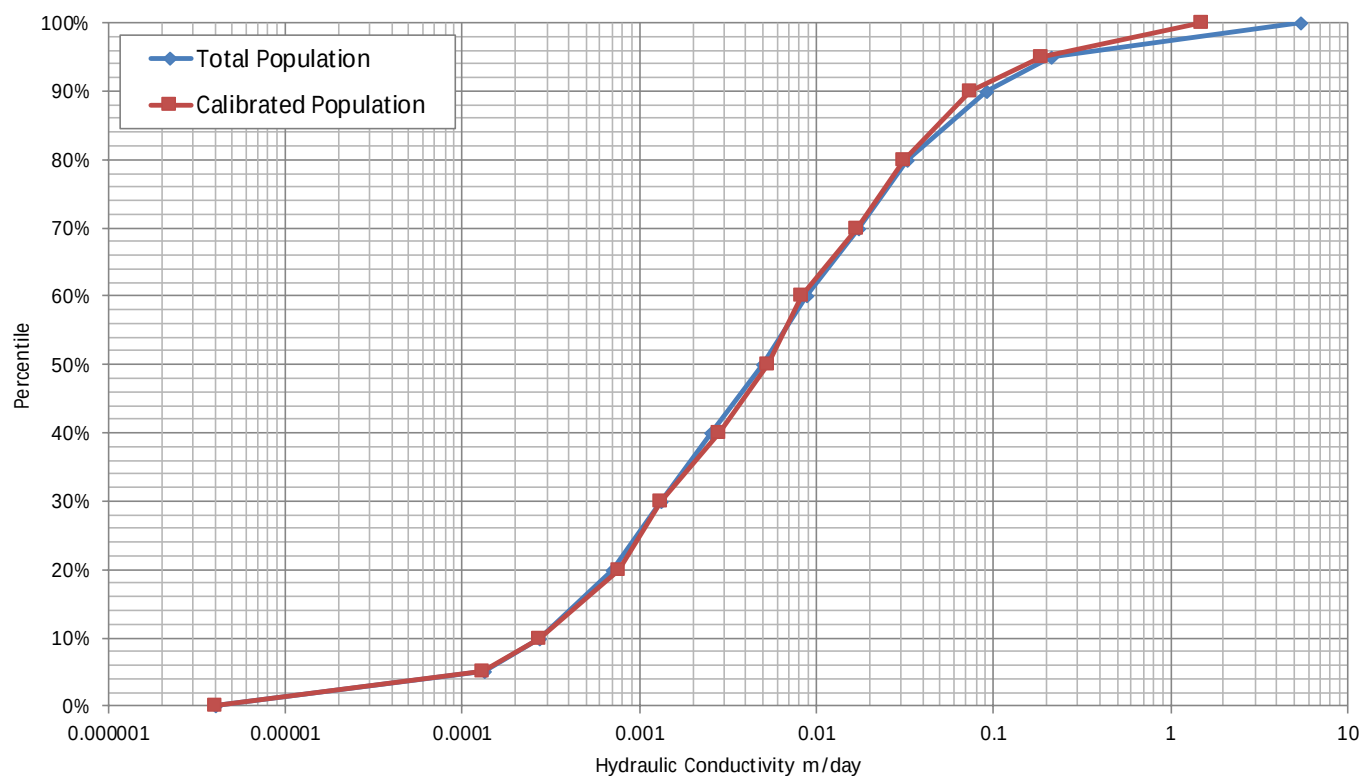


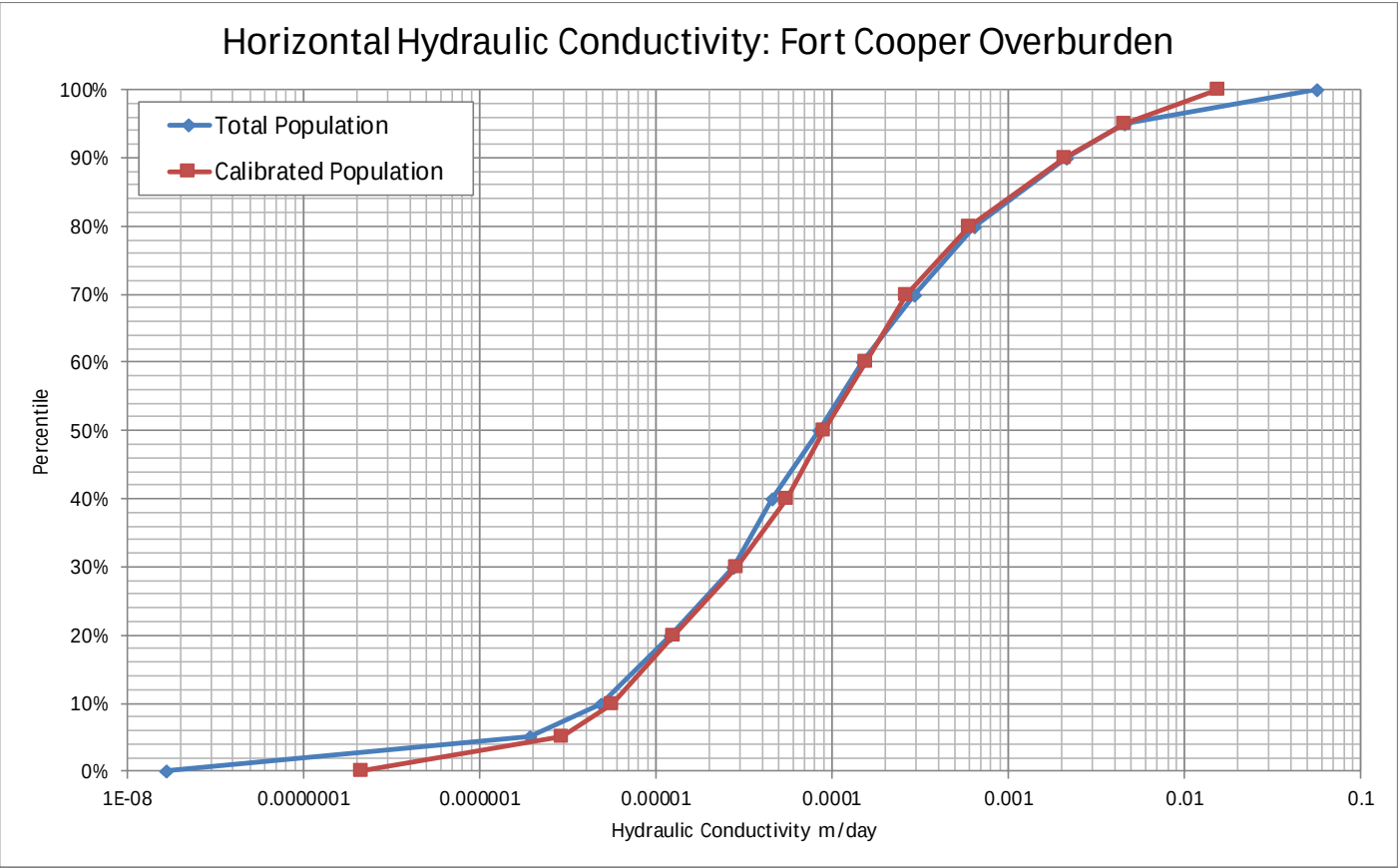
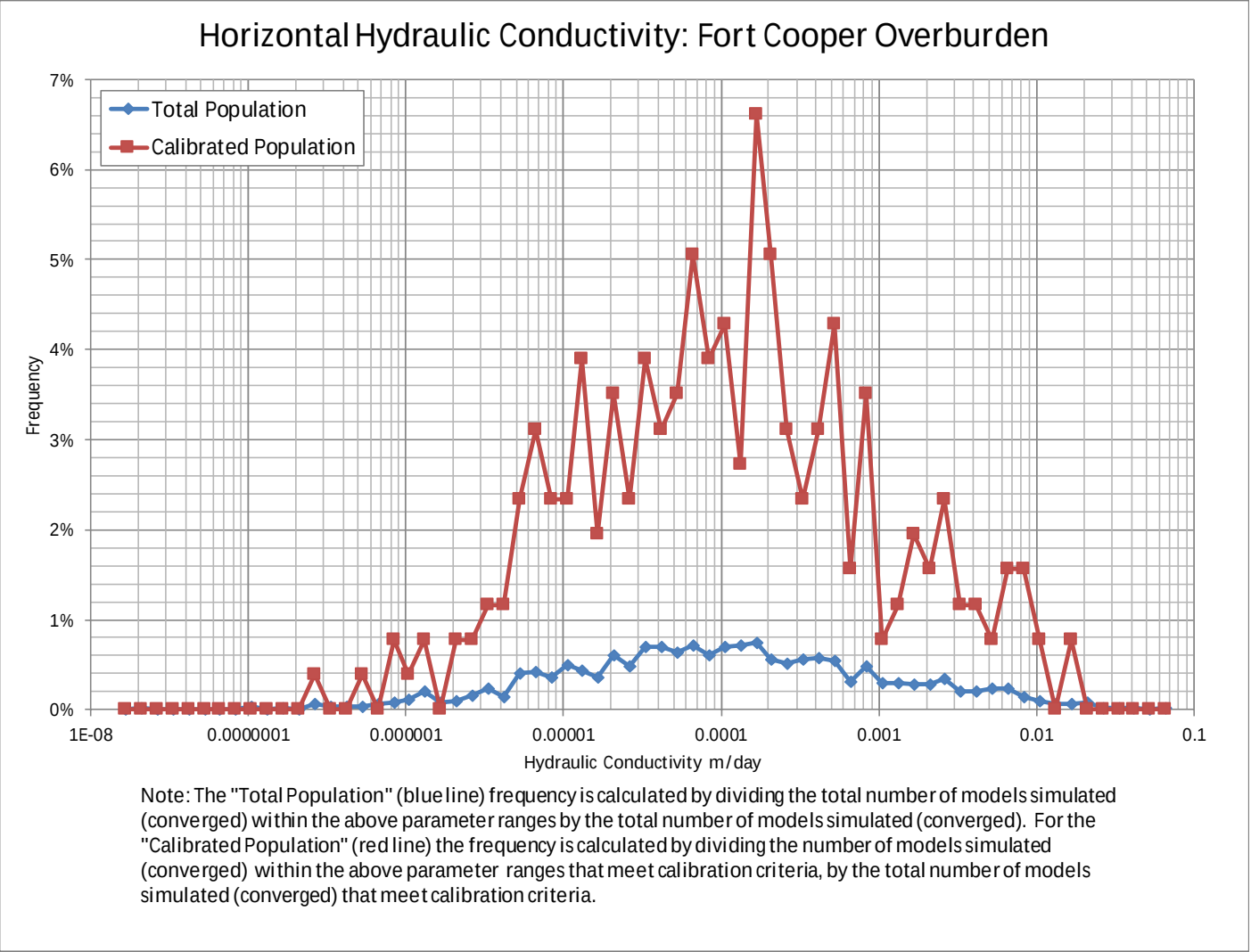
Horizontal Hydraulic Conductivity: Fault - Black Alley Shale

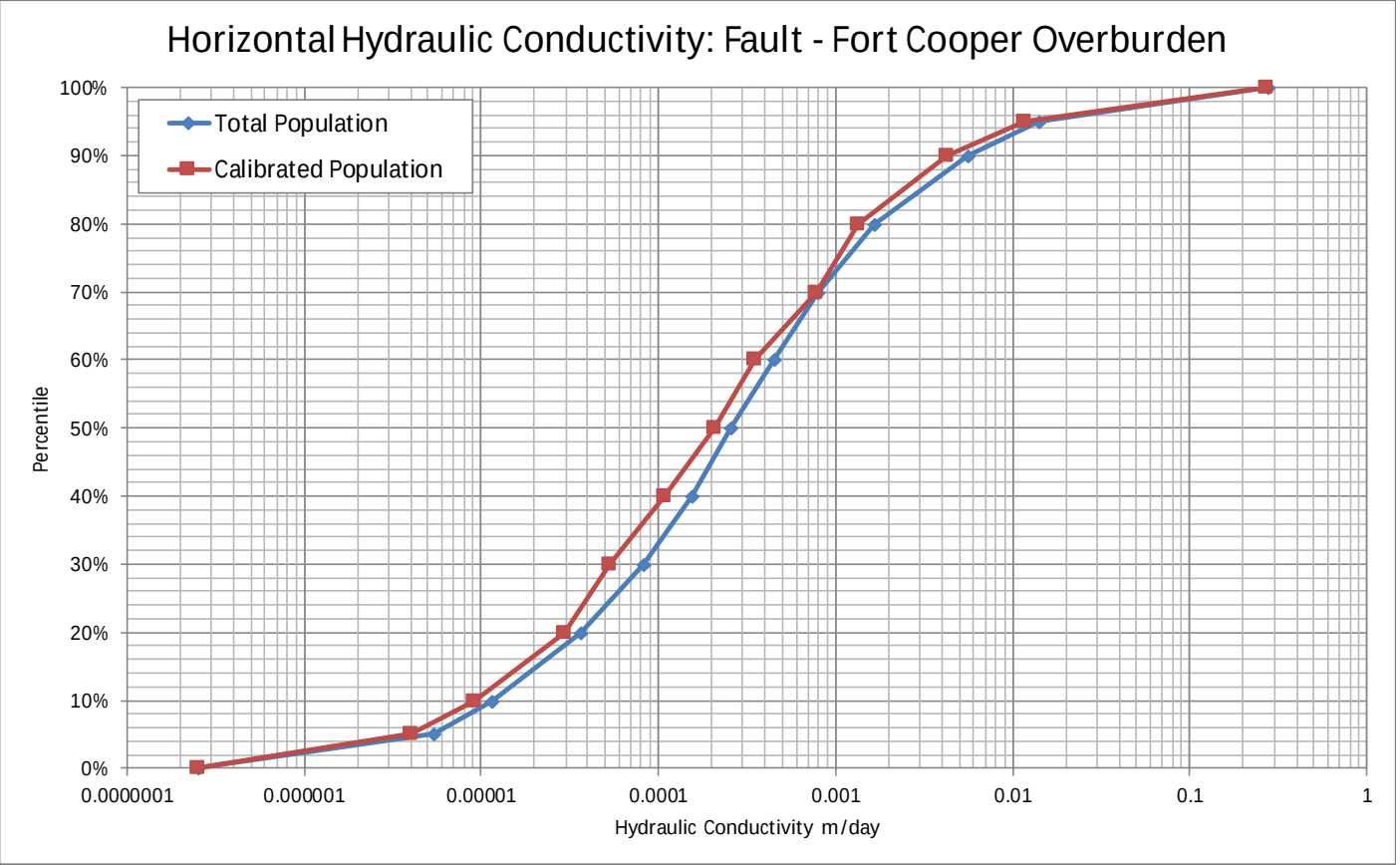
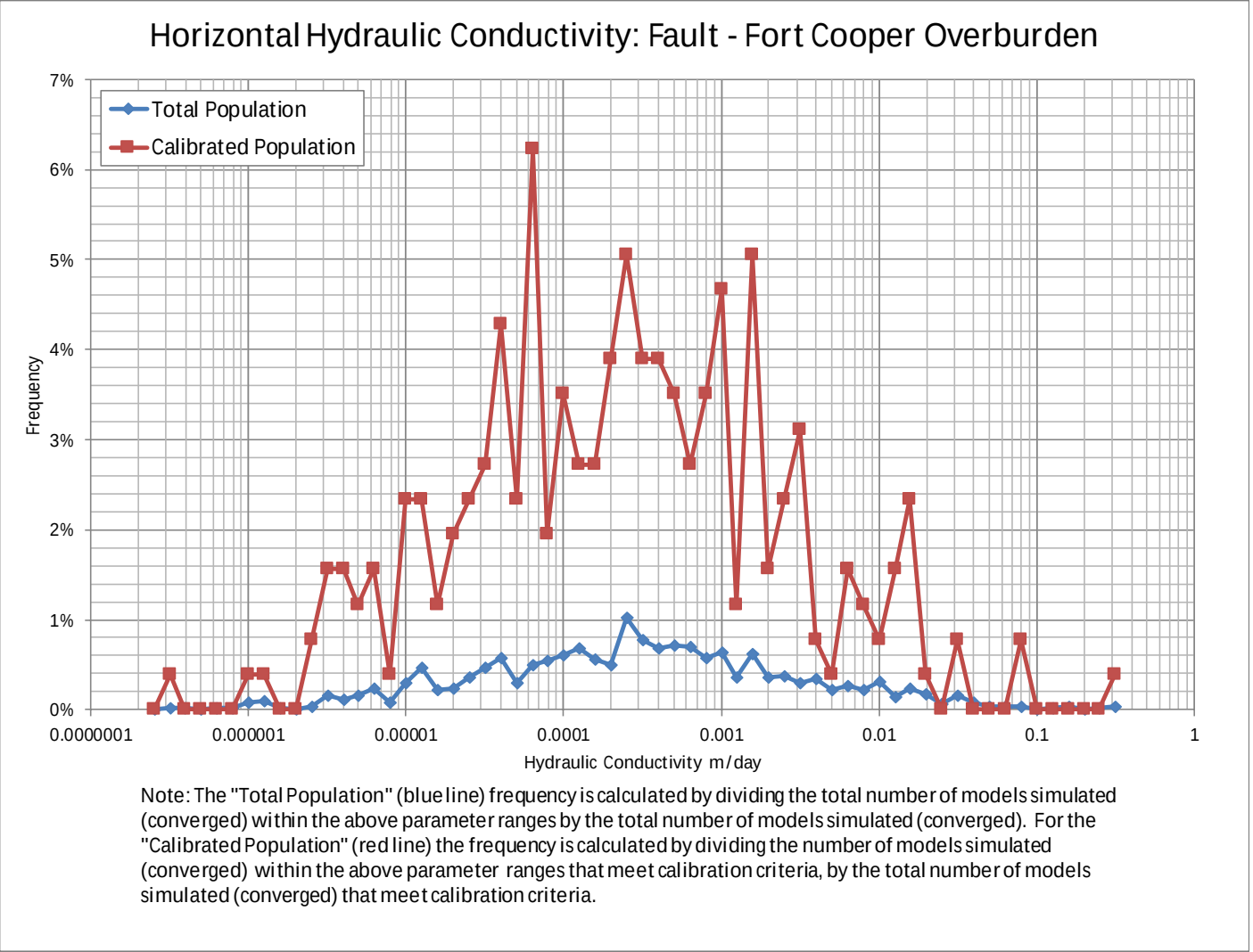


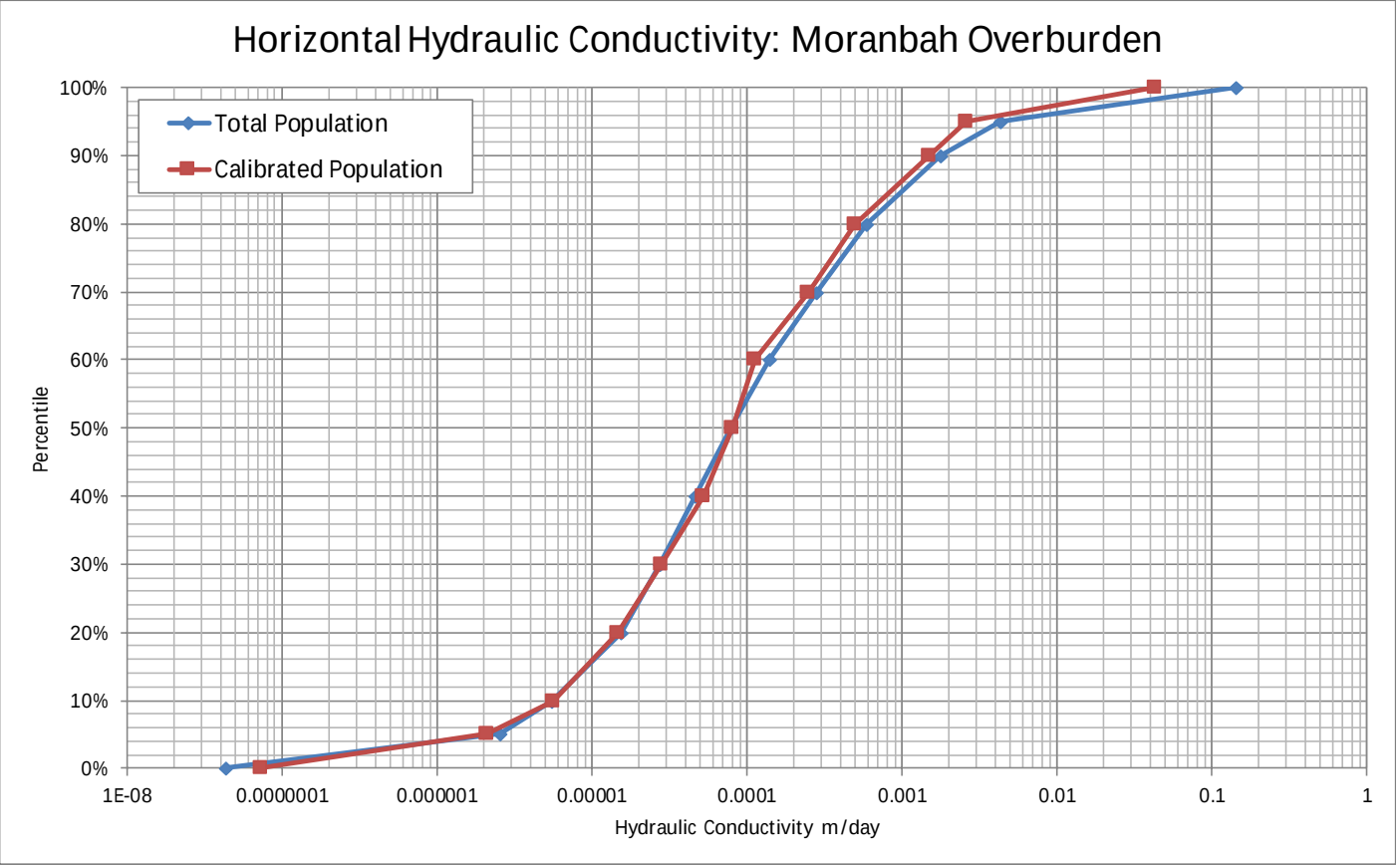
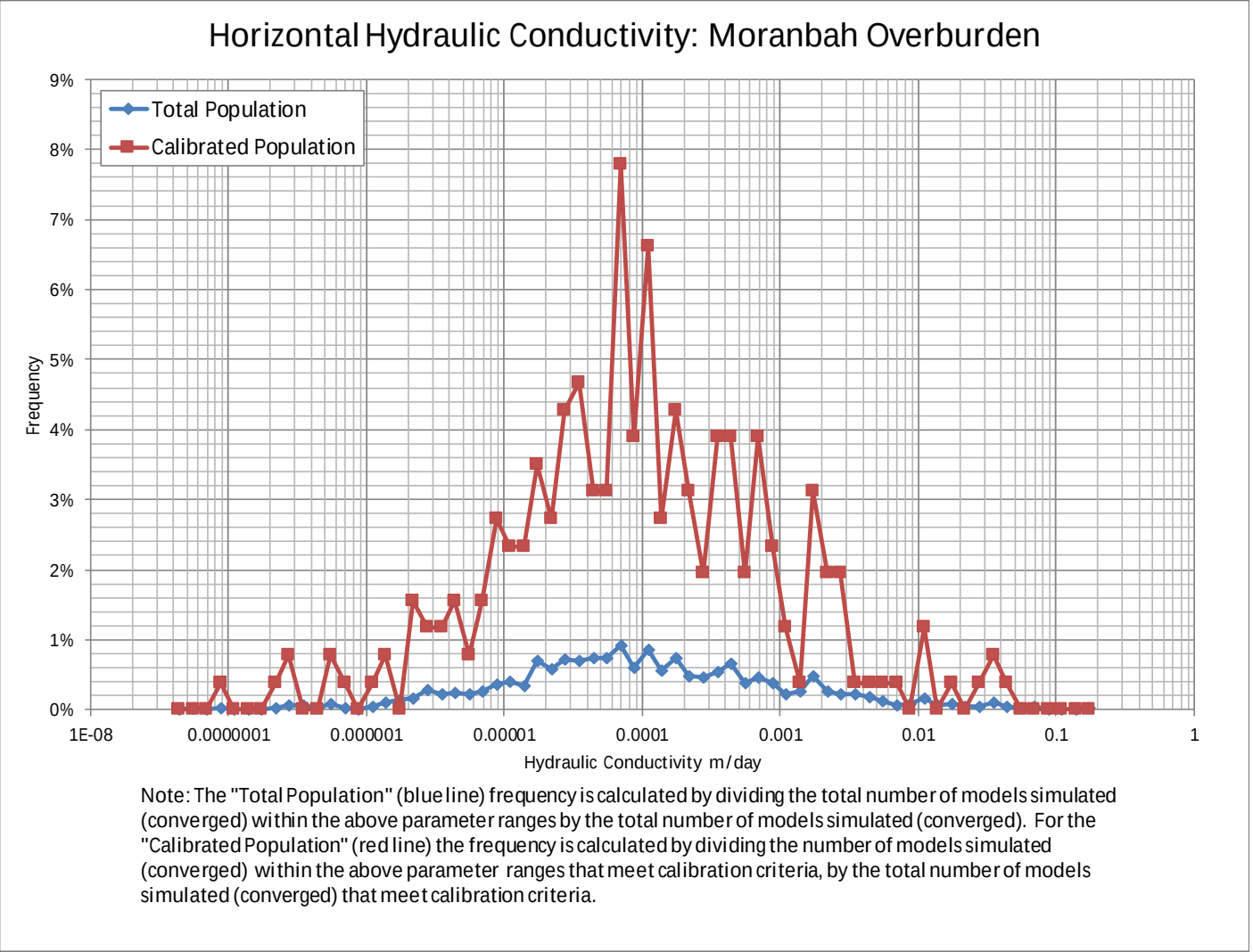
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

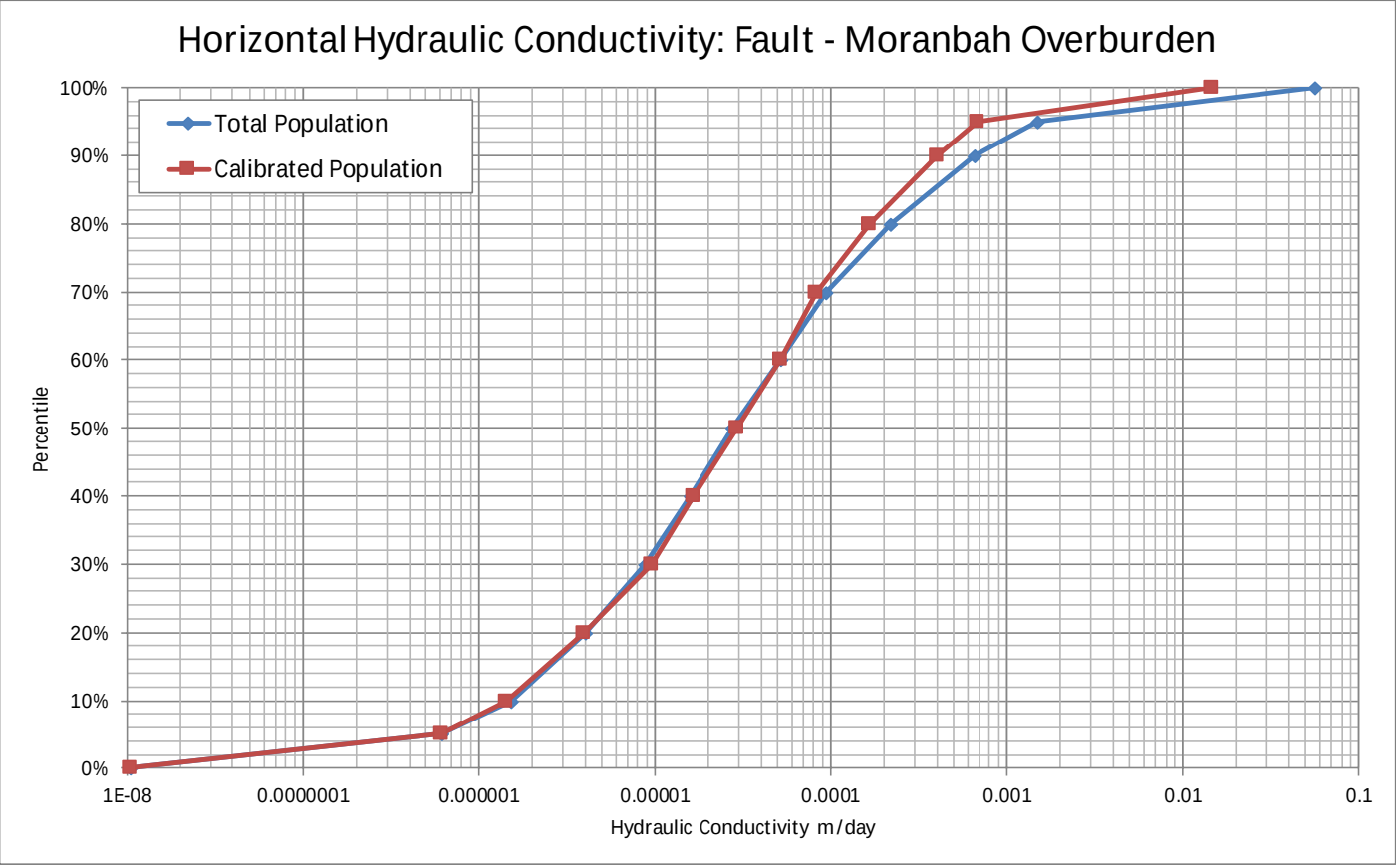
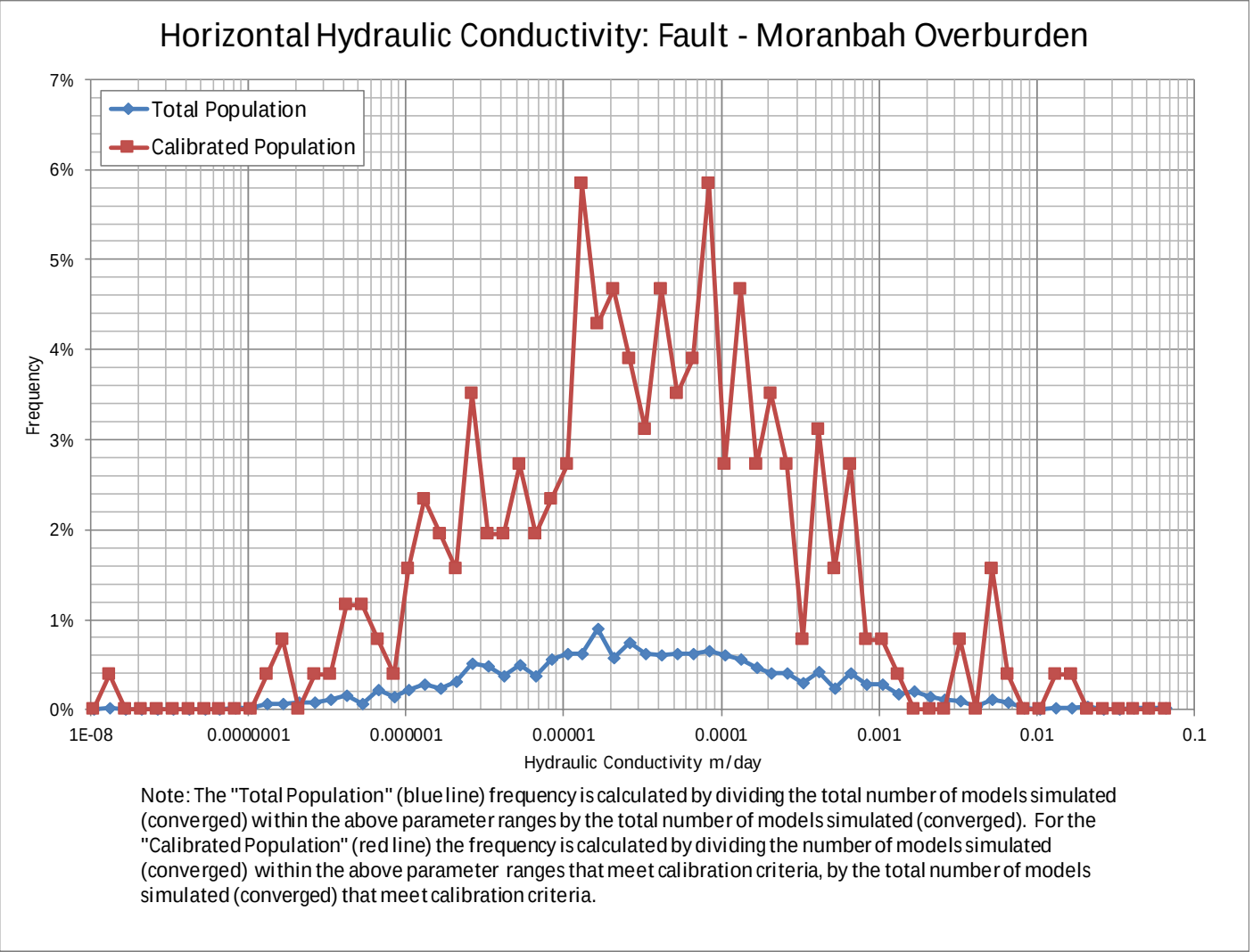
Horizontal Hydraulic Conductivity: Fault - Black Alley Shale

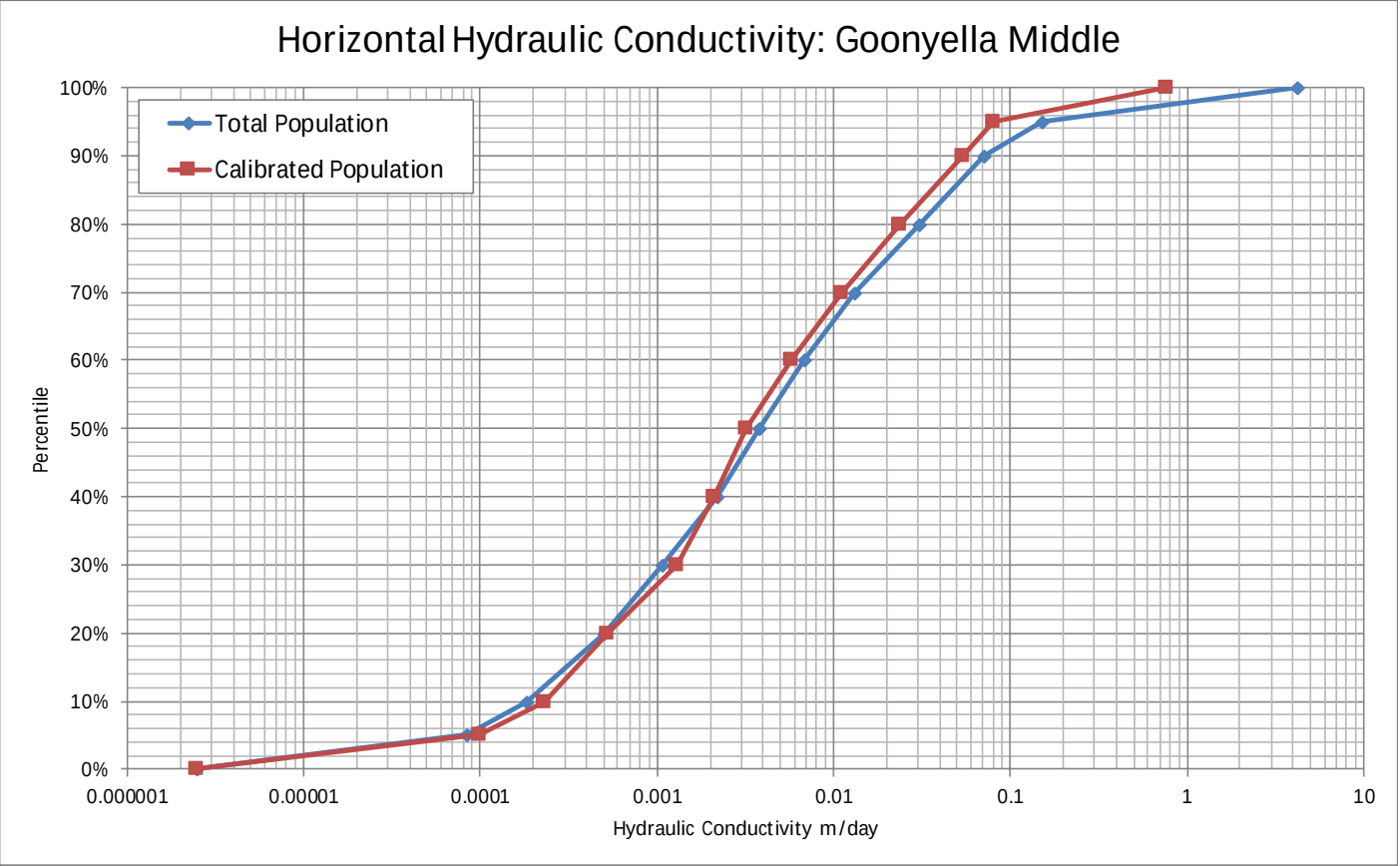
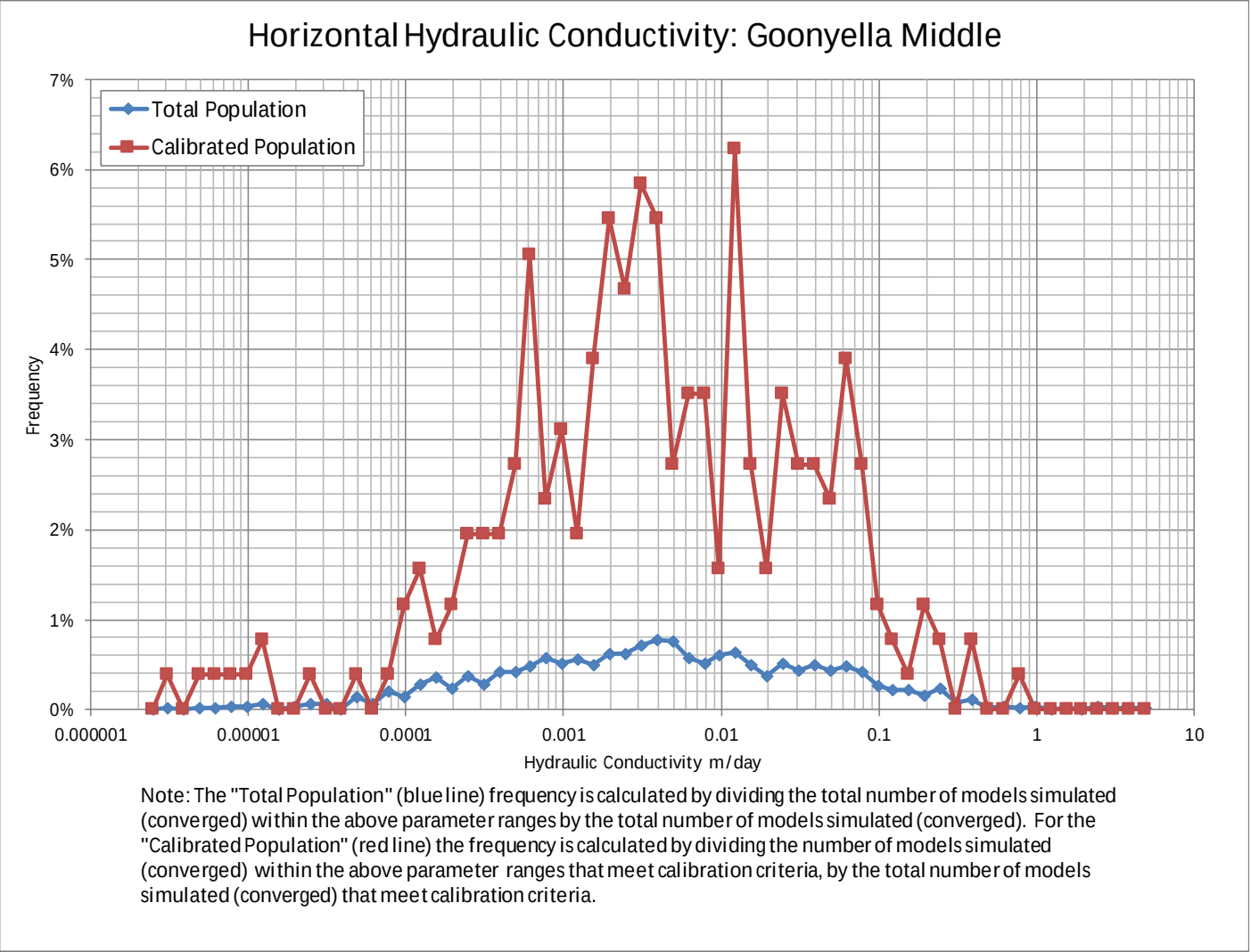




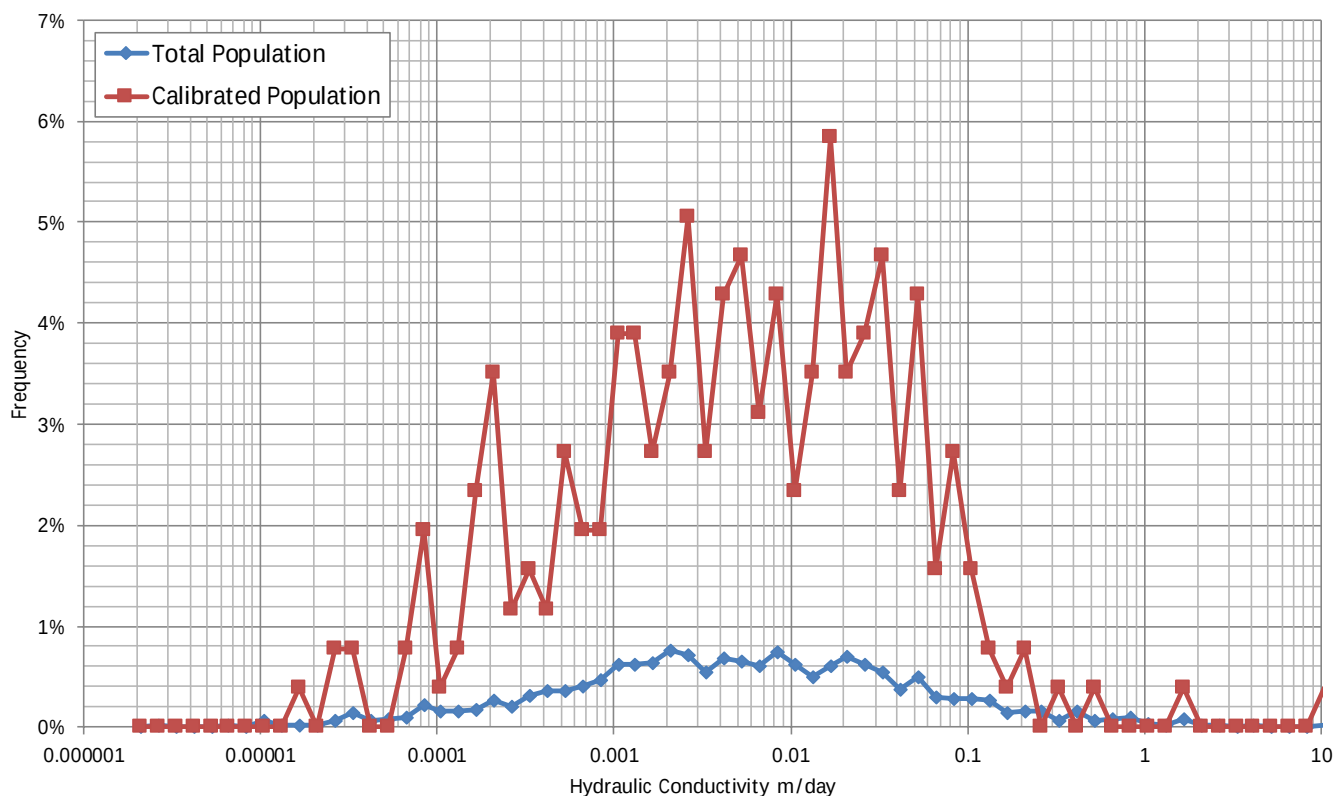






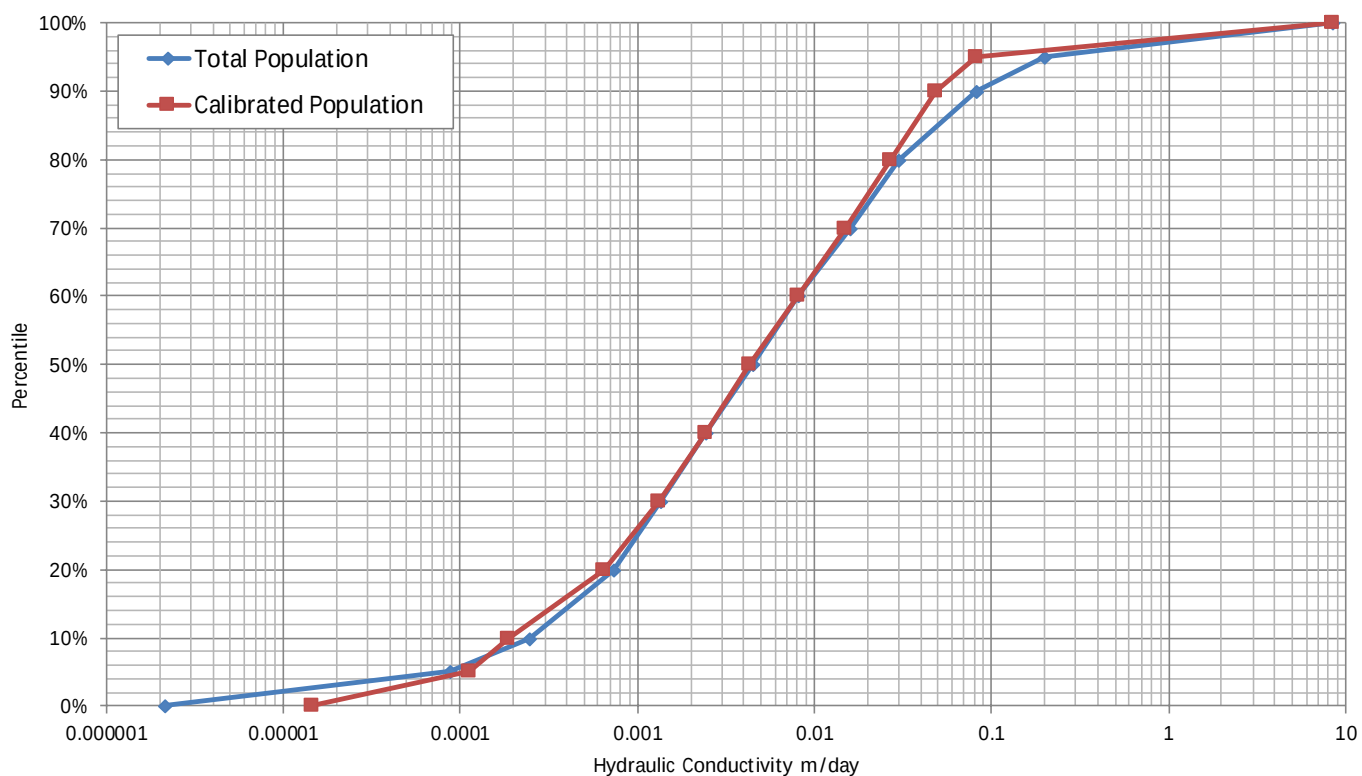


Horizontal Hydraulic Conductivity: Fault - Goonyella Middle

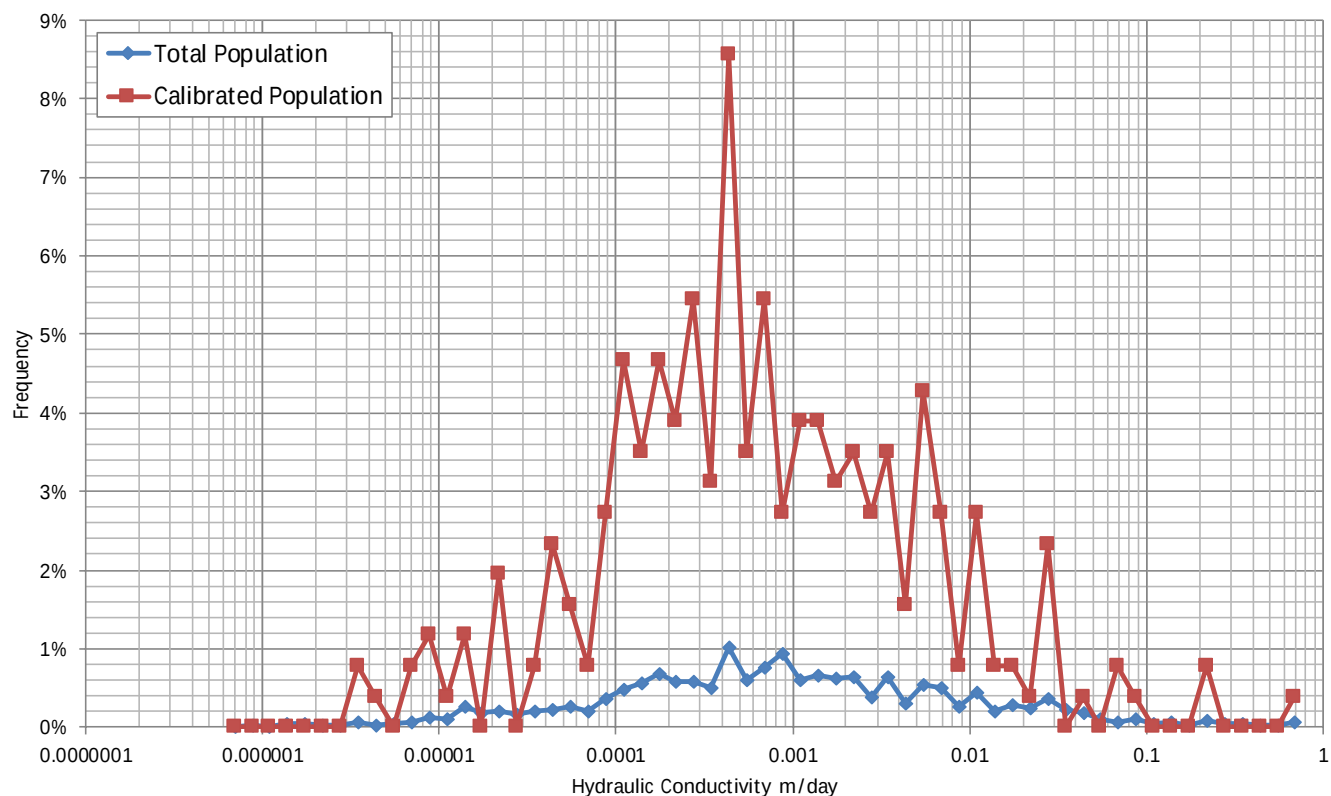


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Fault - Goonyella Middle

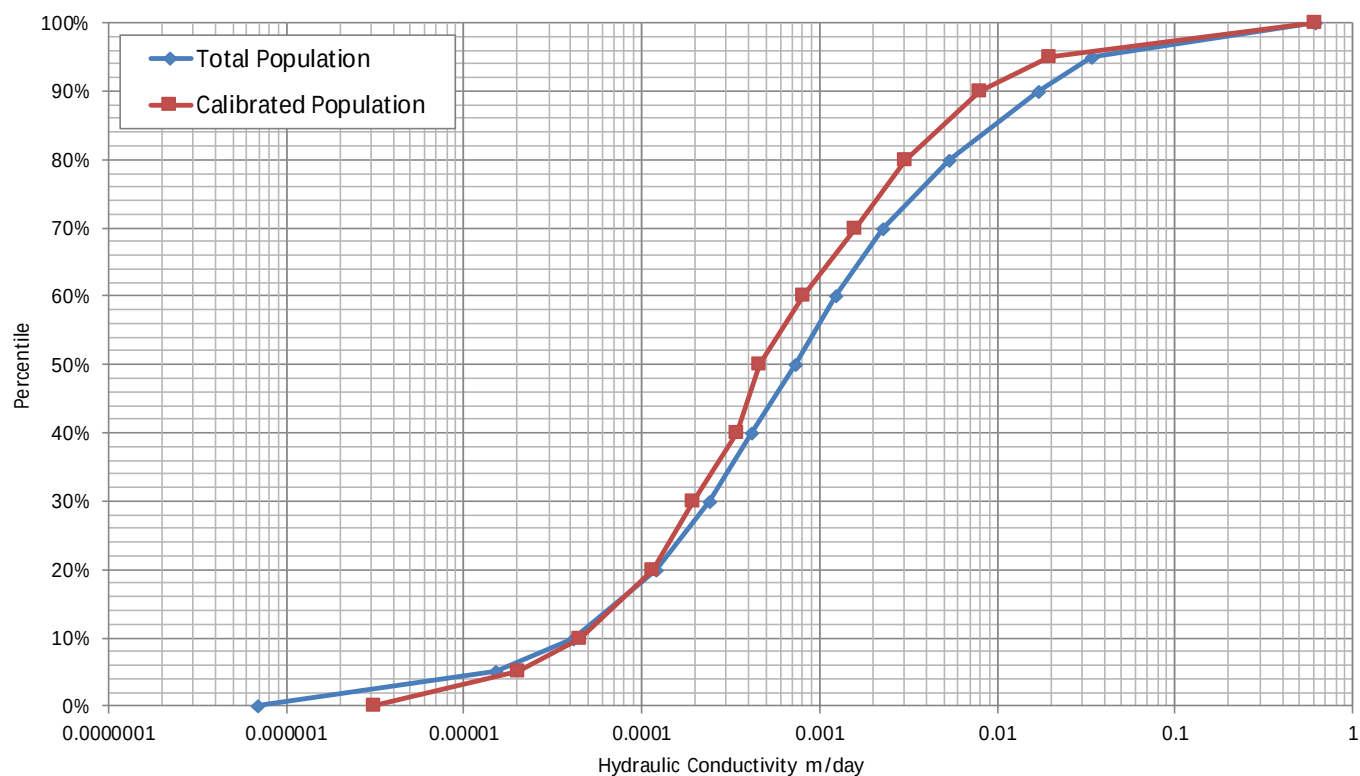


Horizontal Hydraulic Conductivity: Moranbah Underburden

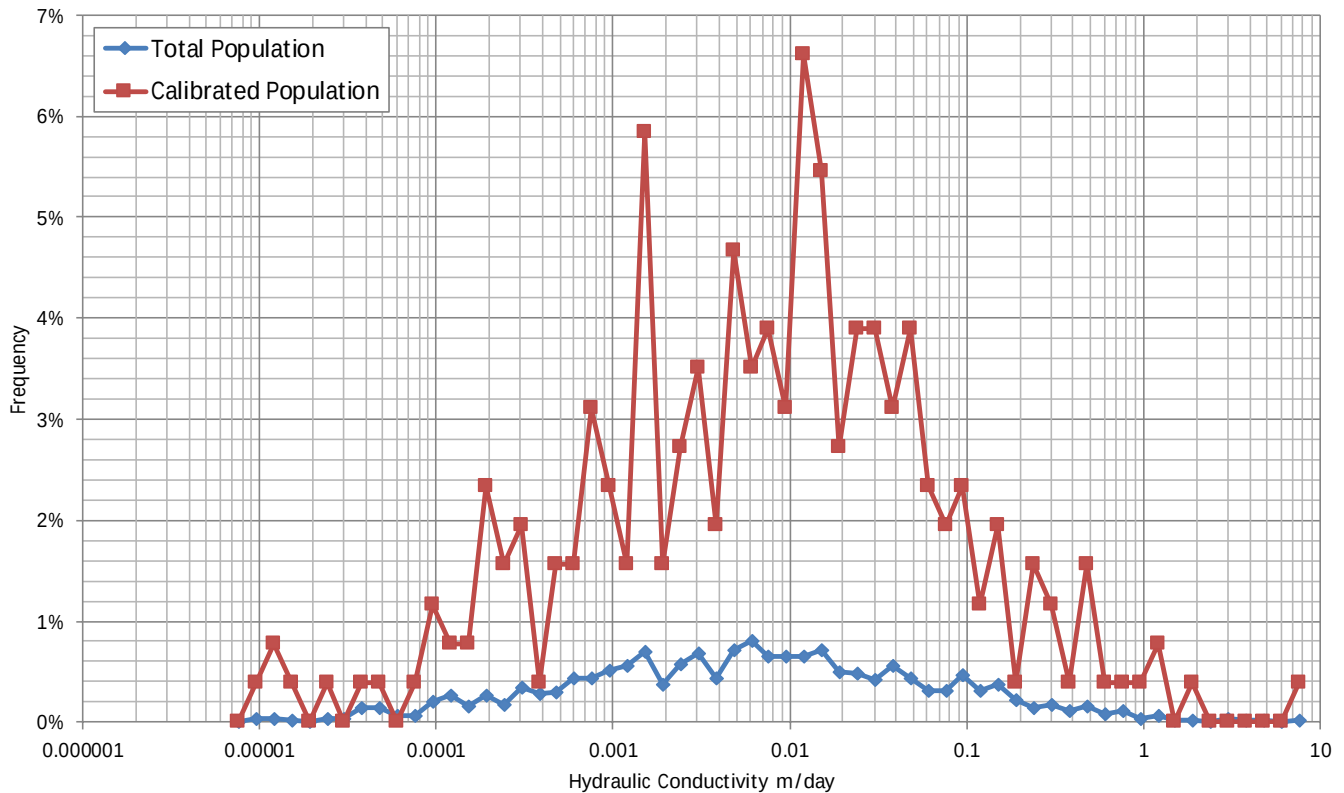


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Moranbah Underburden

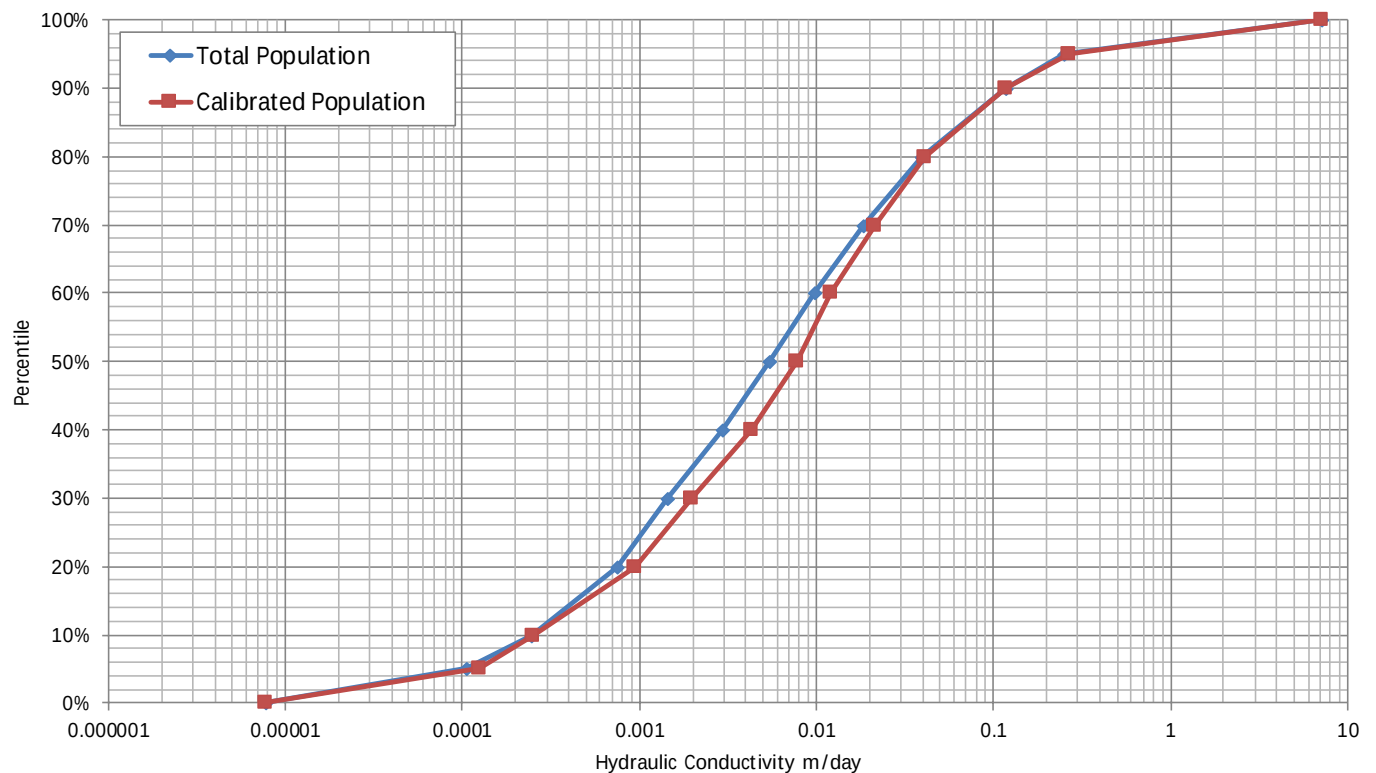


Horizontal Hydraulic Conductivity: Fault - Moranbah Underburden

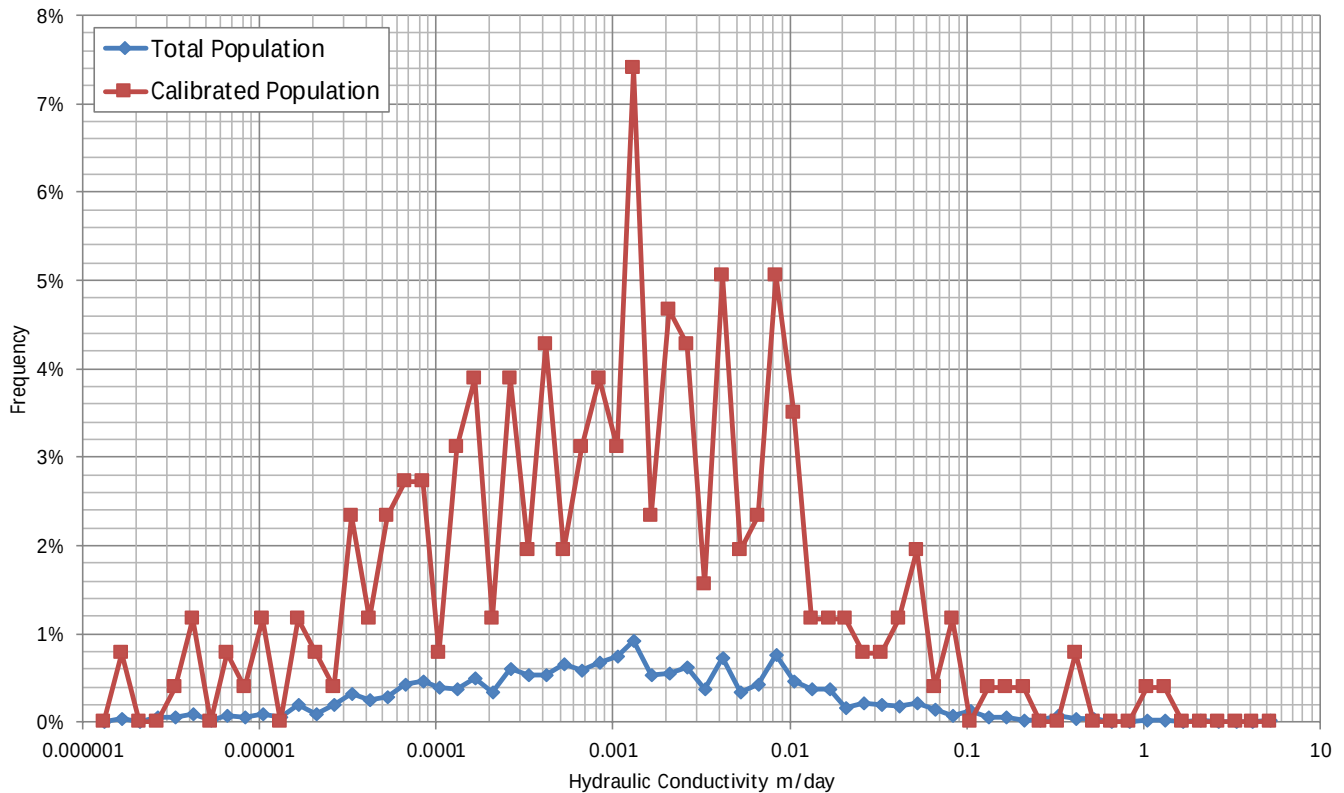


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Fault - Moranbah Underburden

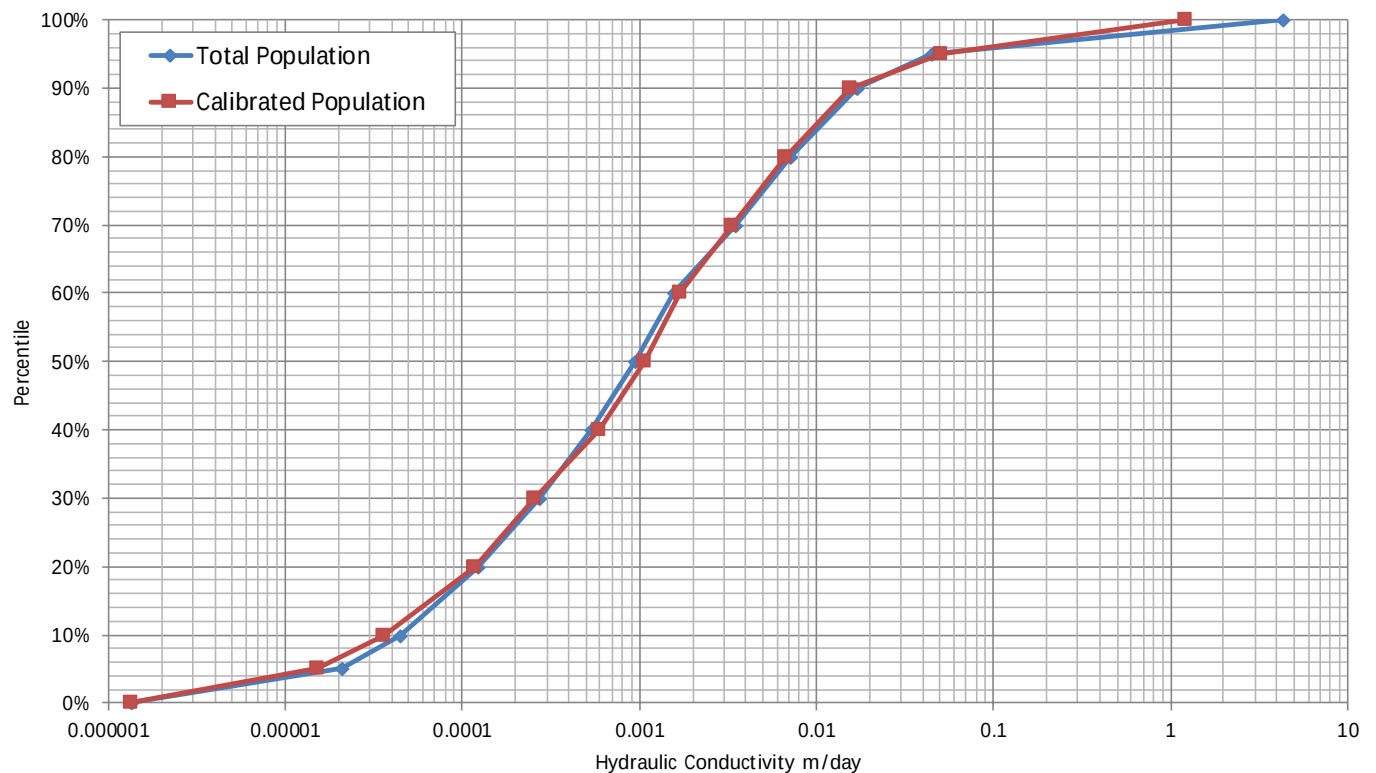


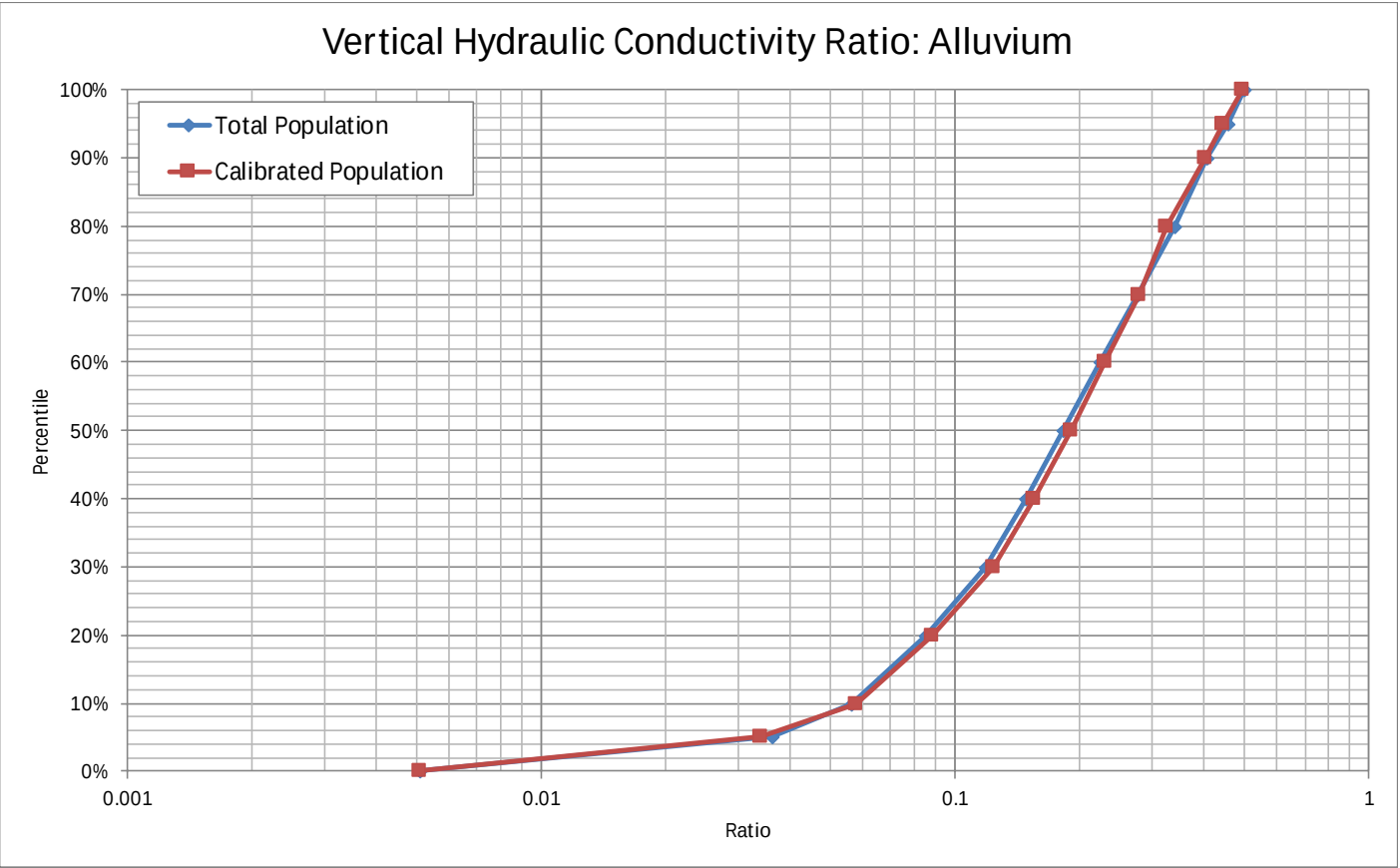
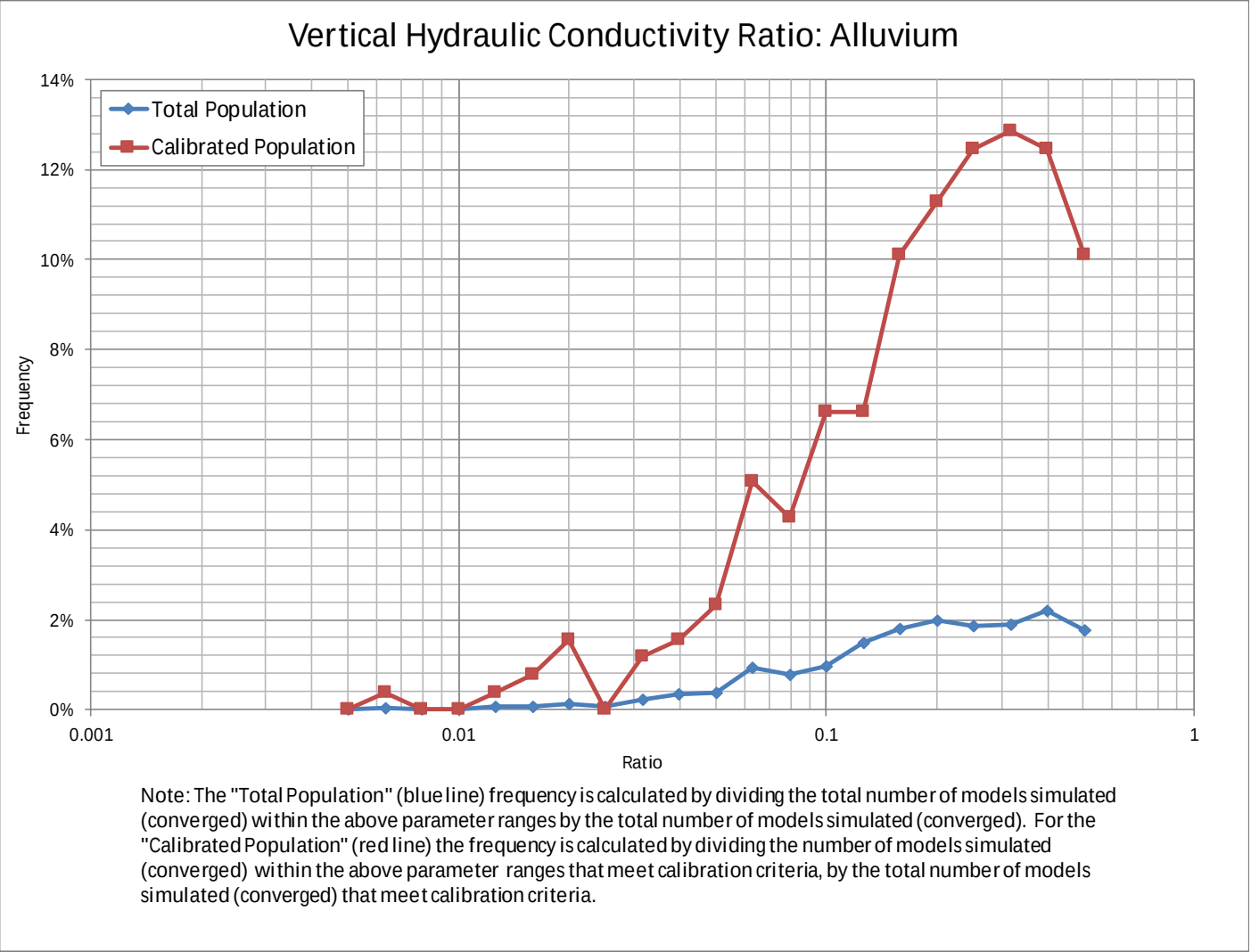
Horizontal Hydraulic Conductivity: Undivided Intrusives

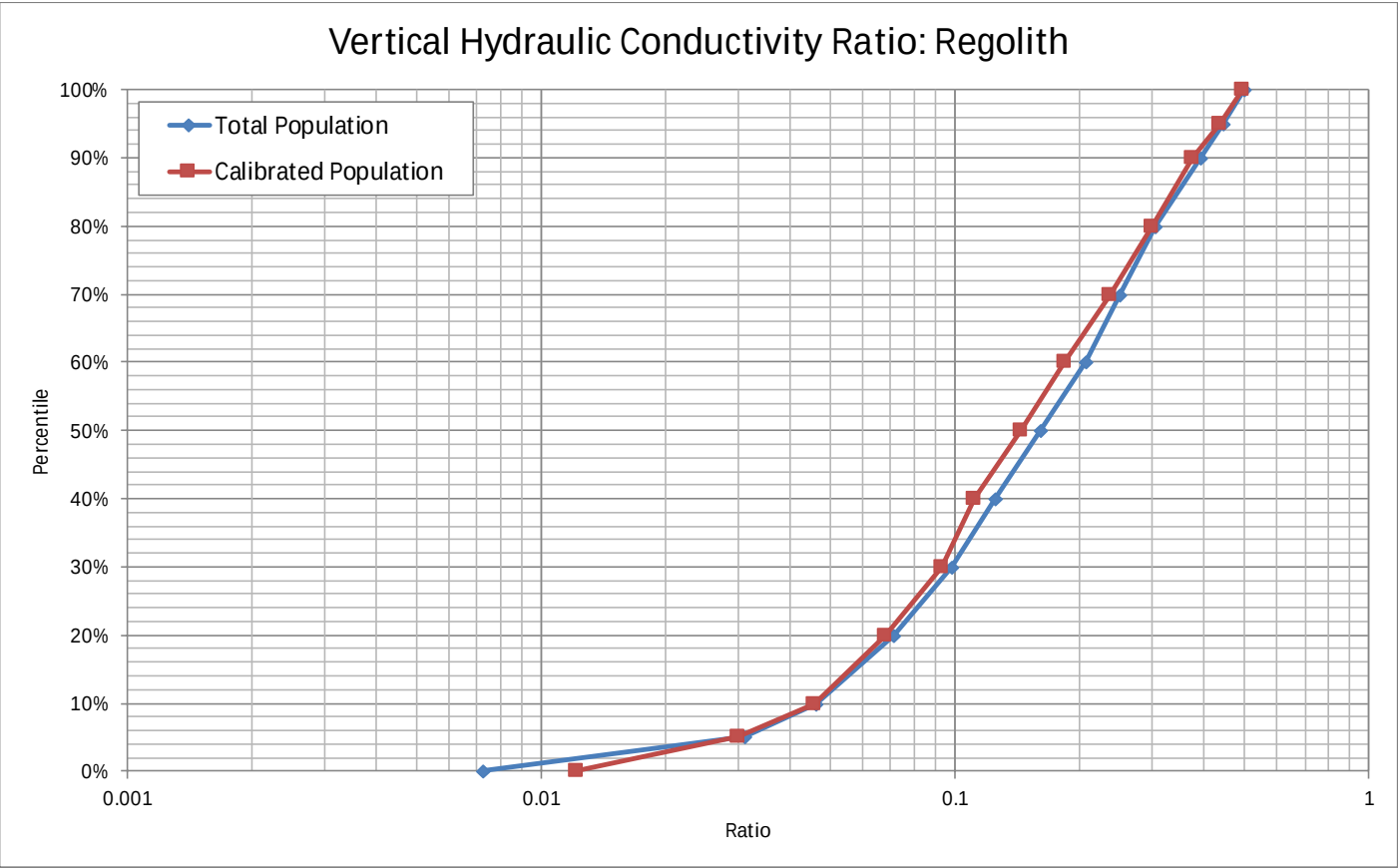
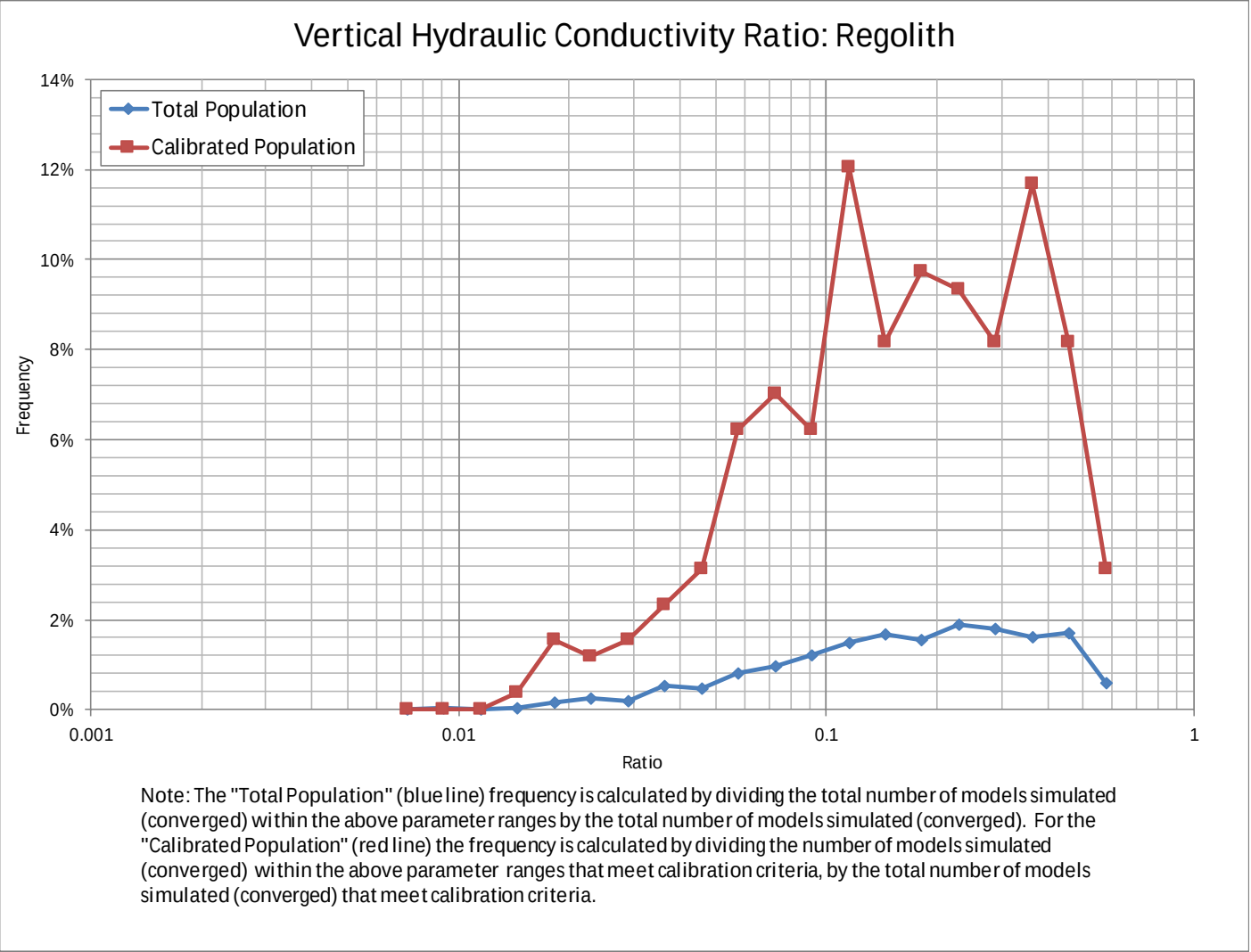


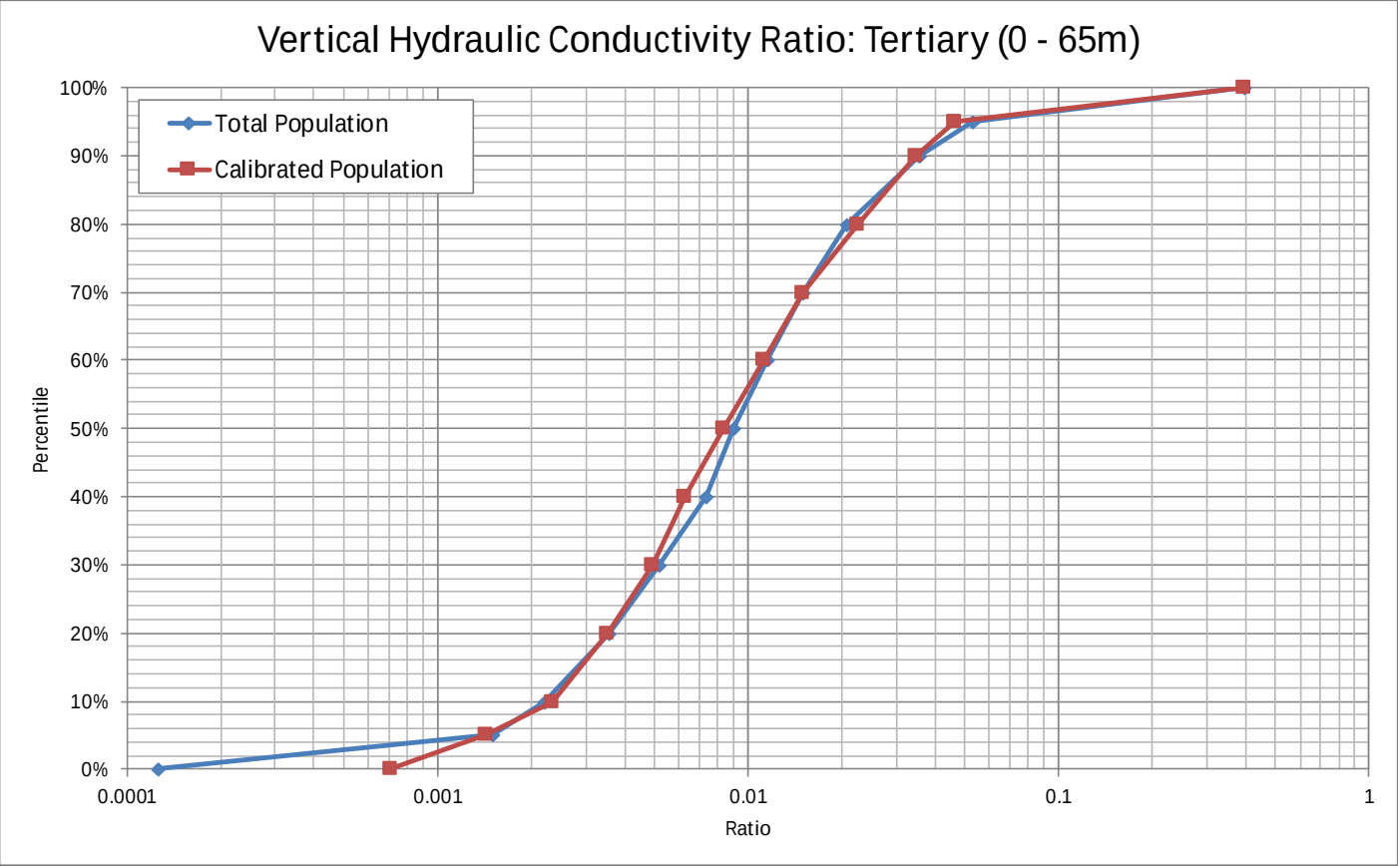
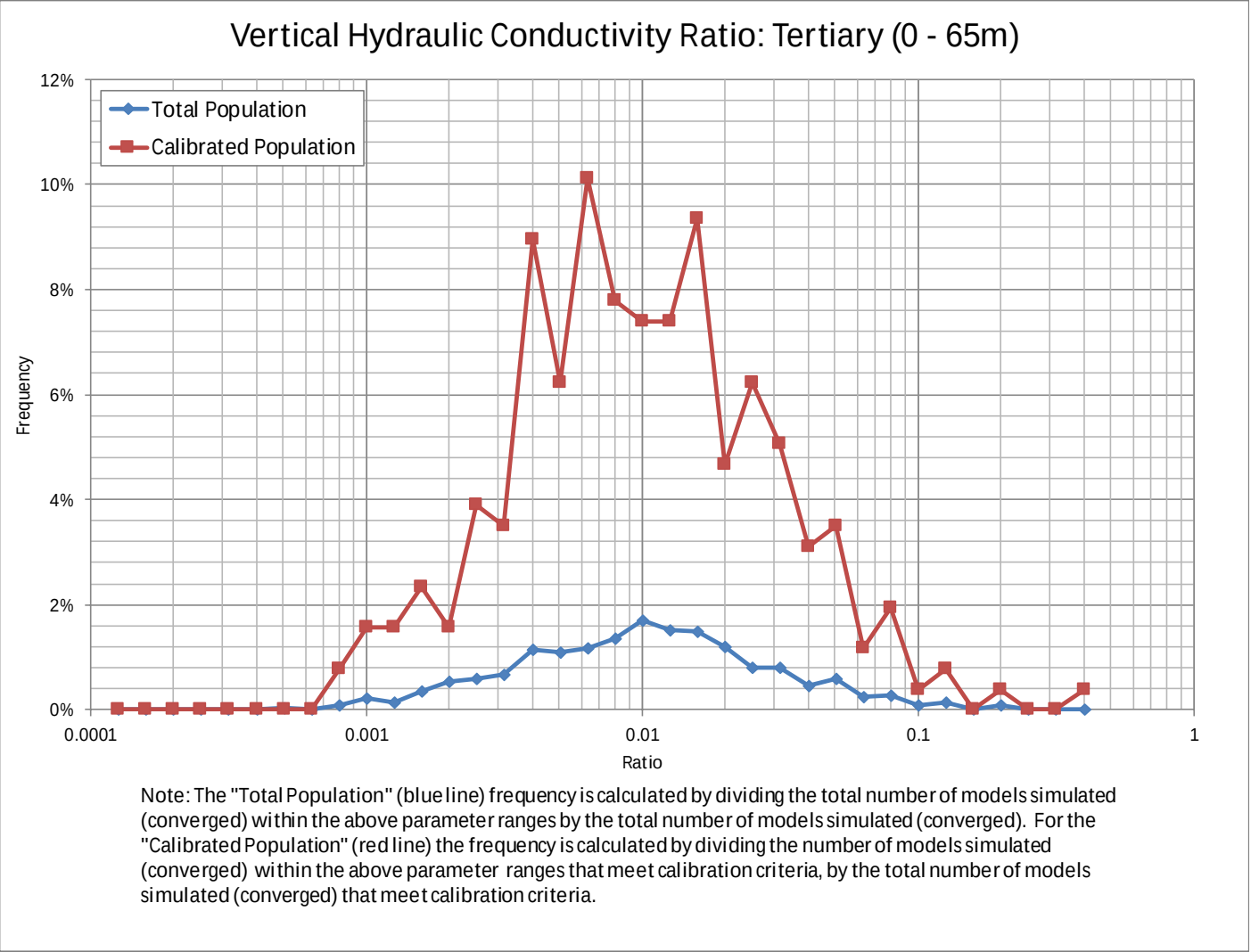
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Horizontal Hydraulic Conductivity: Undivided Intrusives

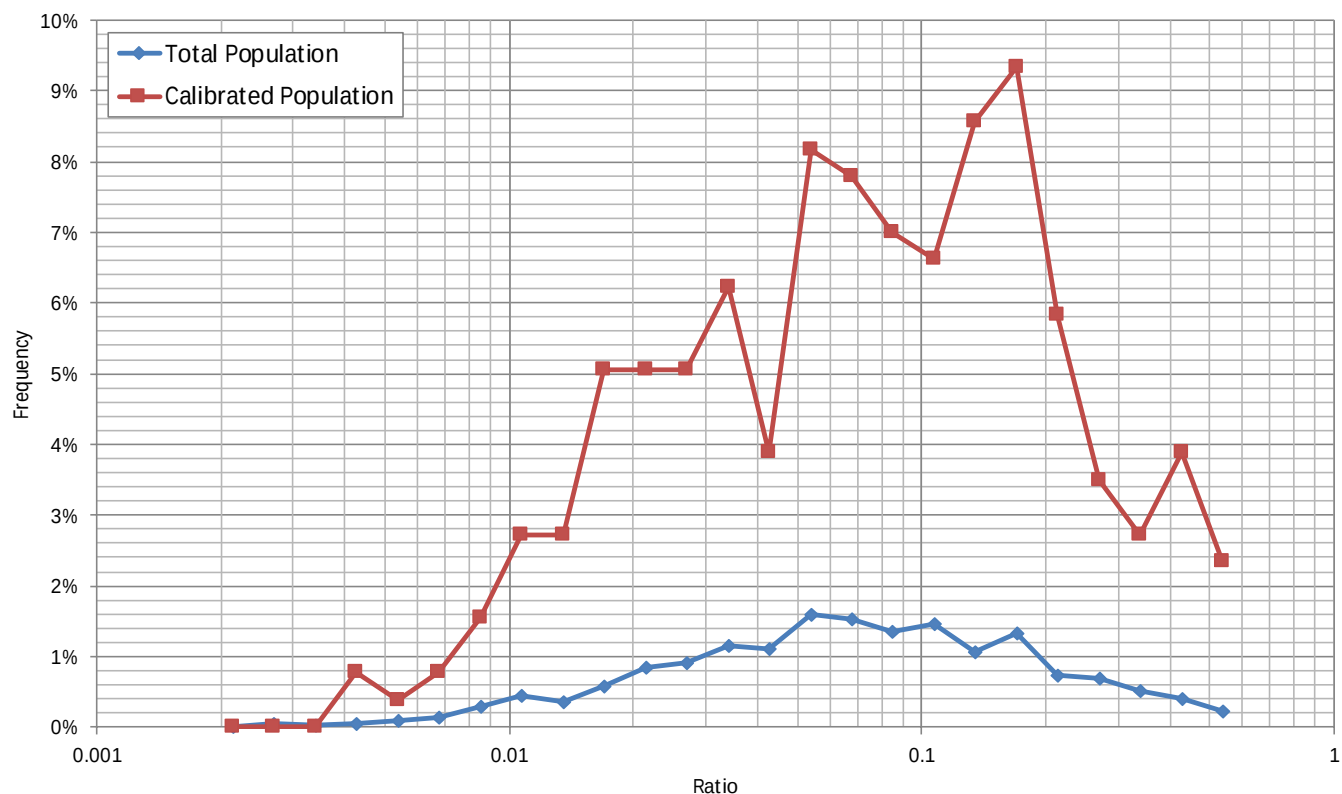






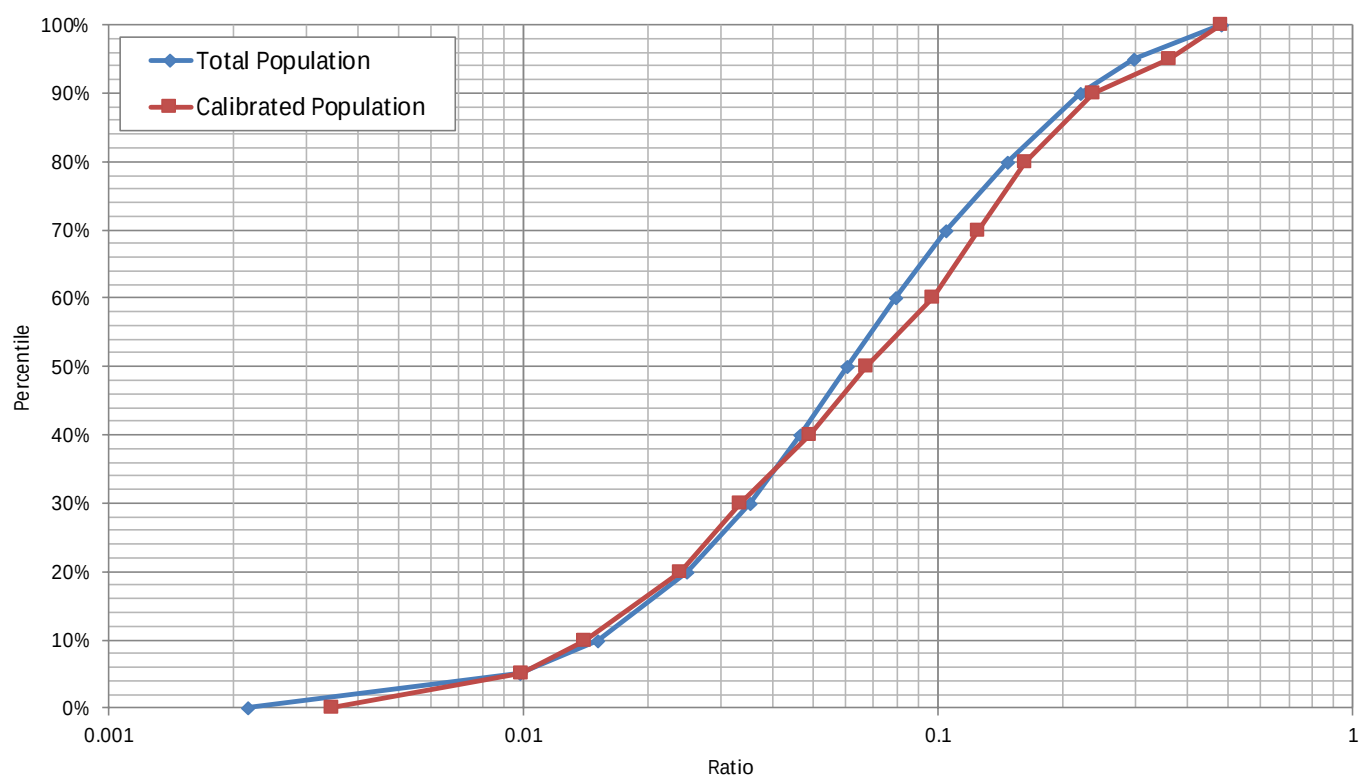


Vertical Hydraulic Conductivity Ratio: Tertiary (65+m)

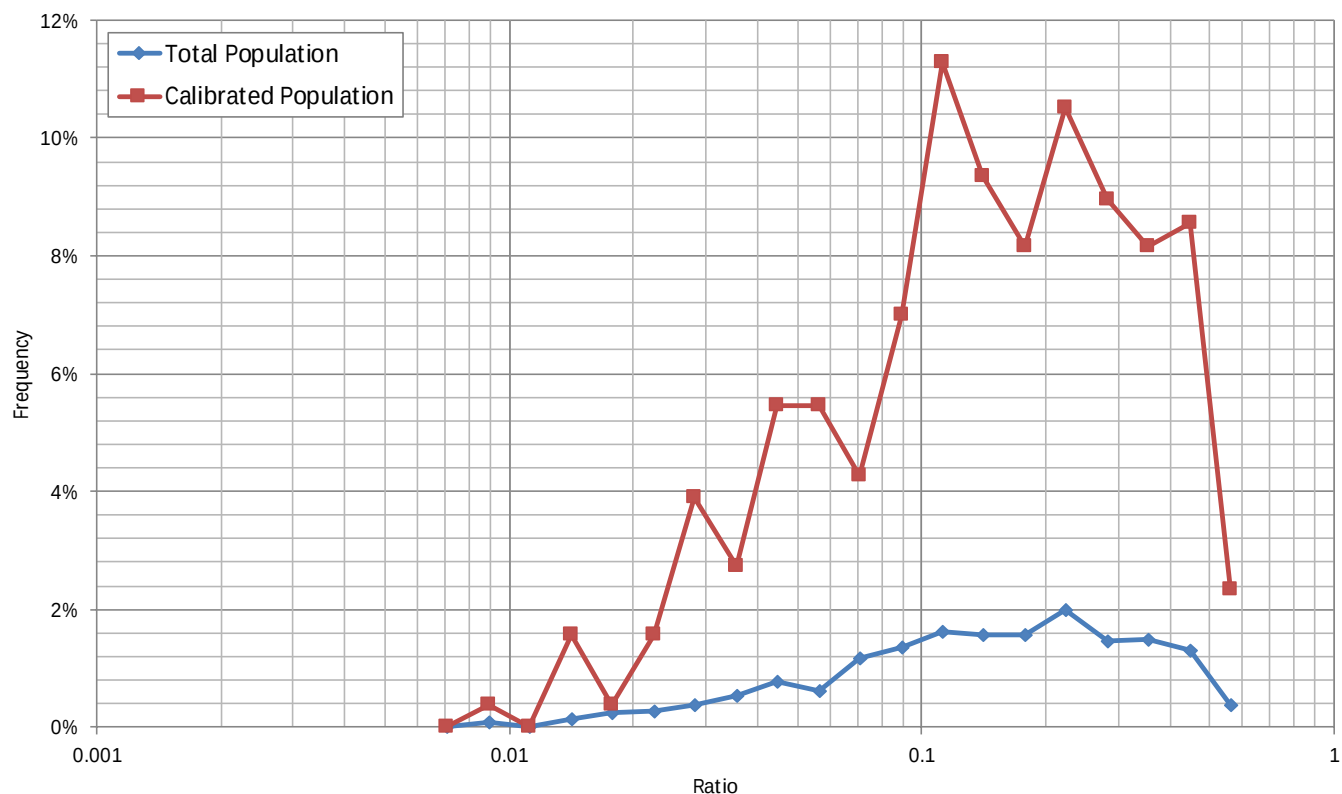


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Vertical Hydraulic Conductivity Ratio: Tertiary (65+m)

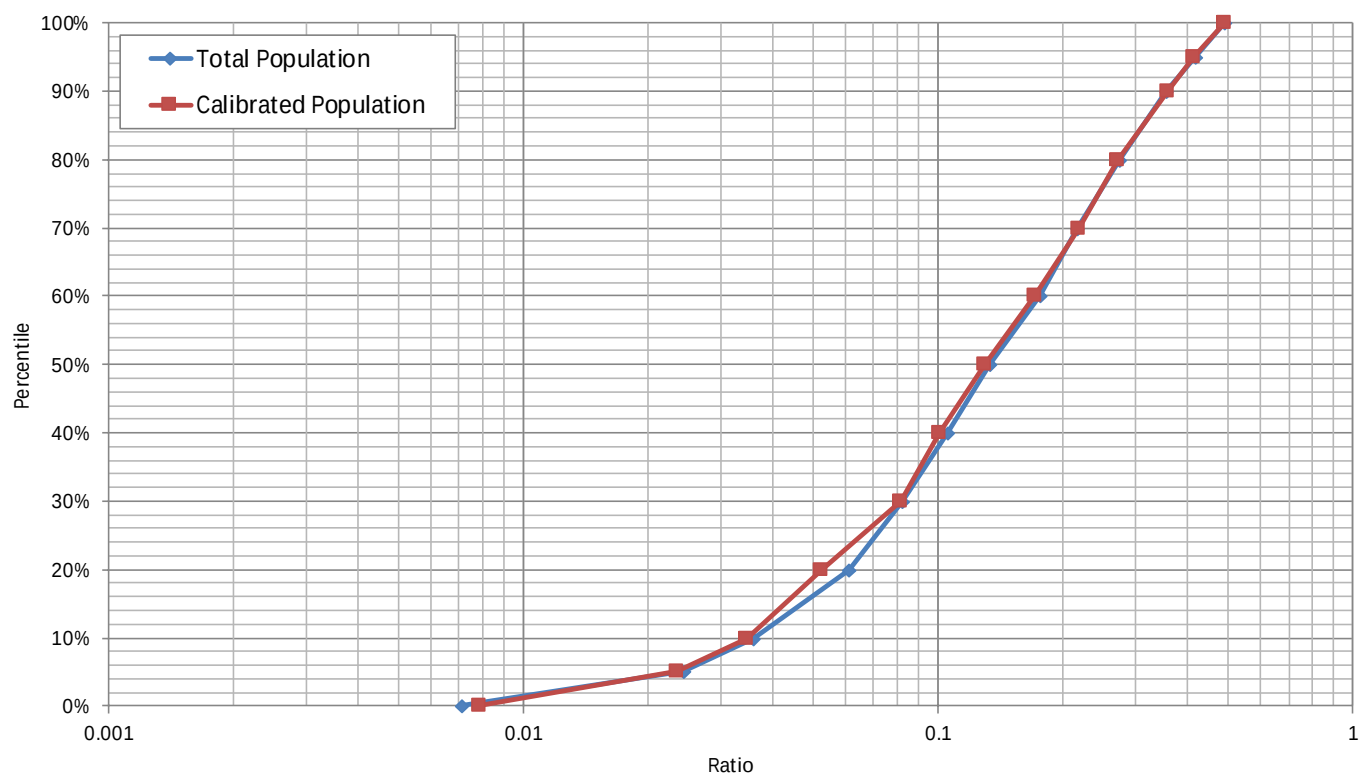


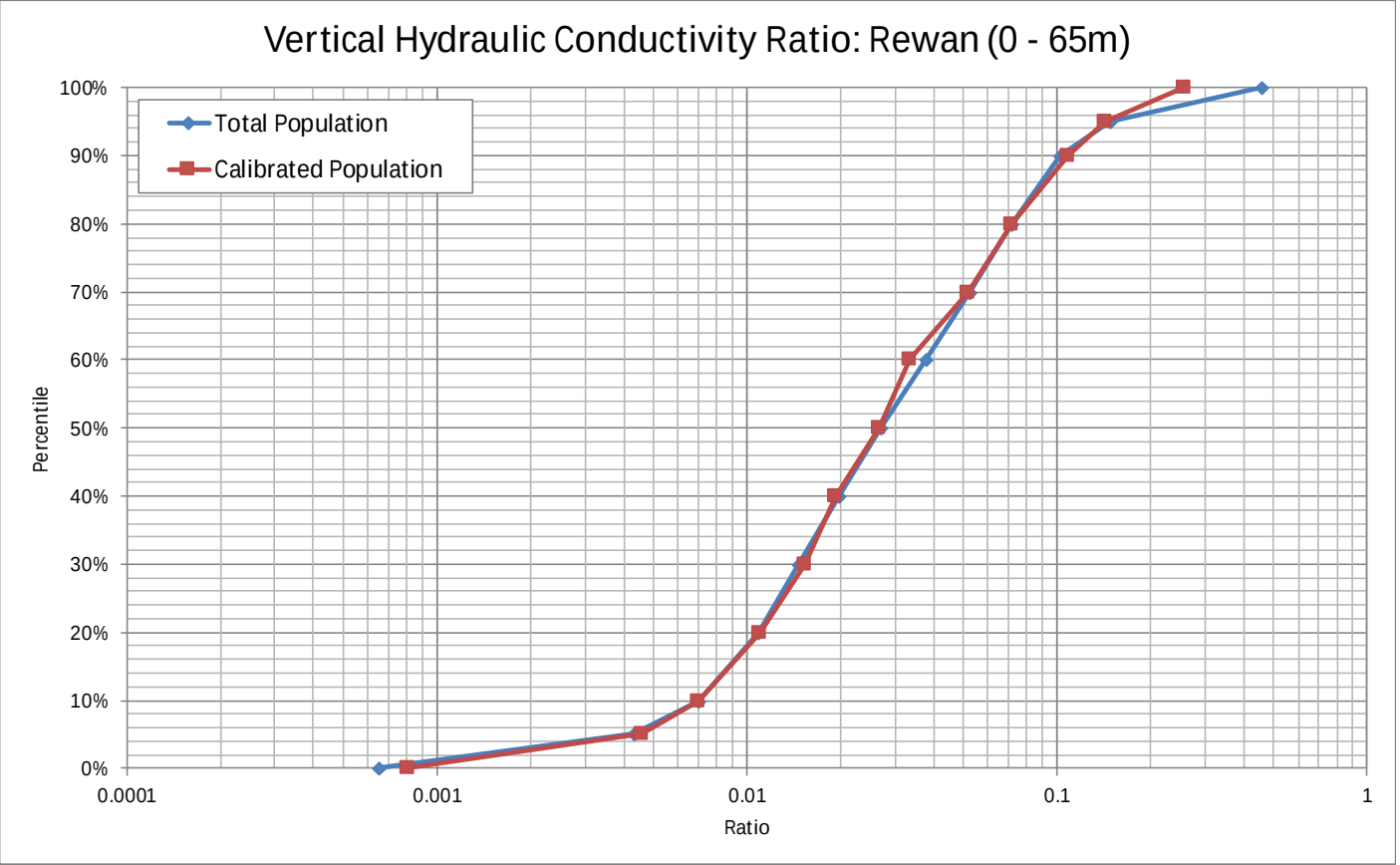
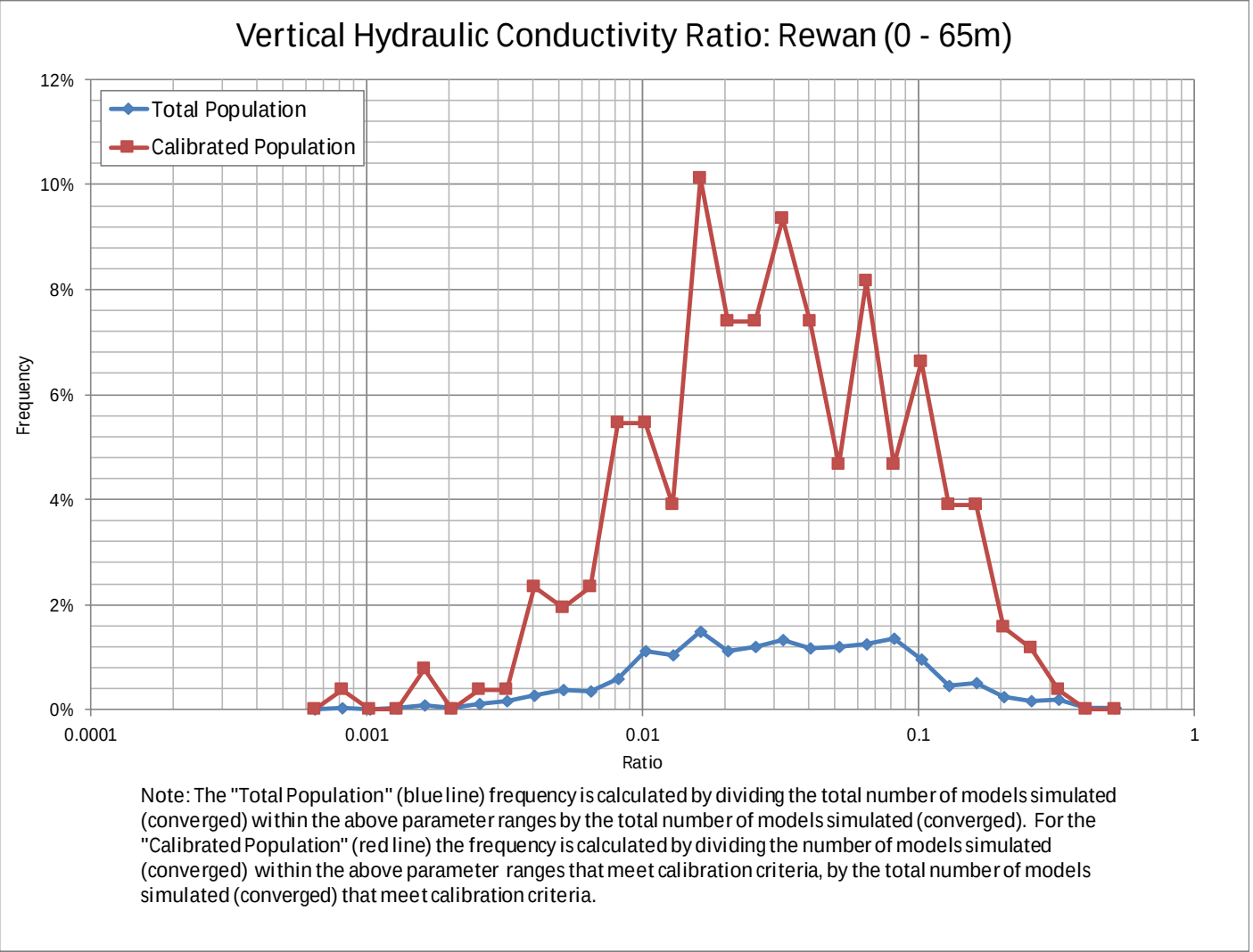
Vertical Hydraulic Conductivity Ratio: Tertiary basalt

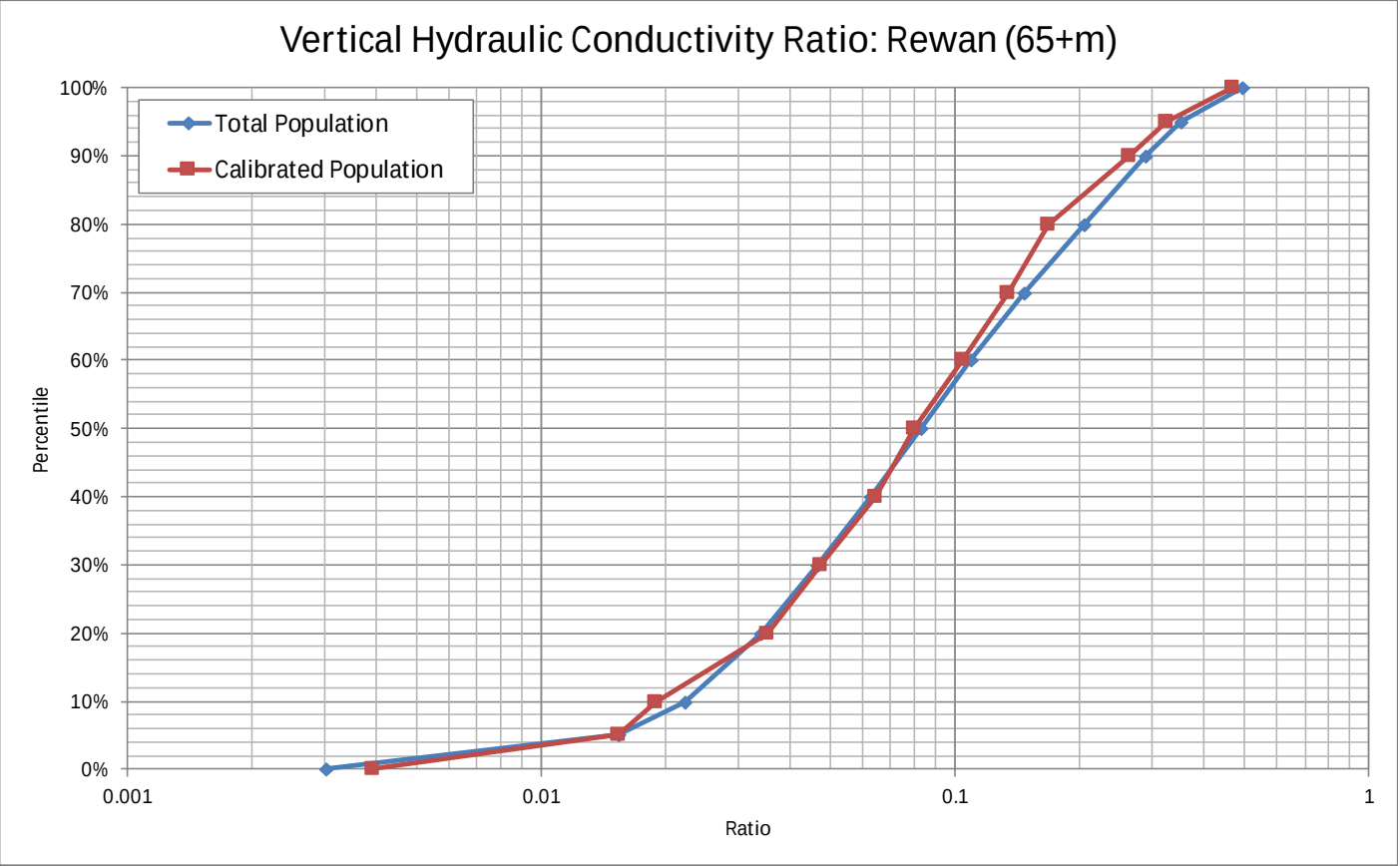
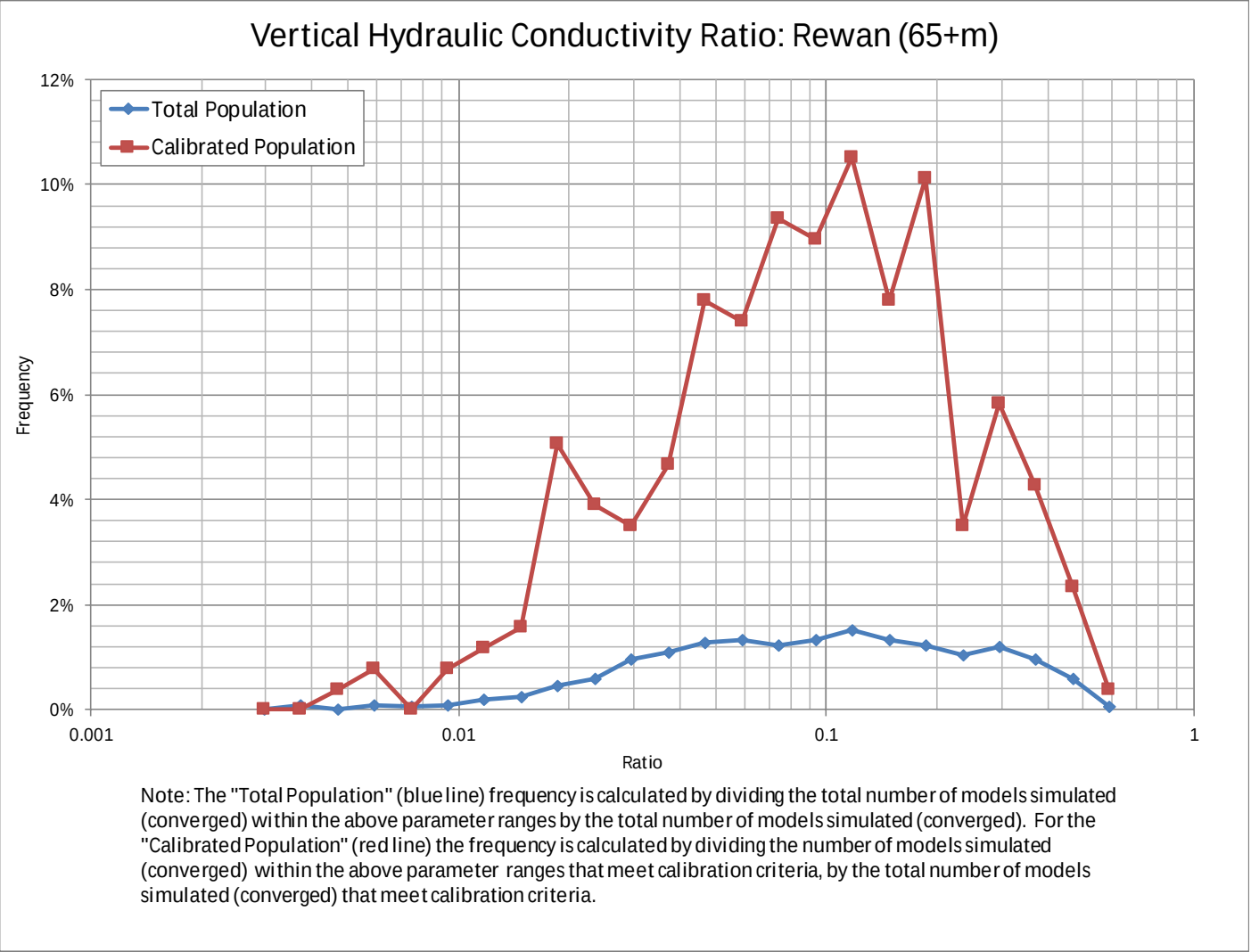


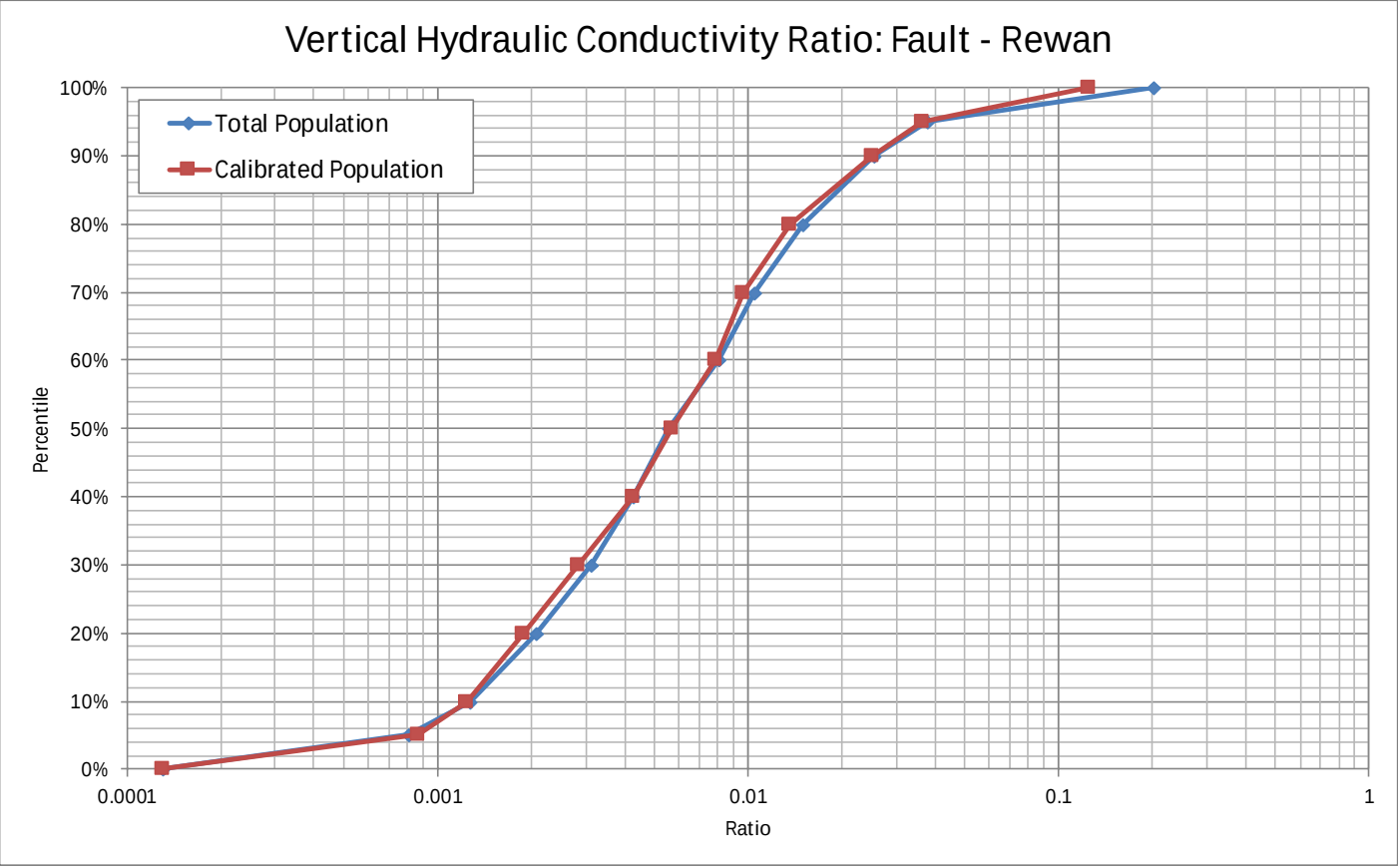
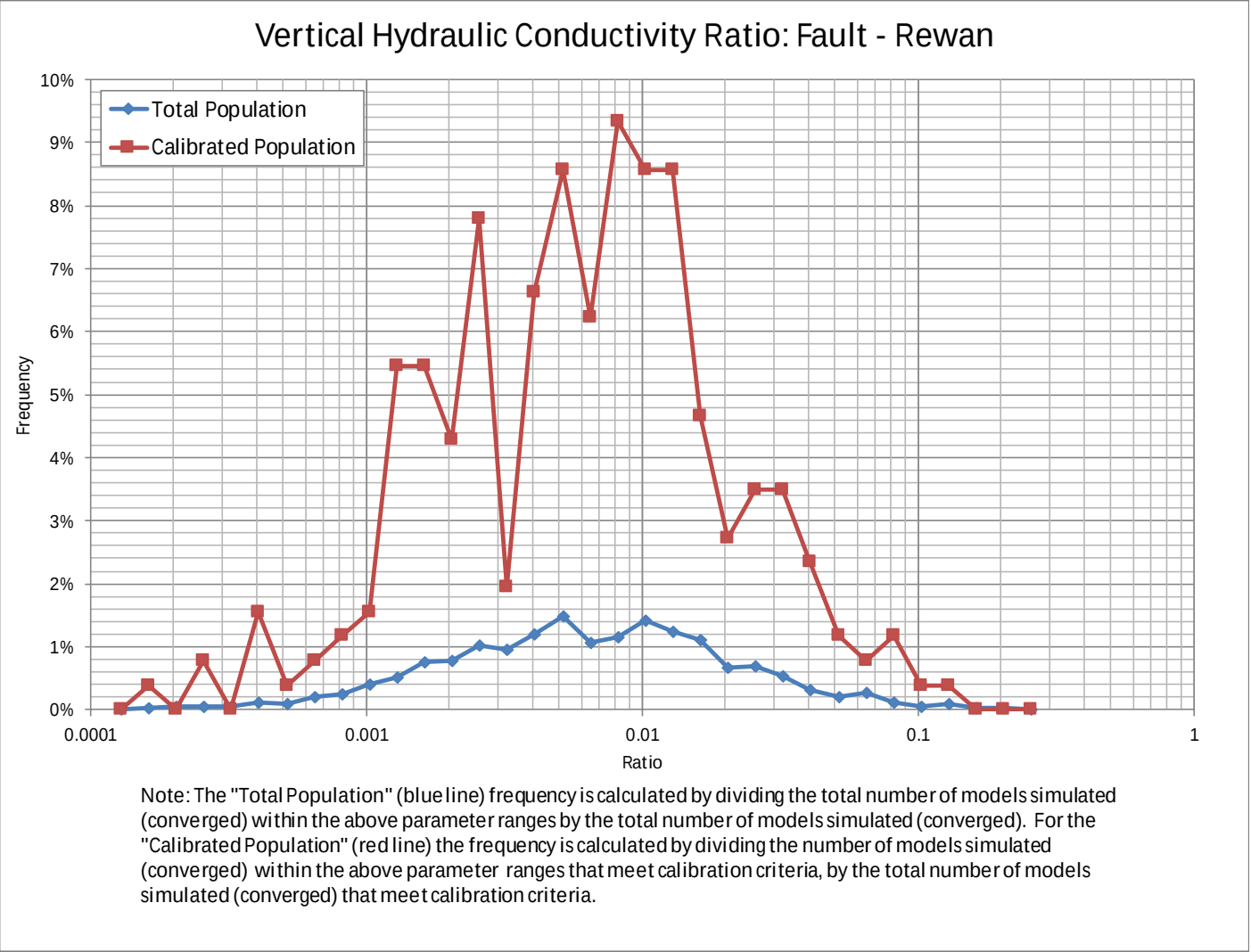
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

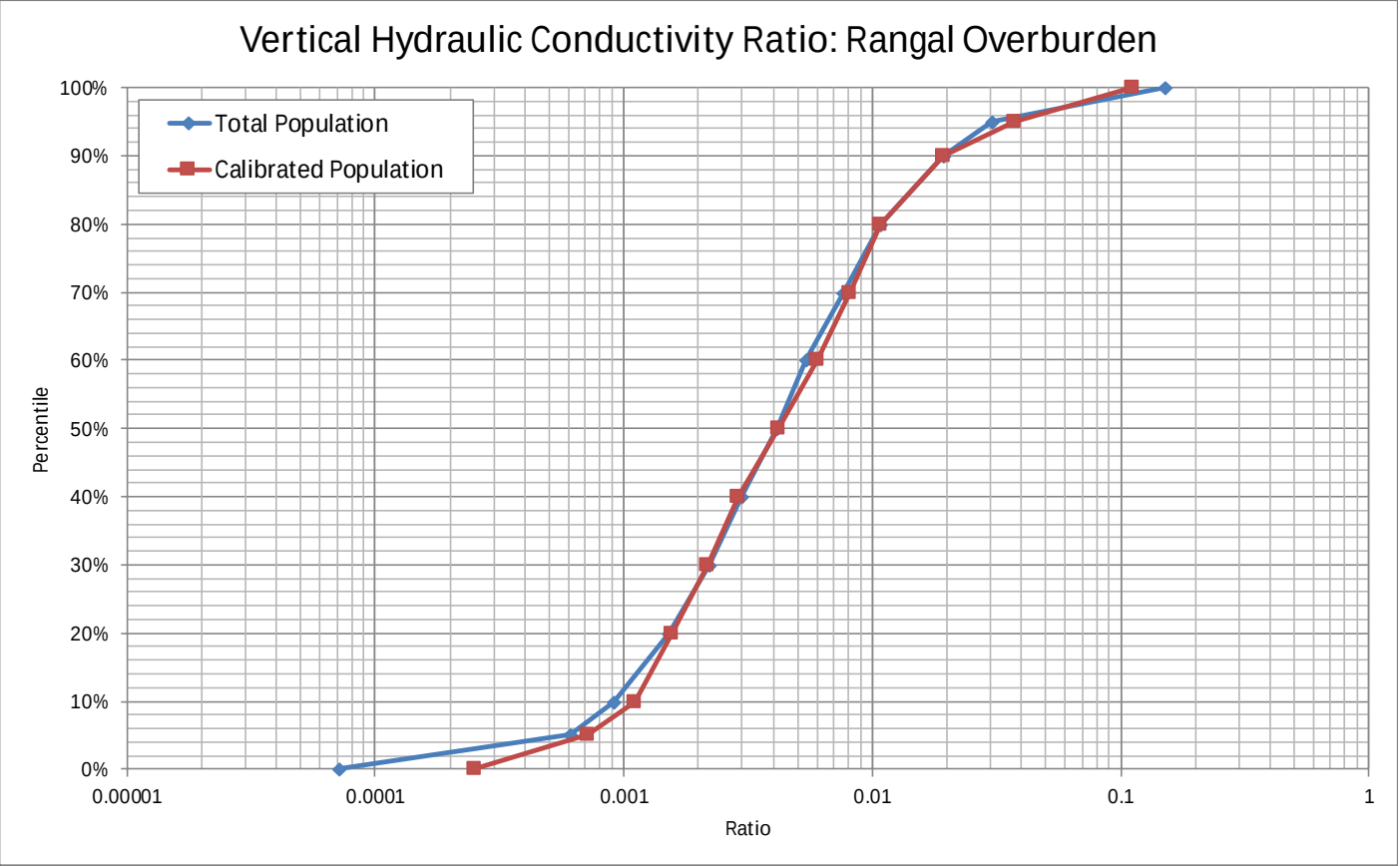
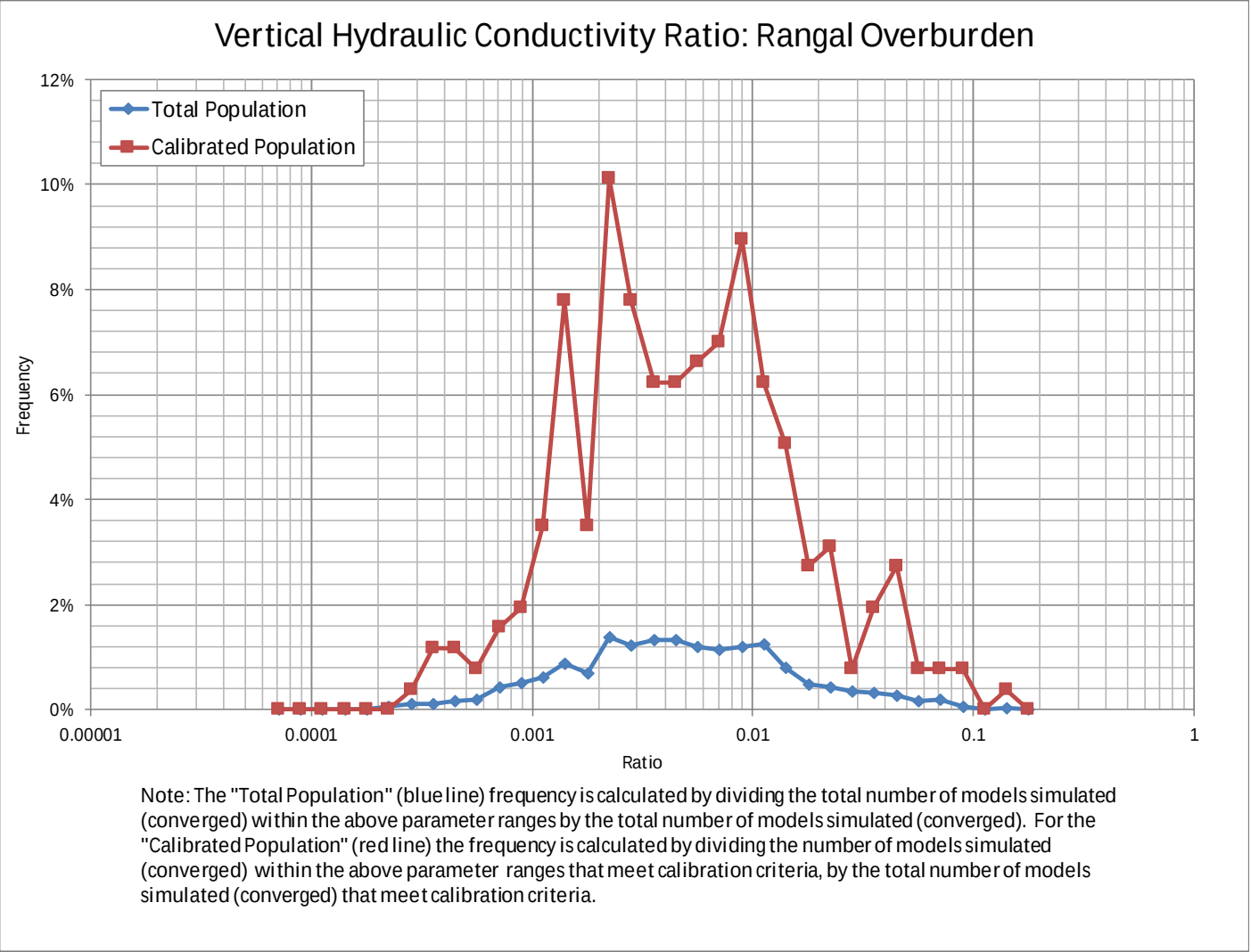
Vertical Hydraulic Conductivity Ratio: Tertiary basalt

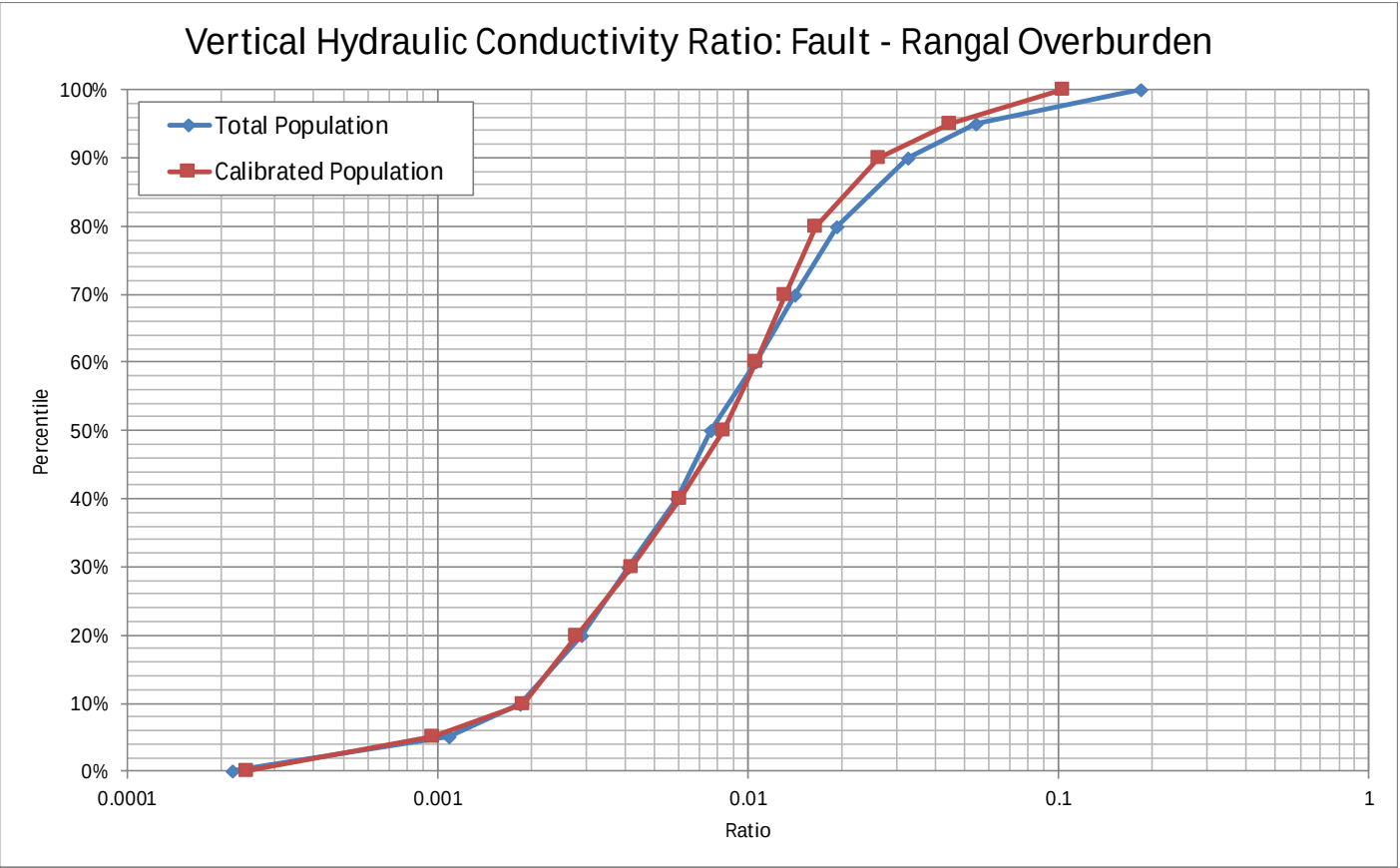
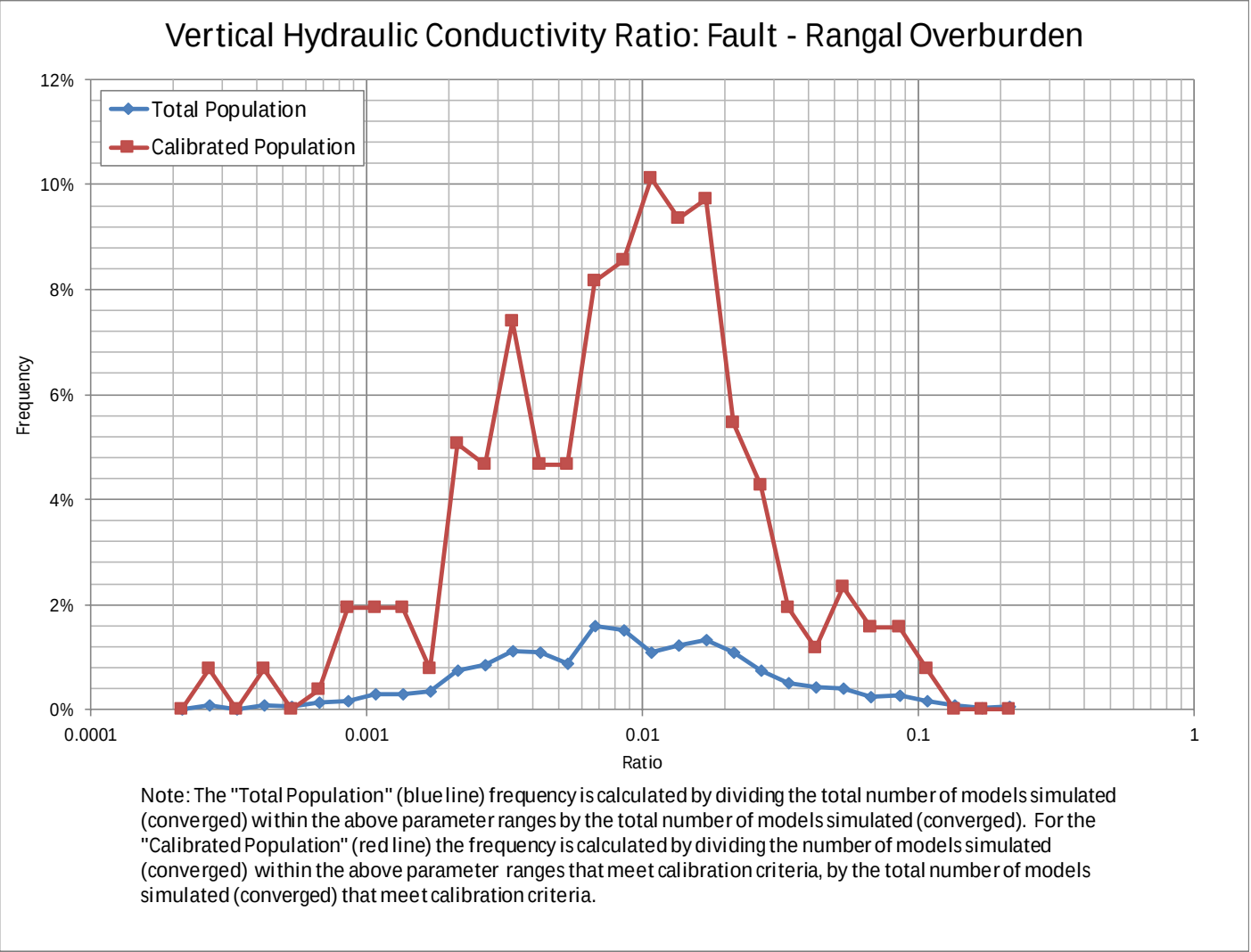




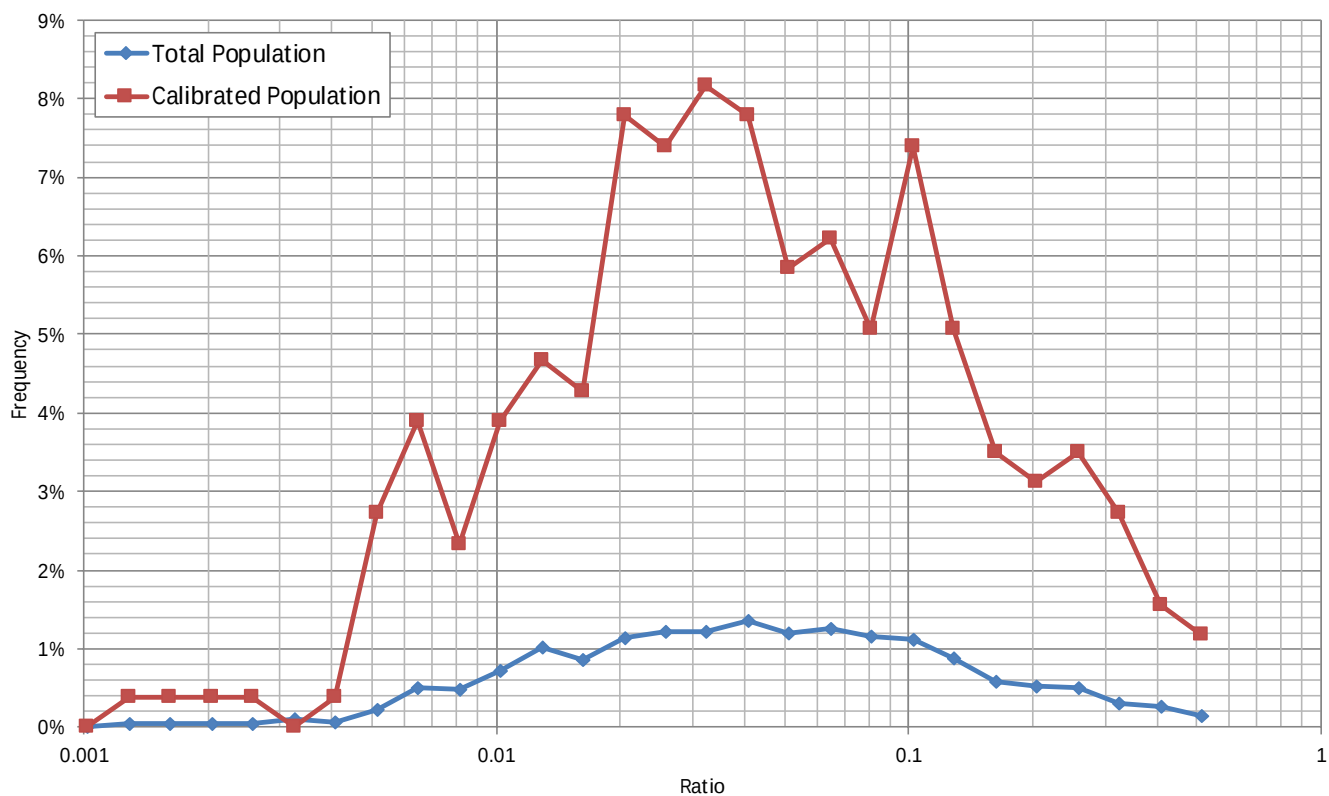






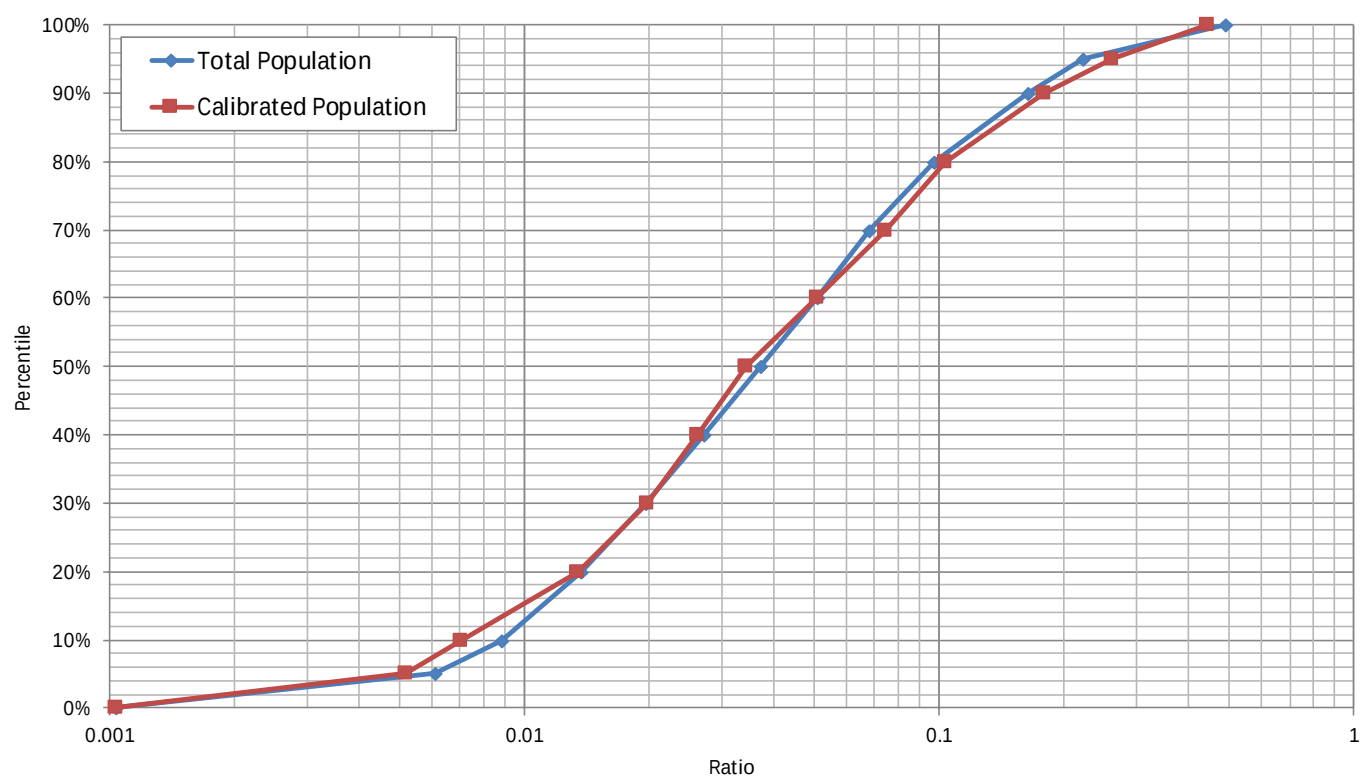


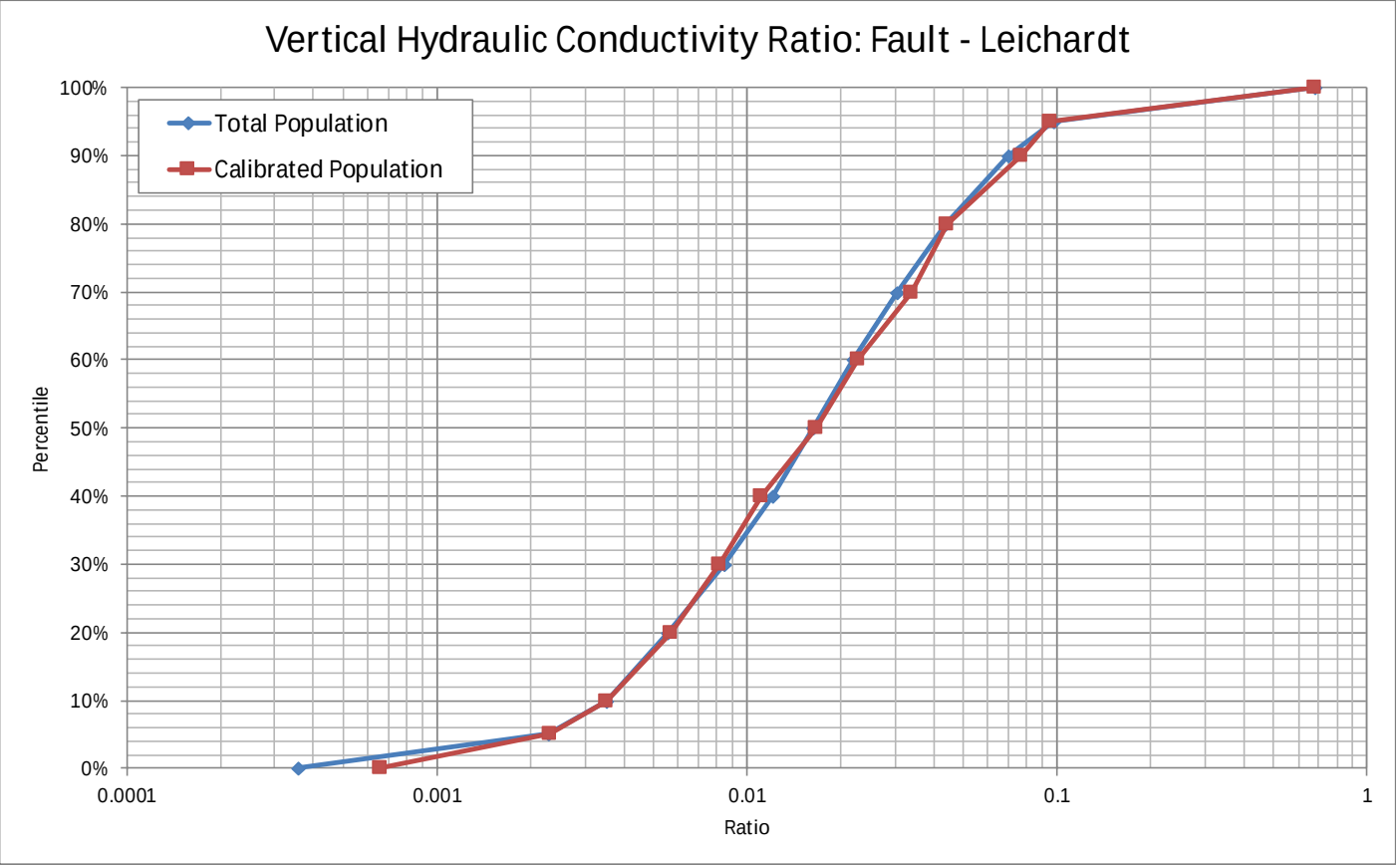
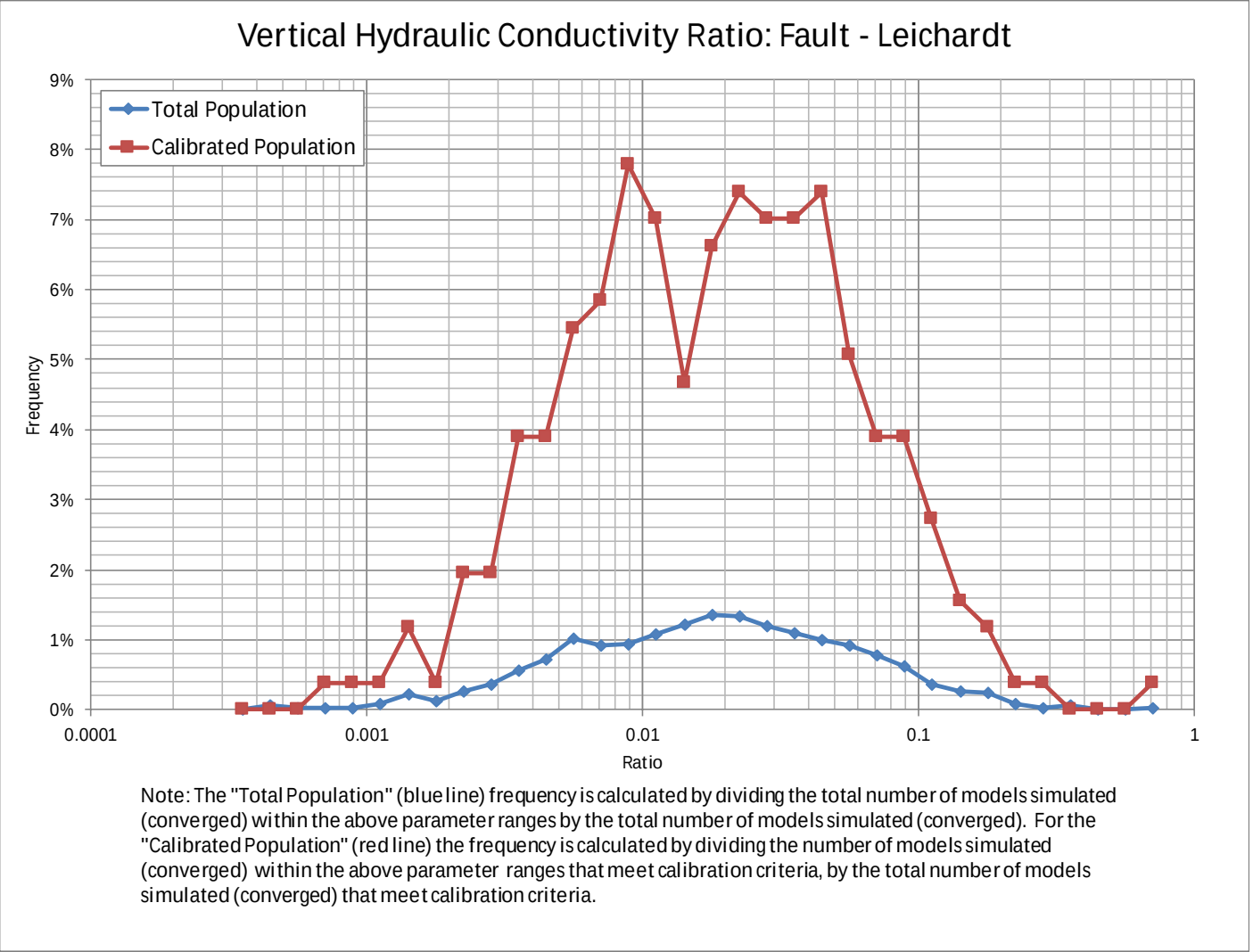
Vertical Hydraulic Conductivity Ratio: Leichardt

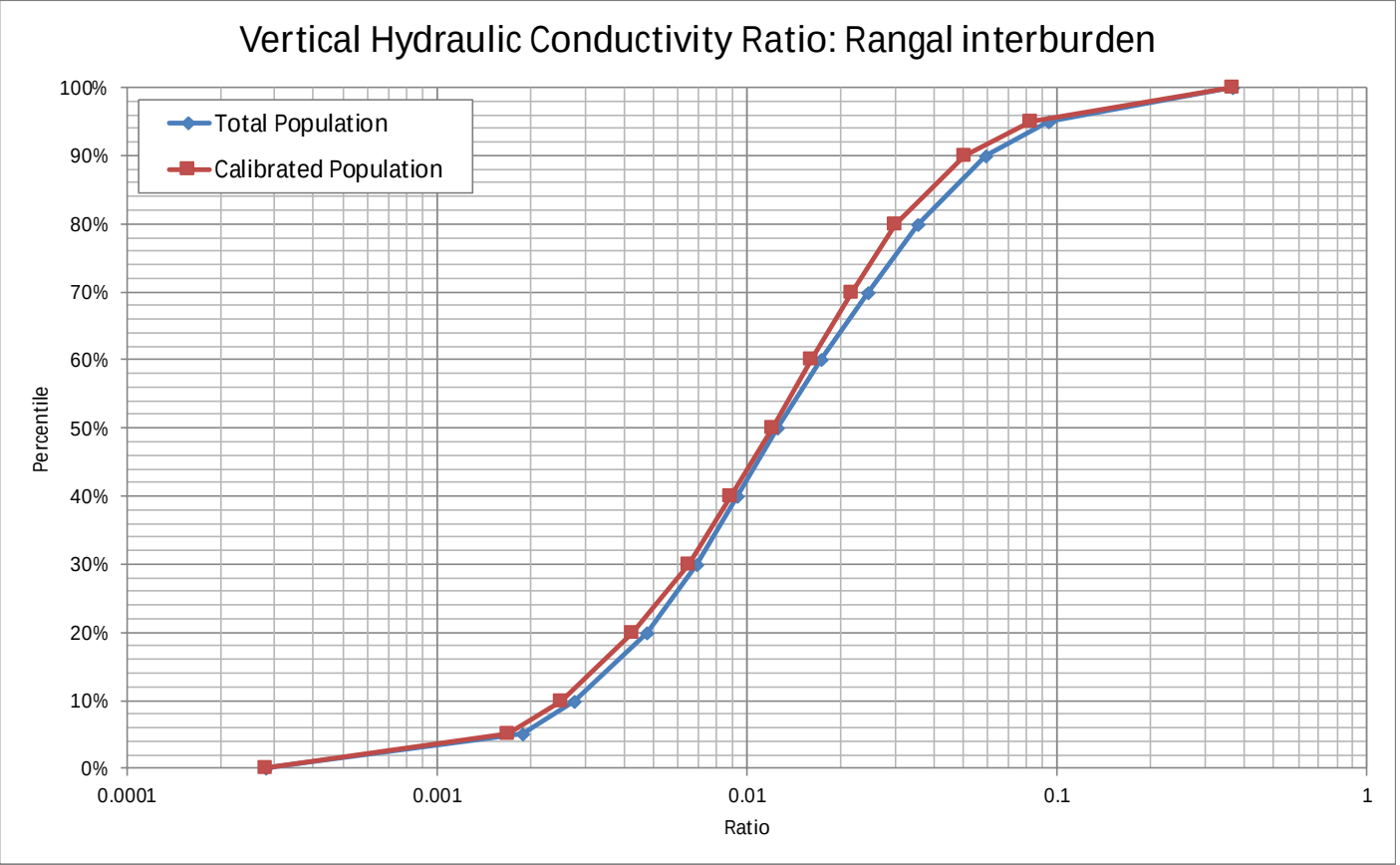
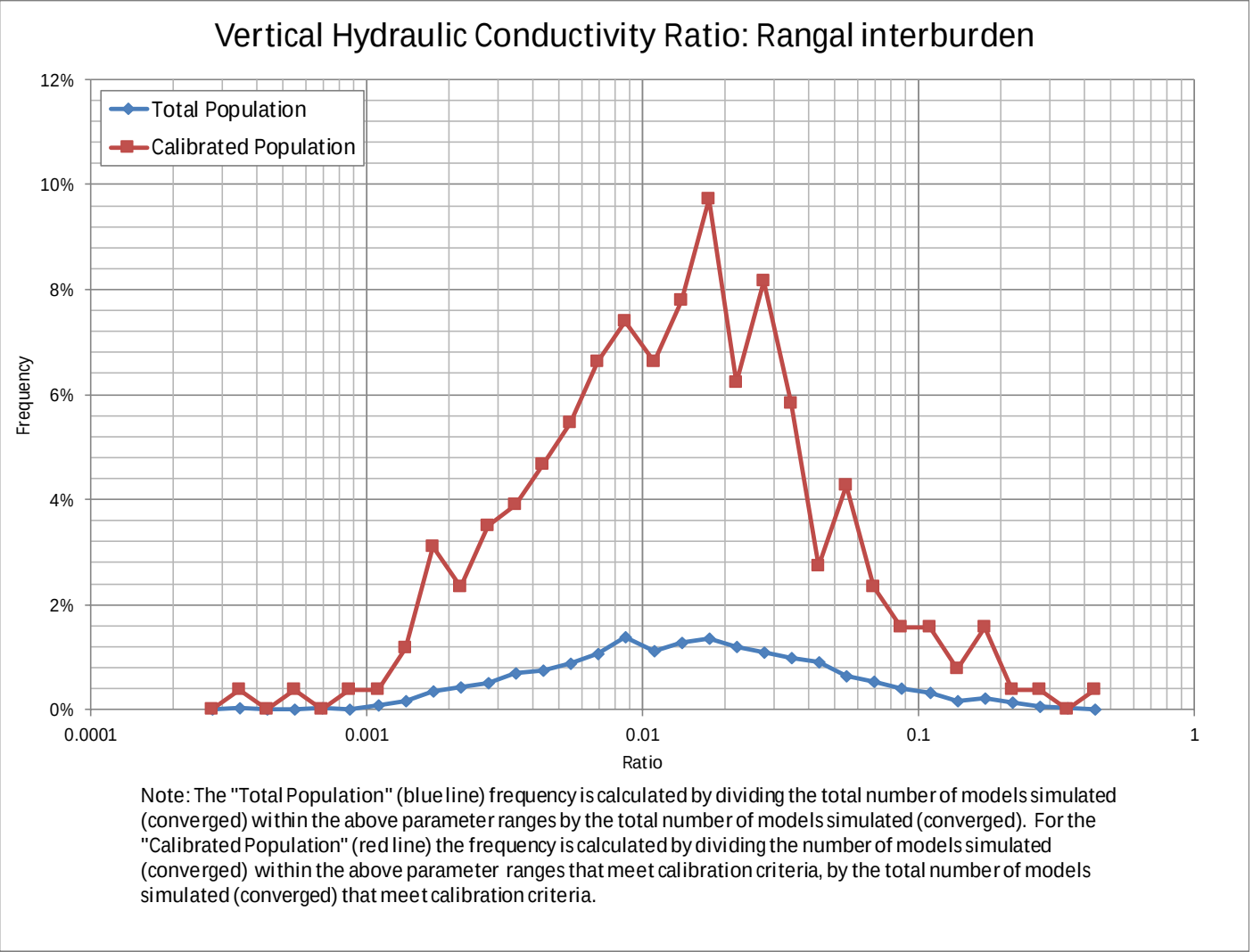


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

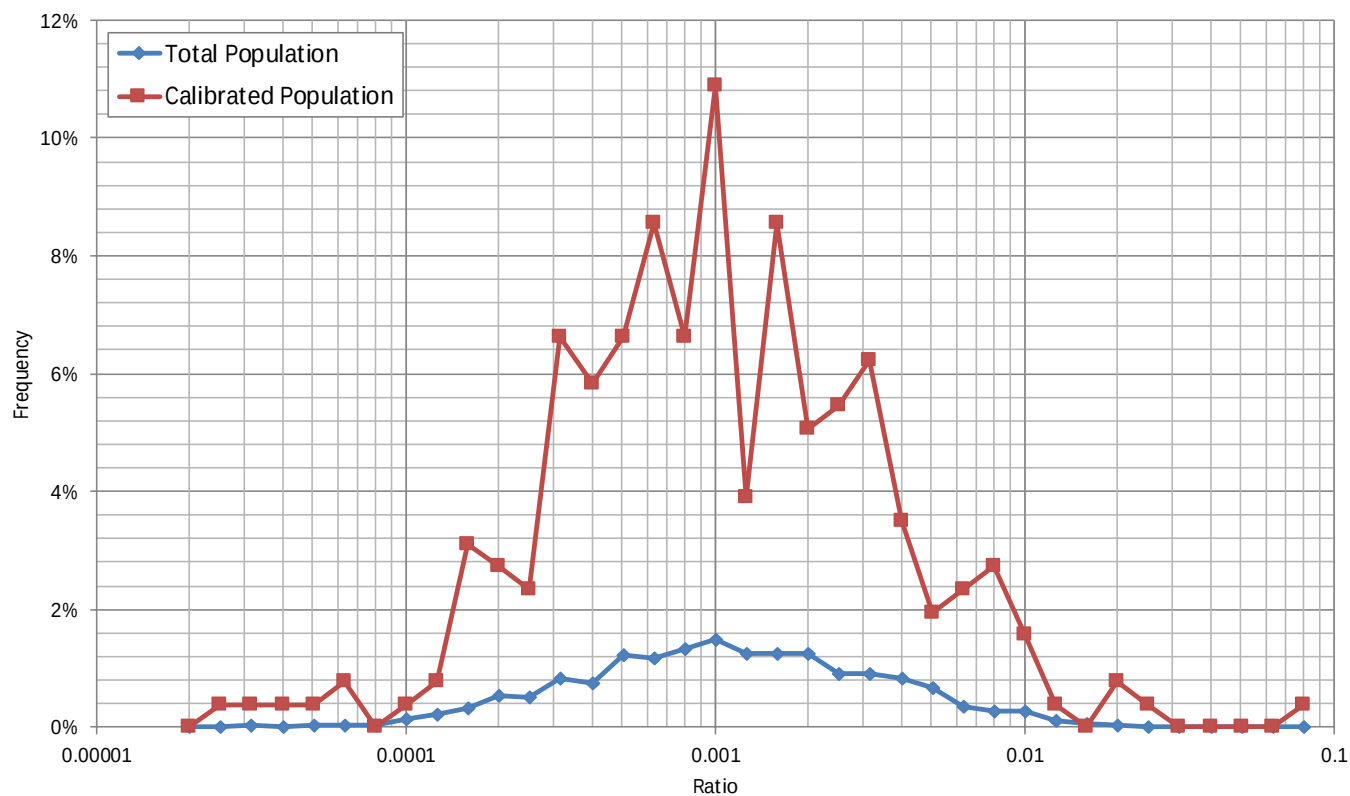
Vertical Hydraulic Conductivity Ratio: Leichardt





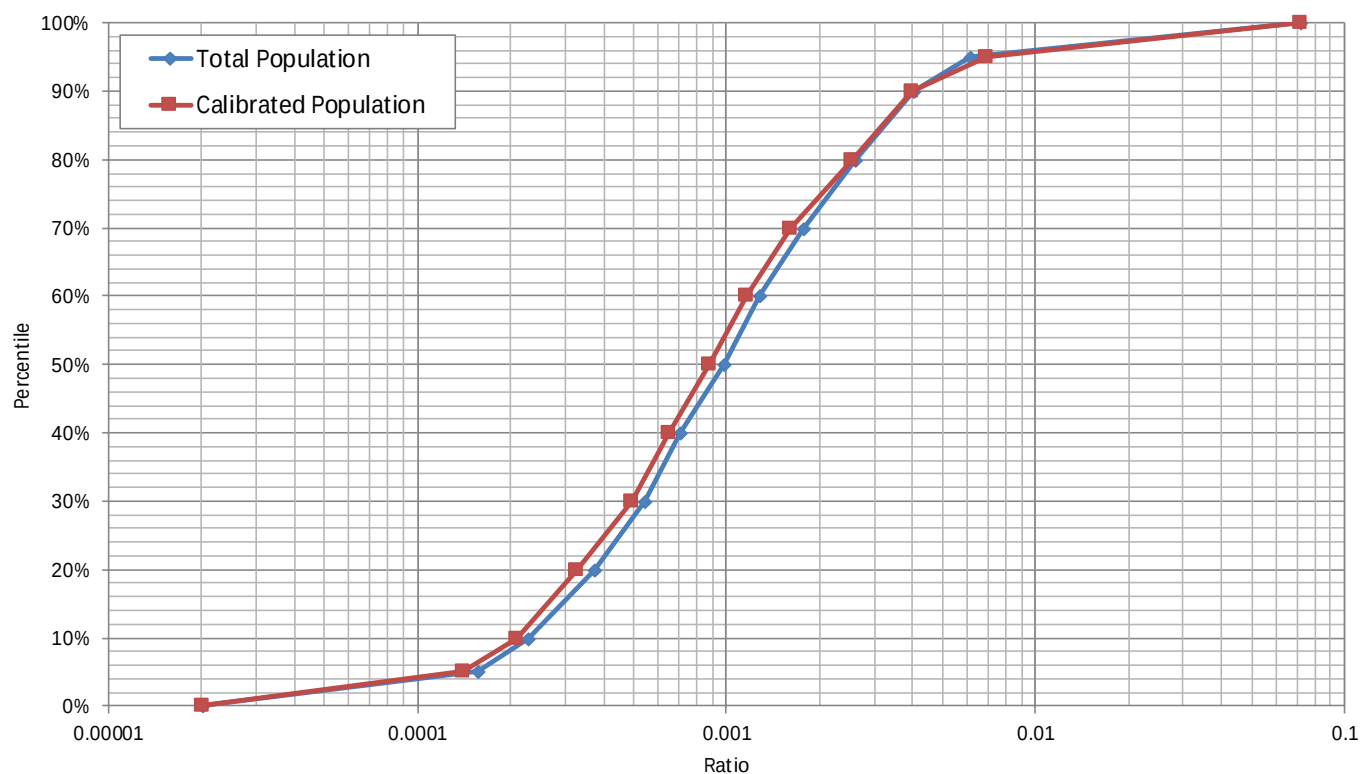


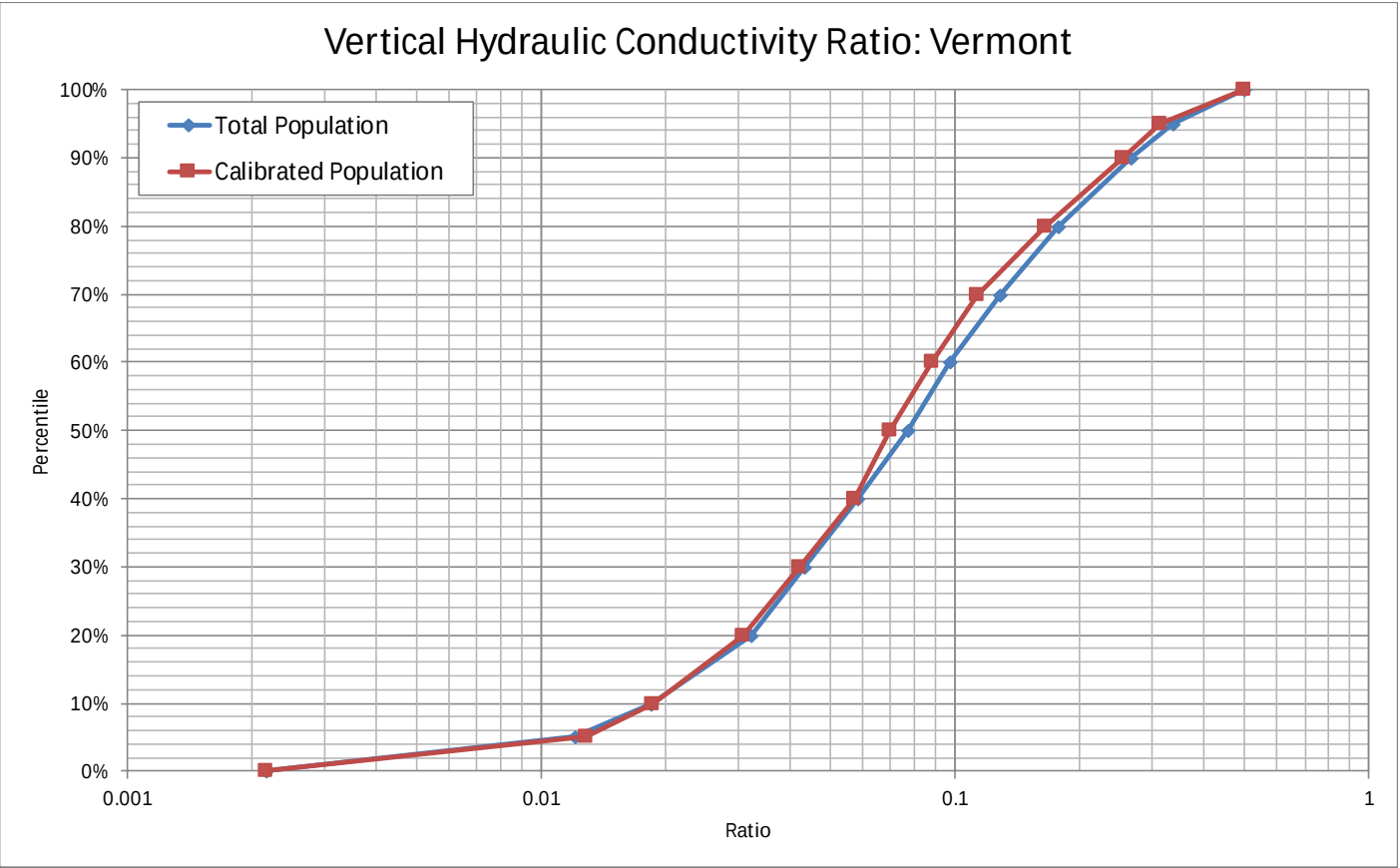
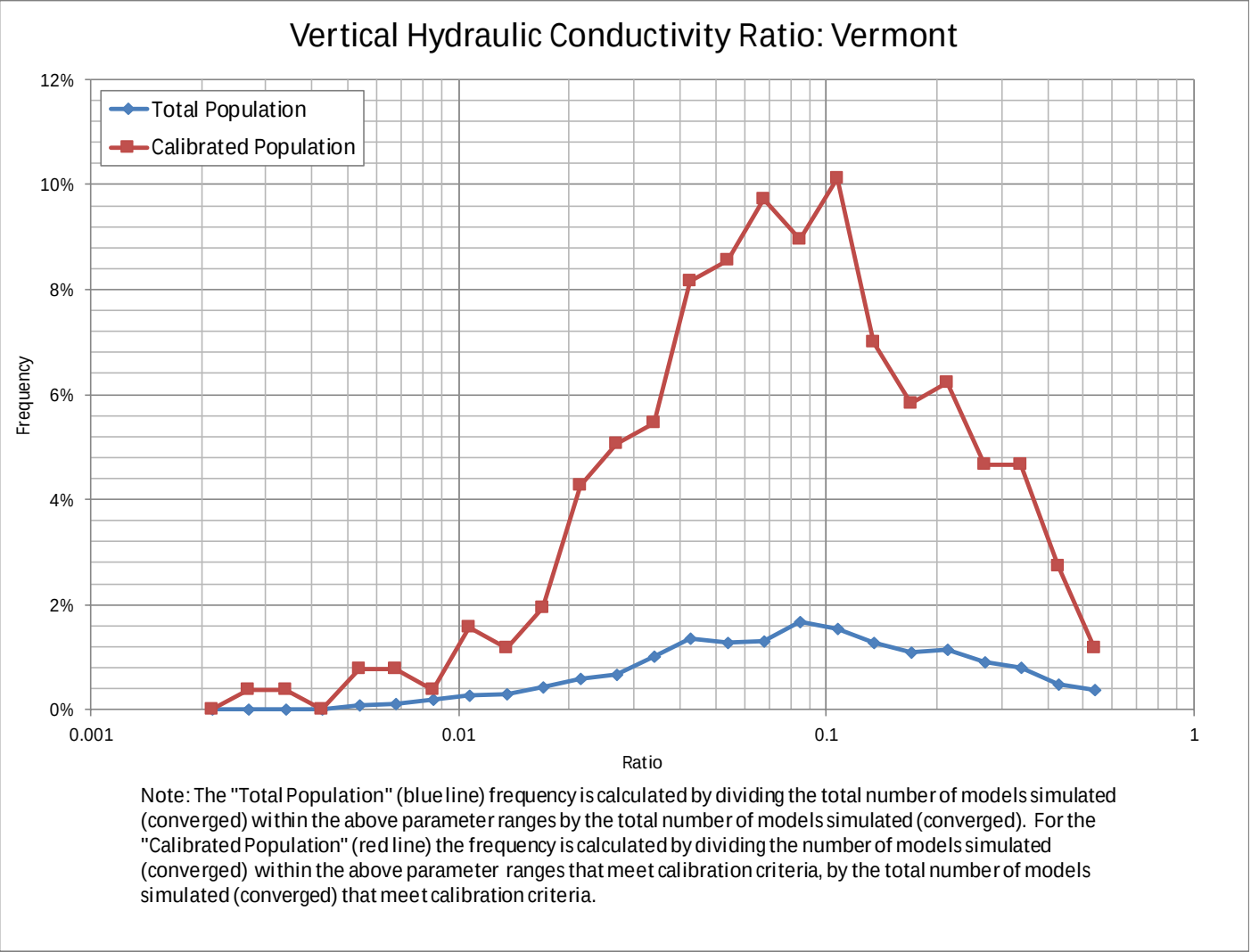
Vertical Hydraulic Conductivity Ratio: Fault - Rangal interburden

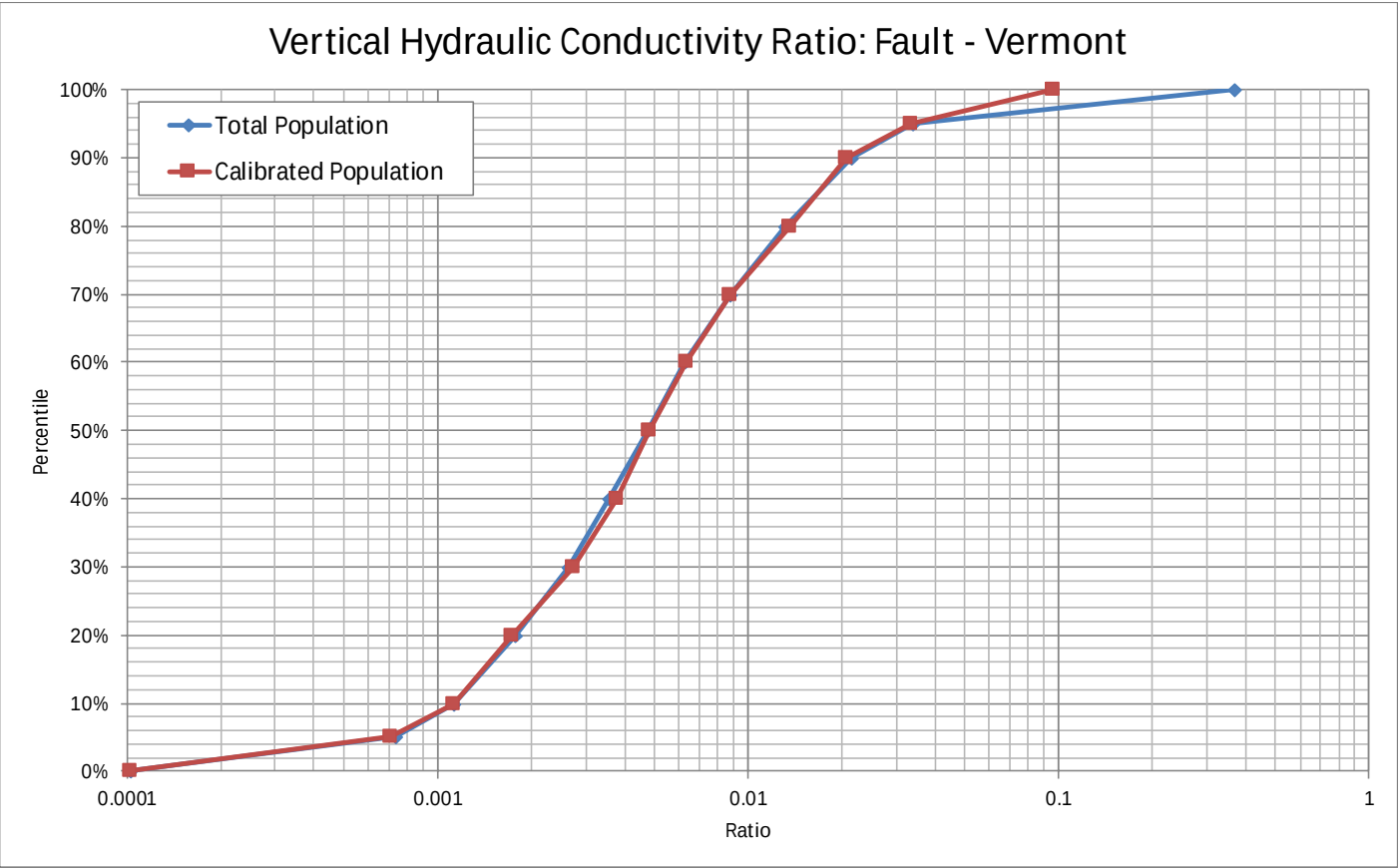
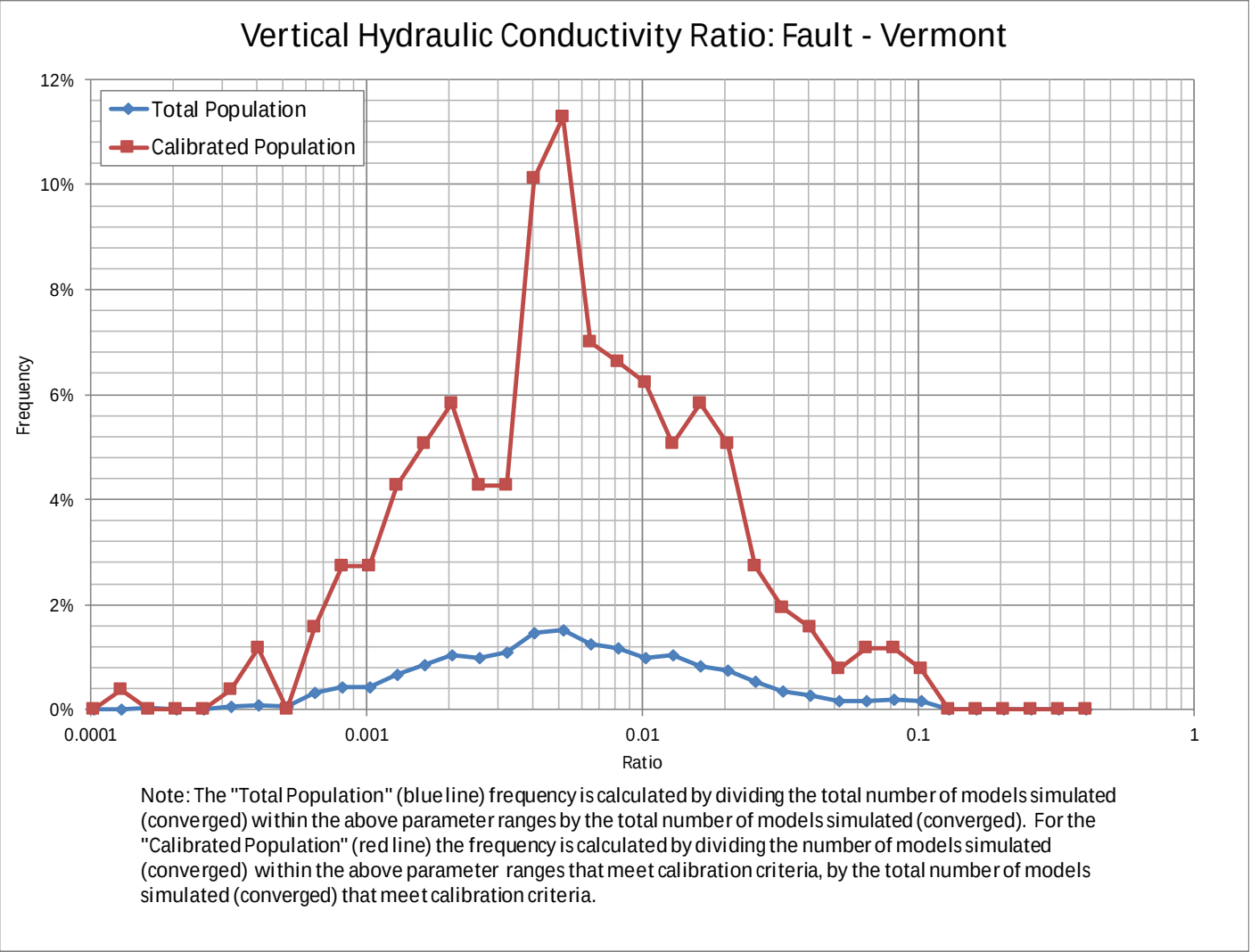


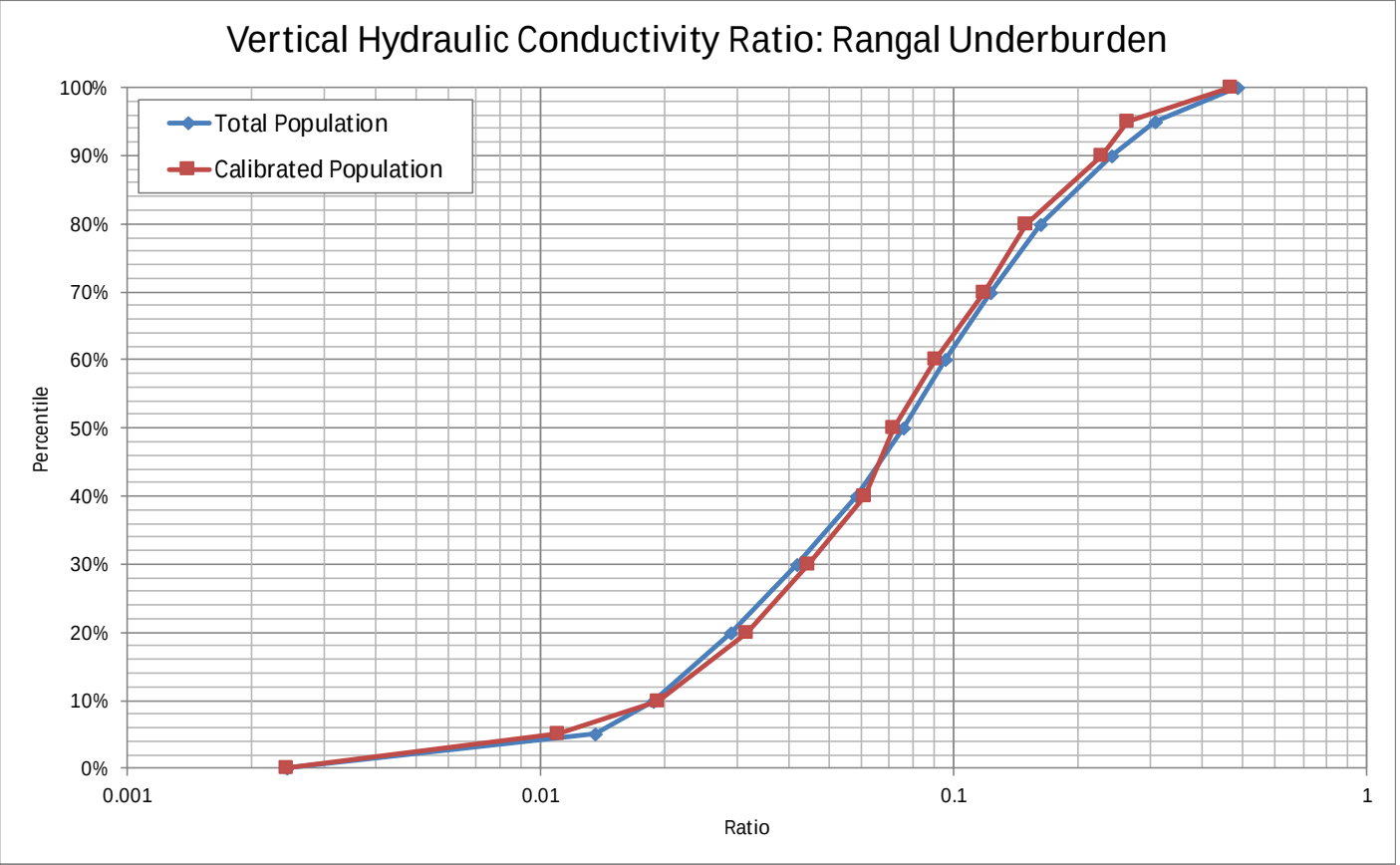
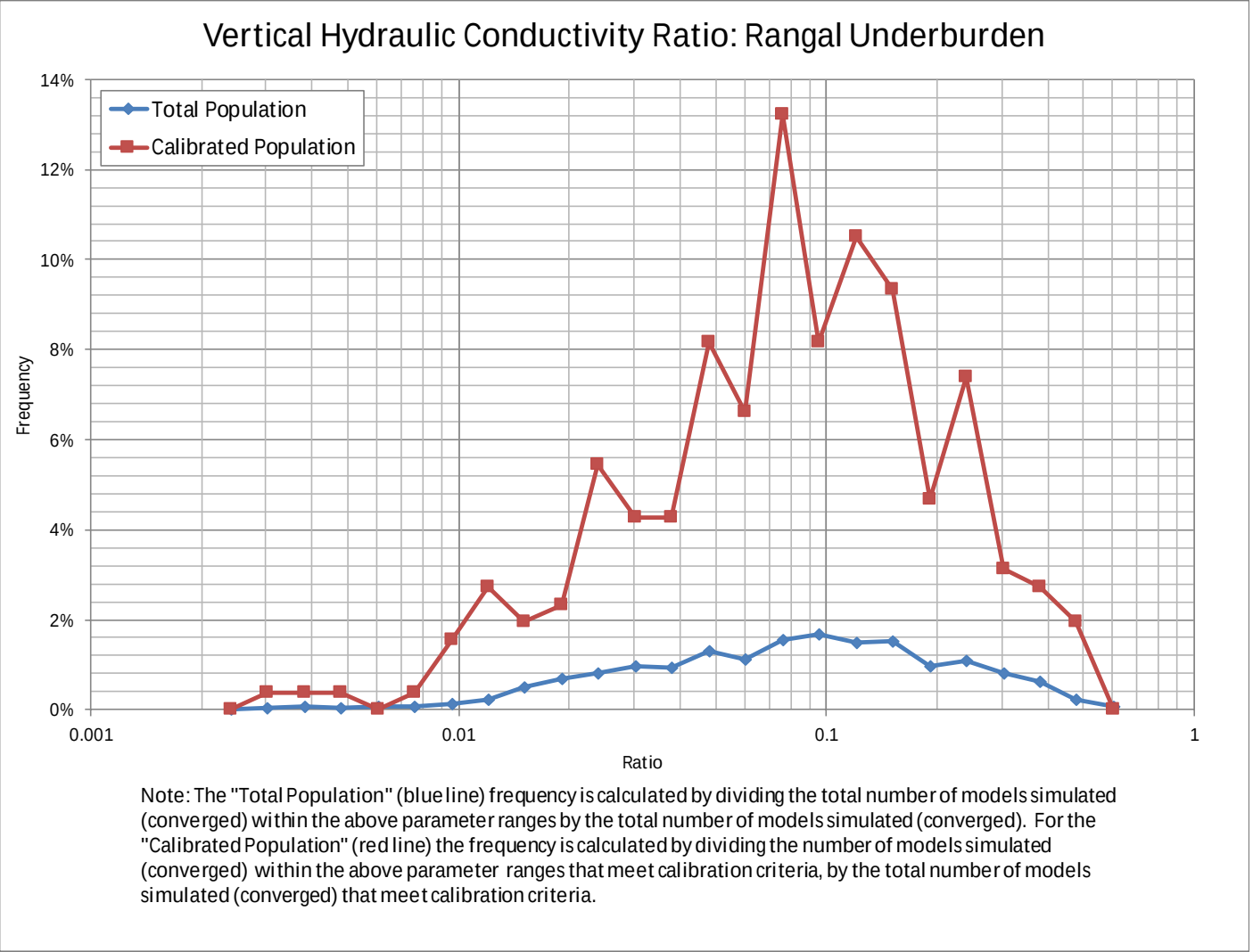
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

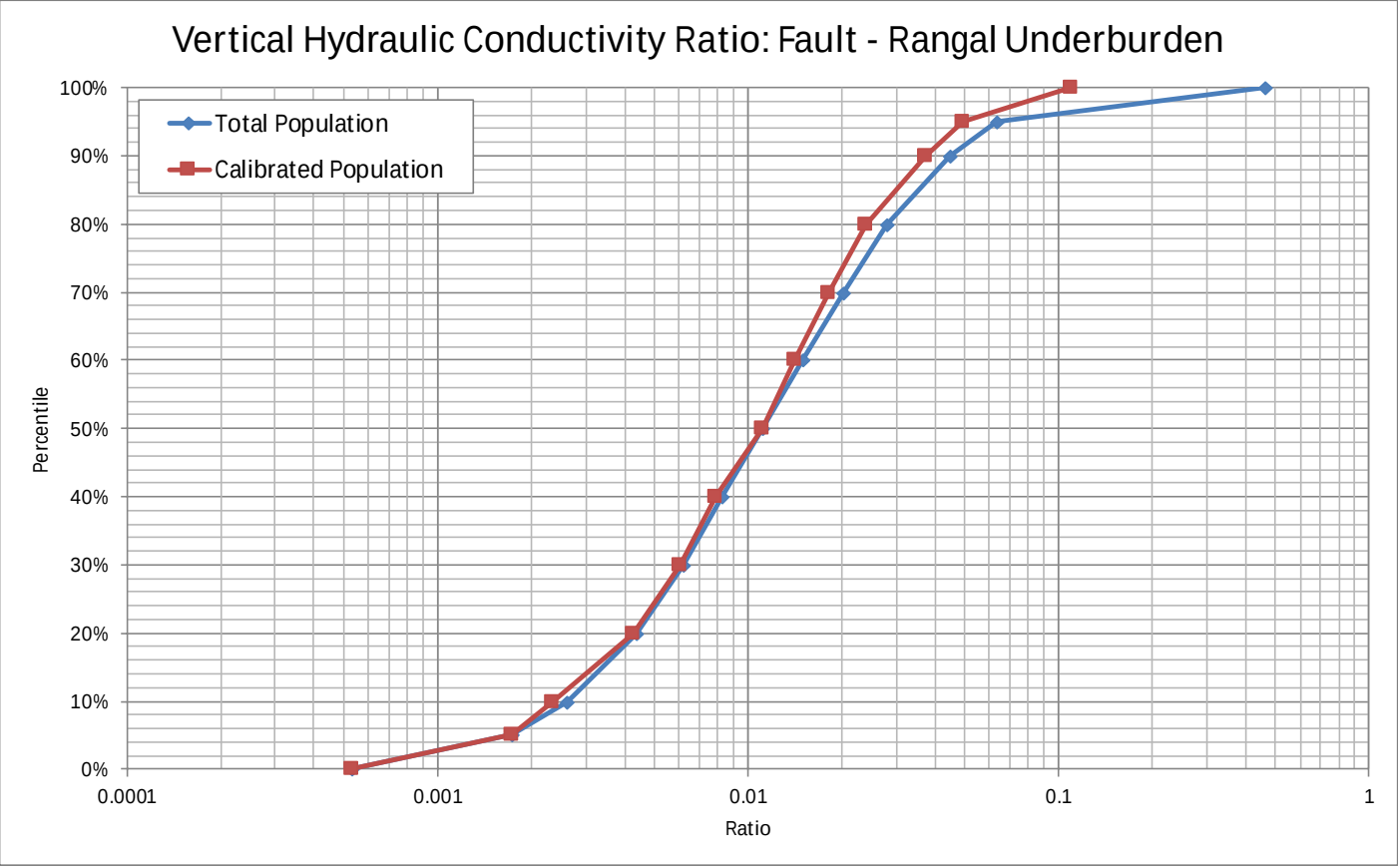
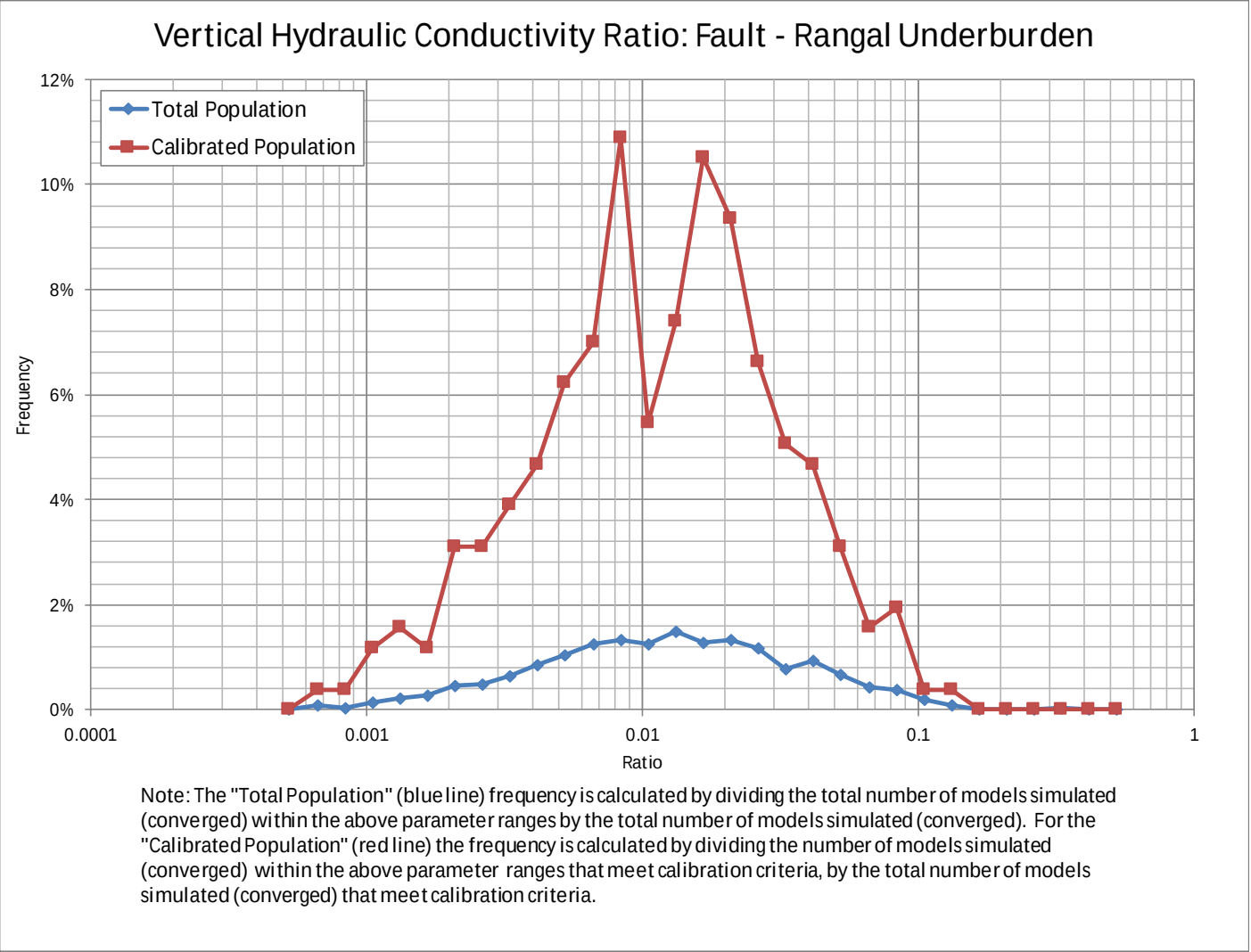
Vertical Hydraulic Conductivity Ratio: Fault - Rangal interburden

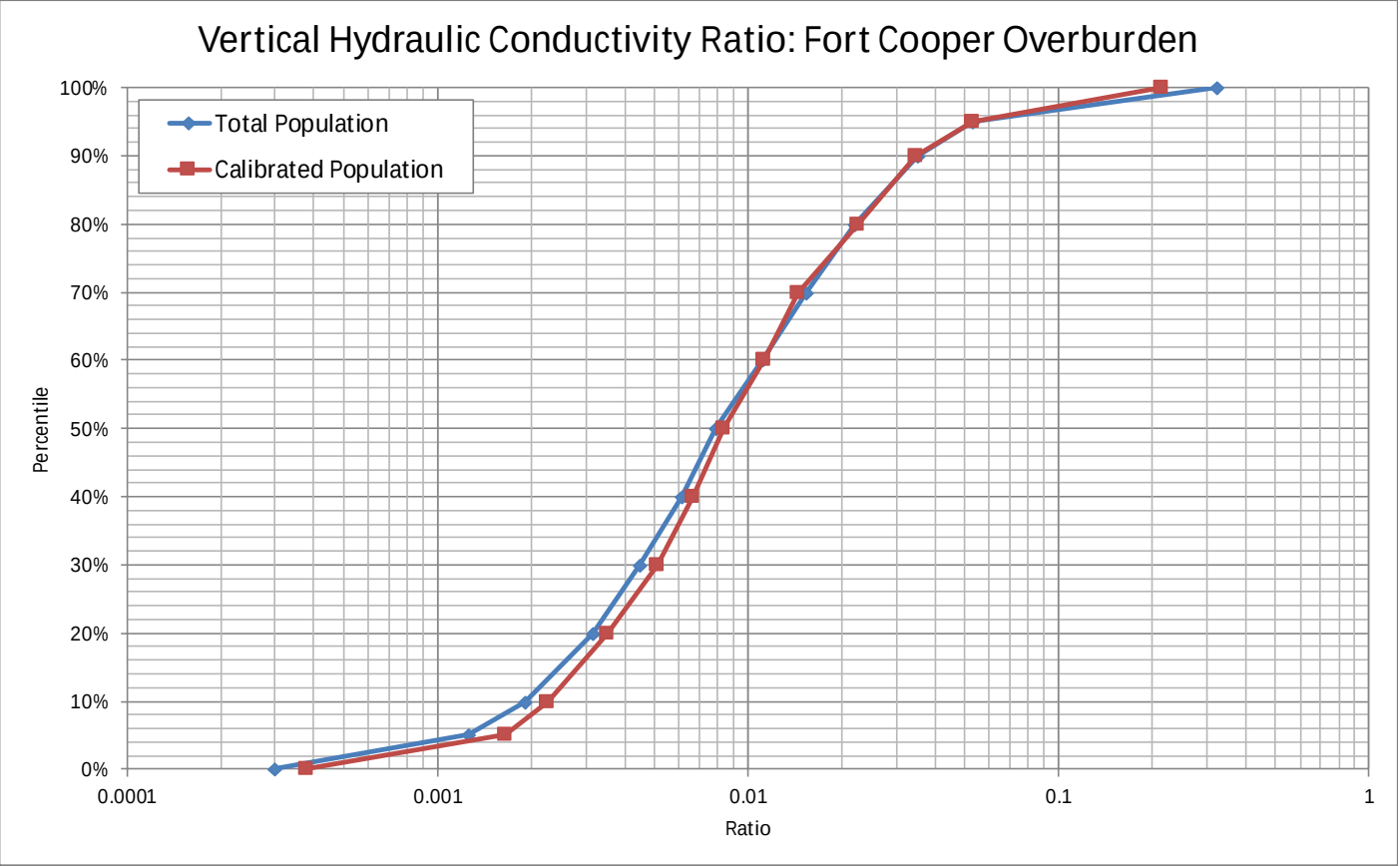
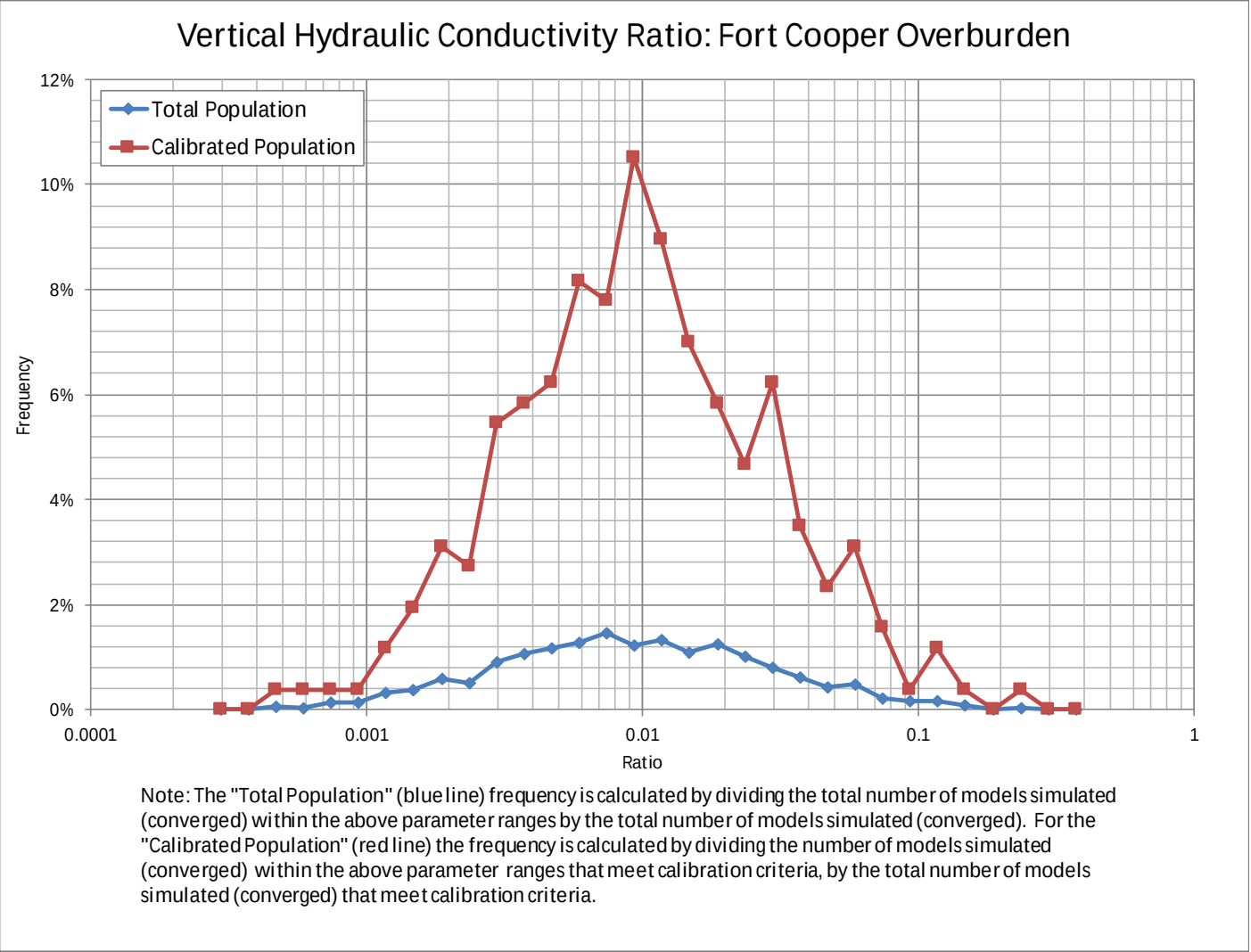


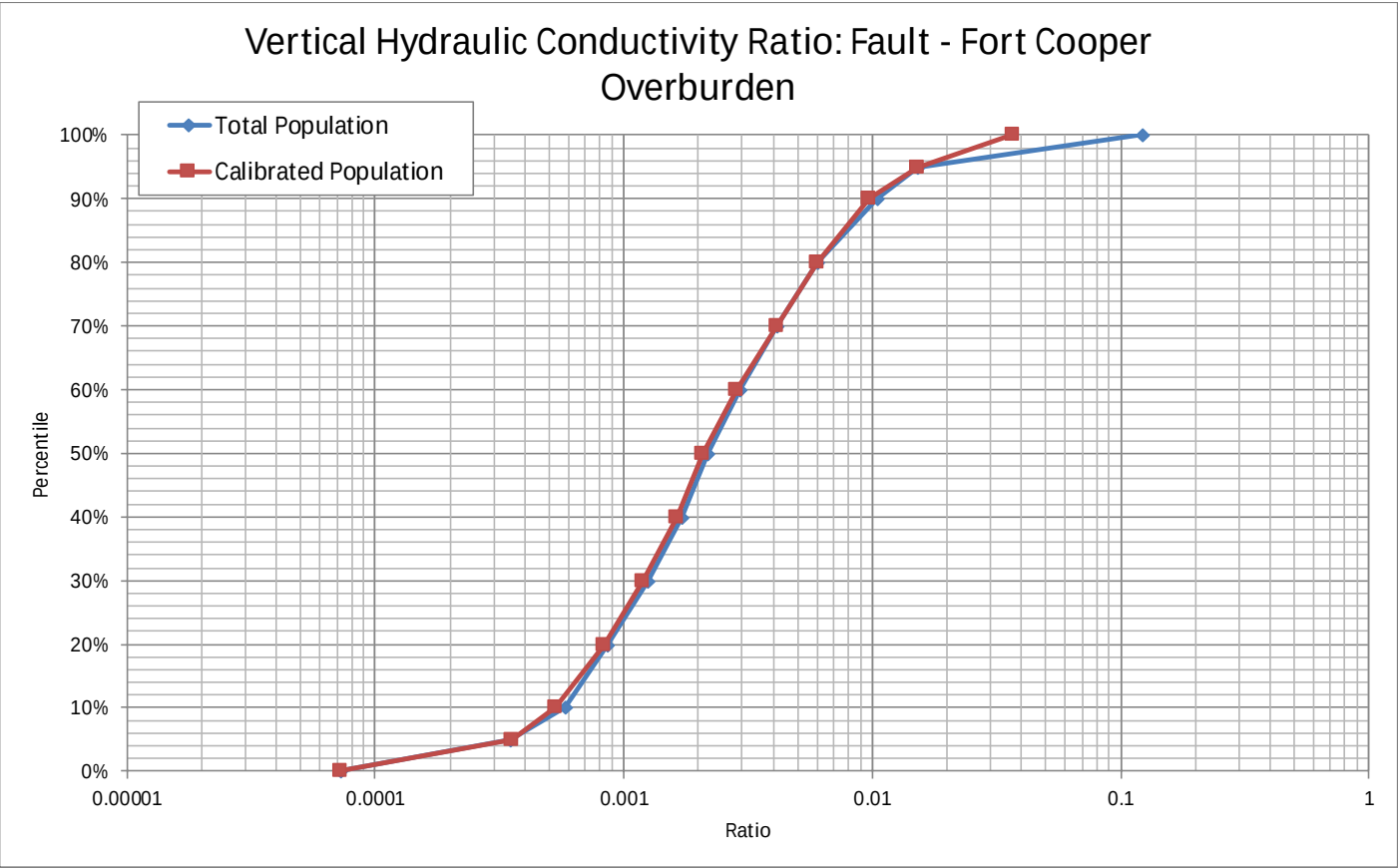
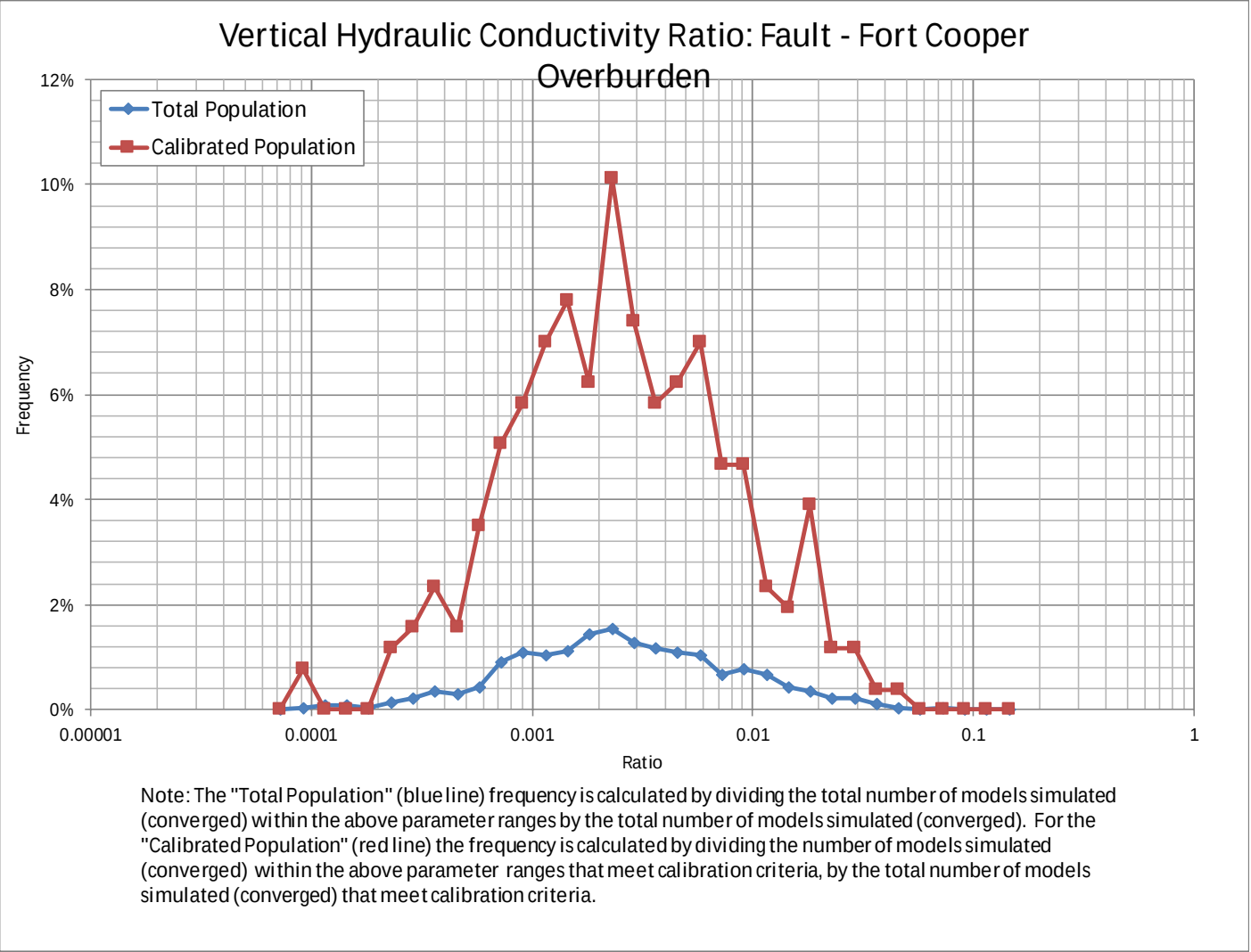




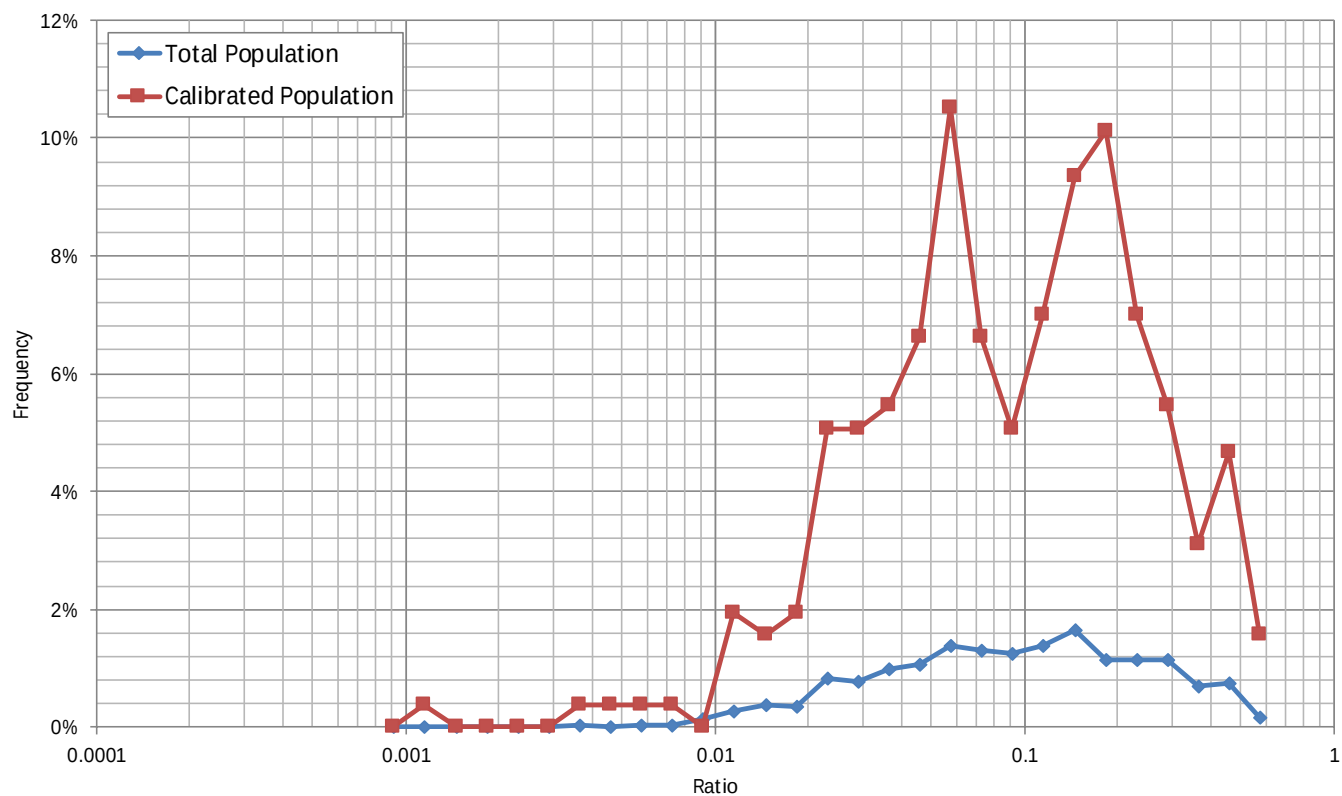






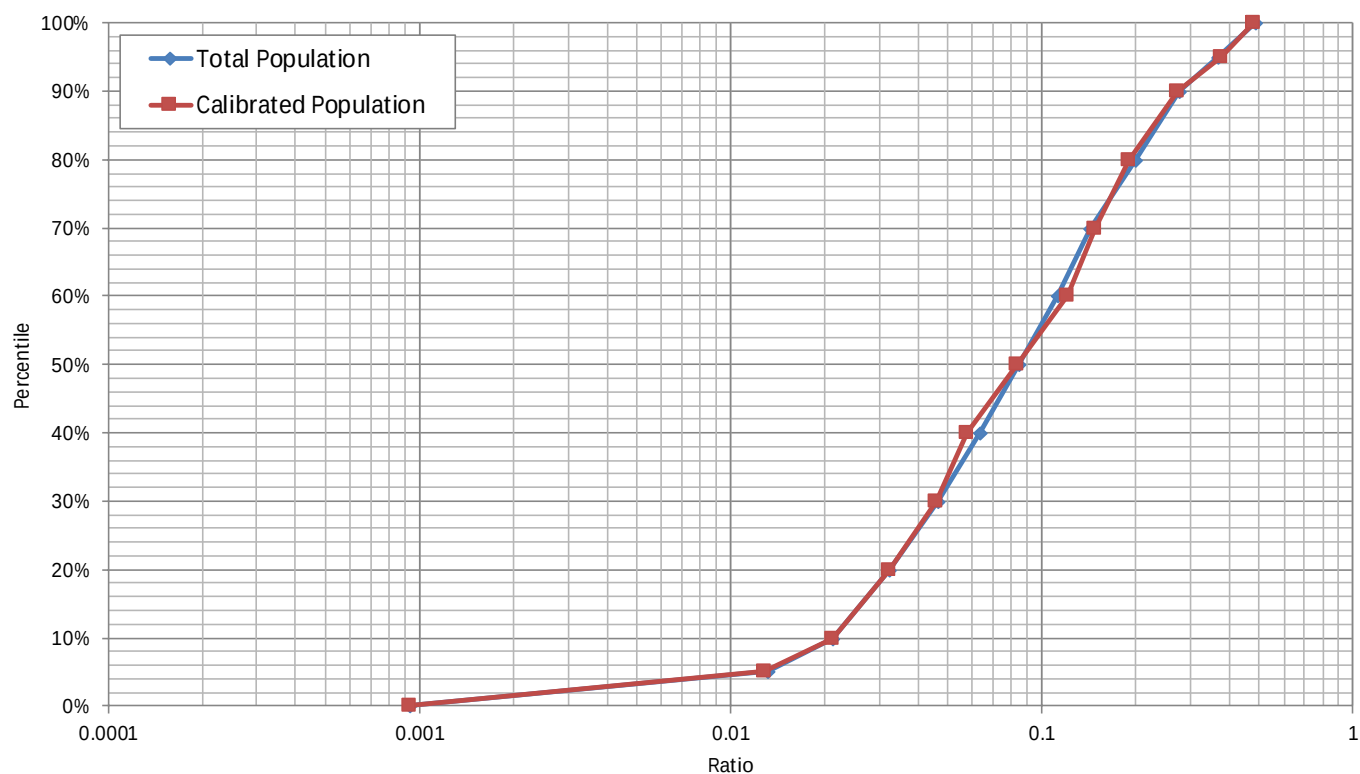


Vertical Hydraulic Conductivity Ratio: Black Alley Shale

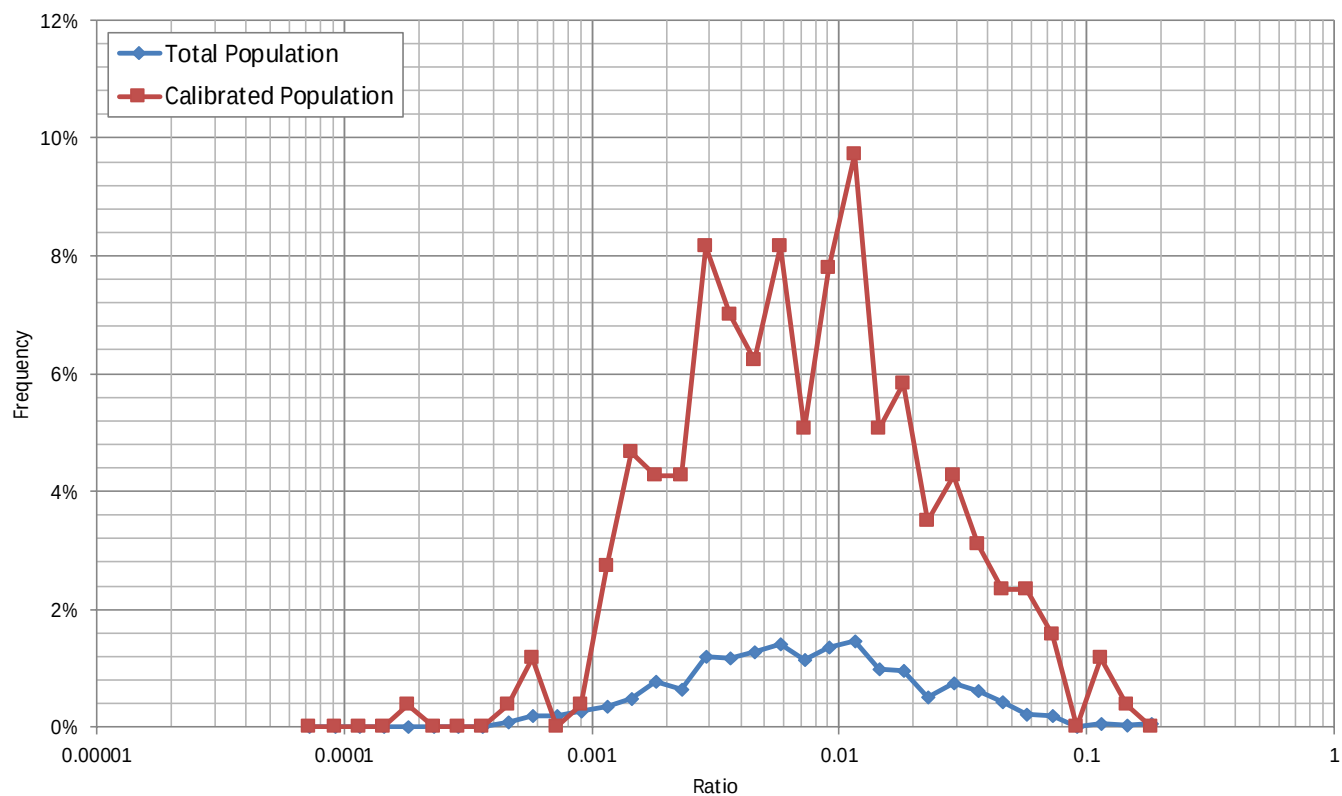


Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Vertical Hydraulic Conductivity Ratio: Black Alley Shale

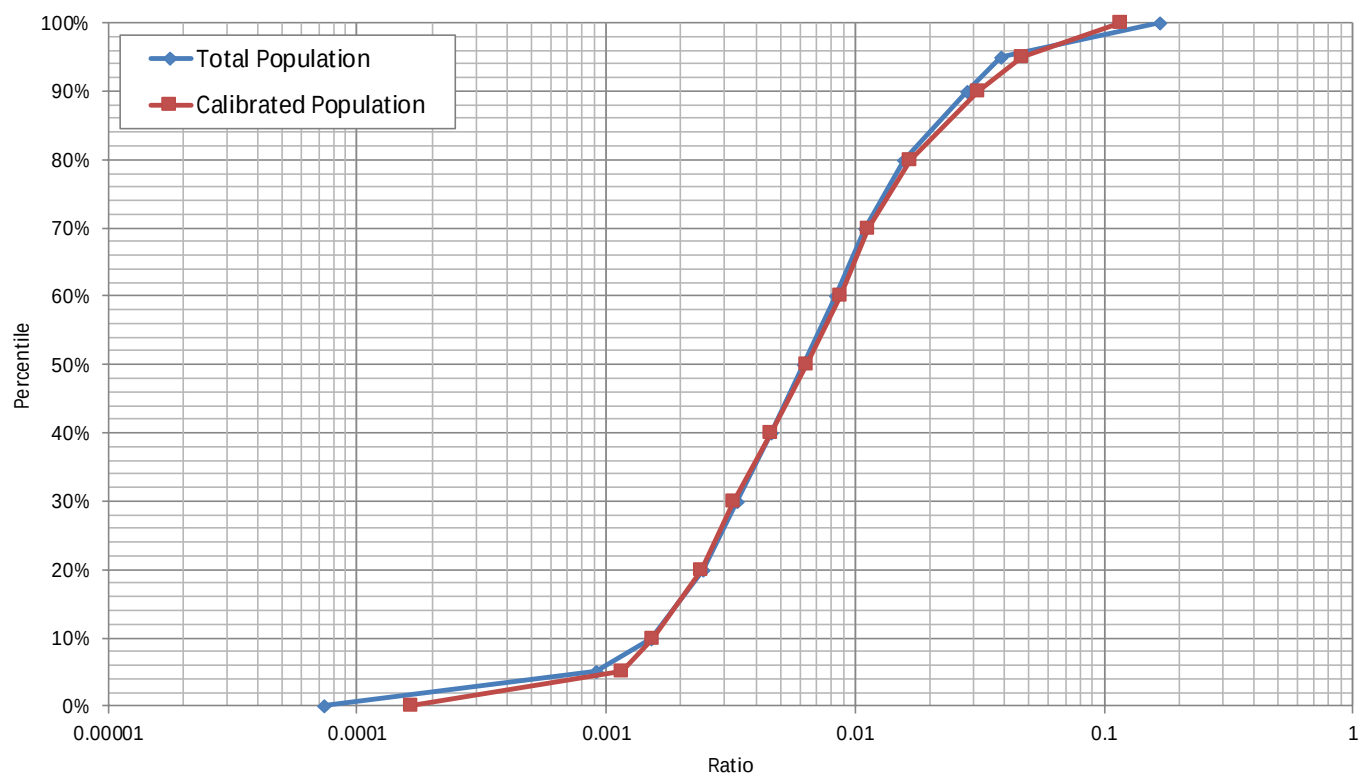


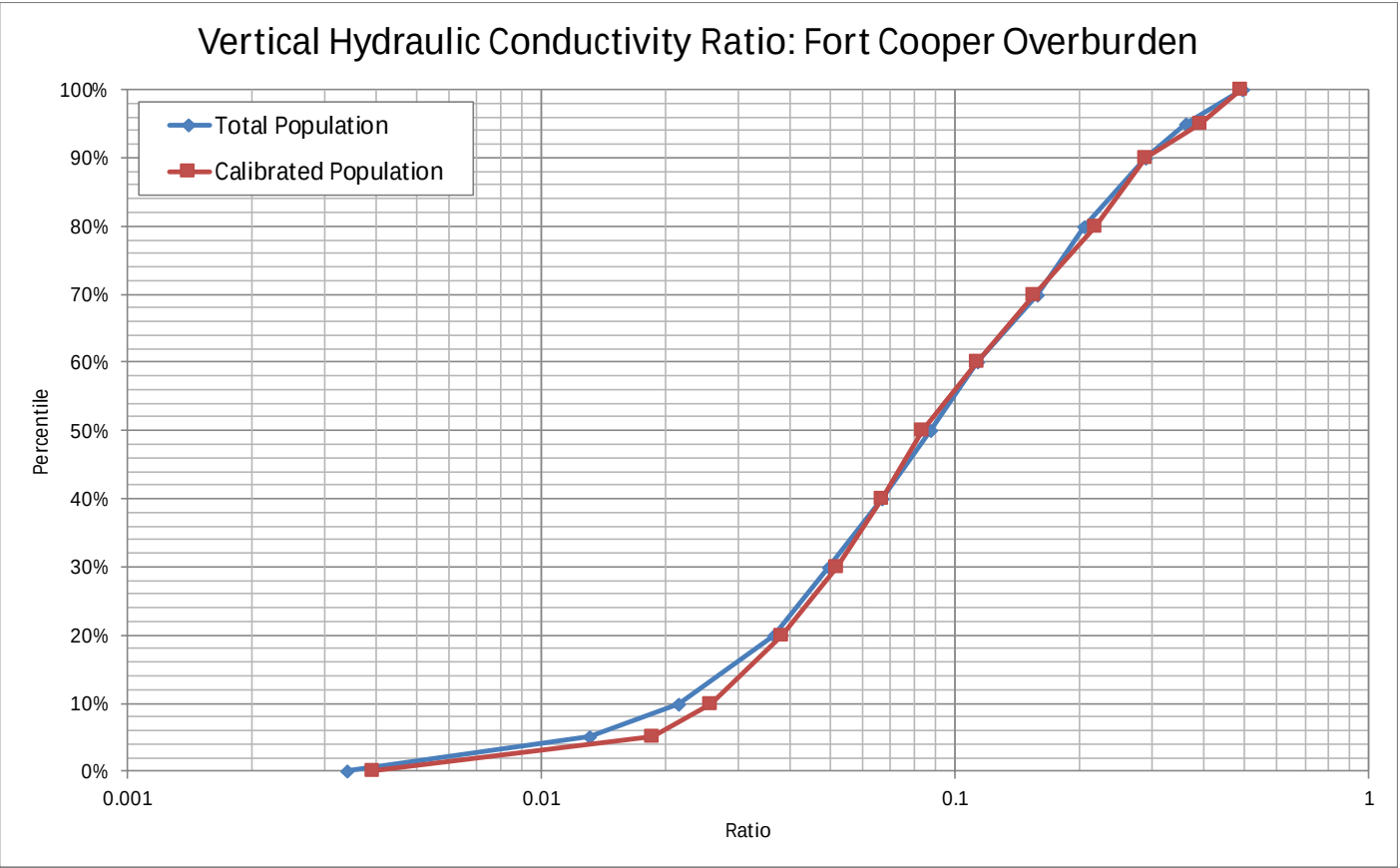
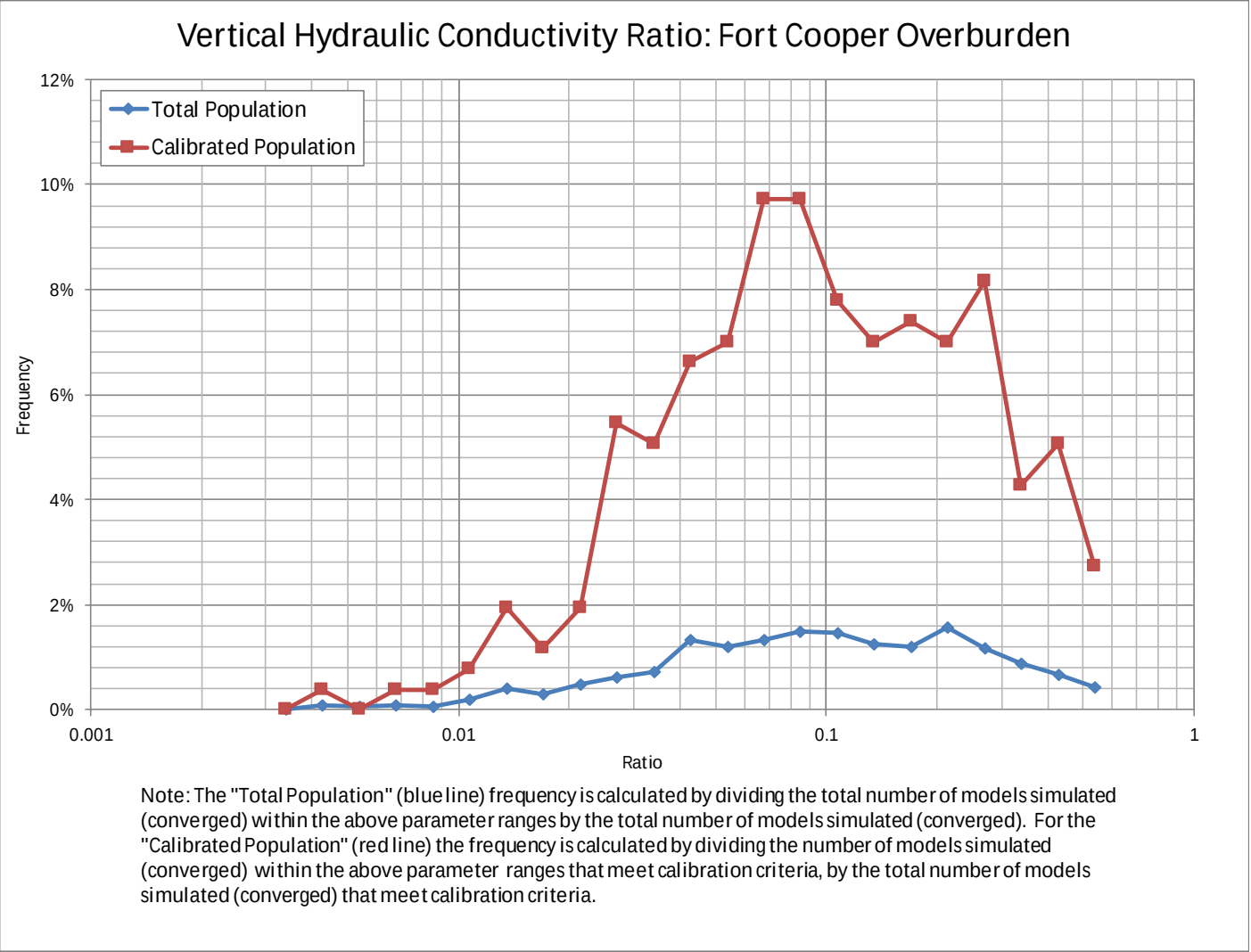
Vertical Hydraulic Conductivity Ratio: Fault - Black Alley Shale

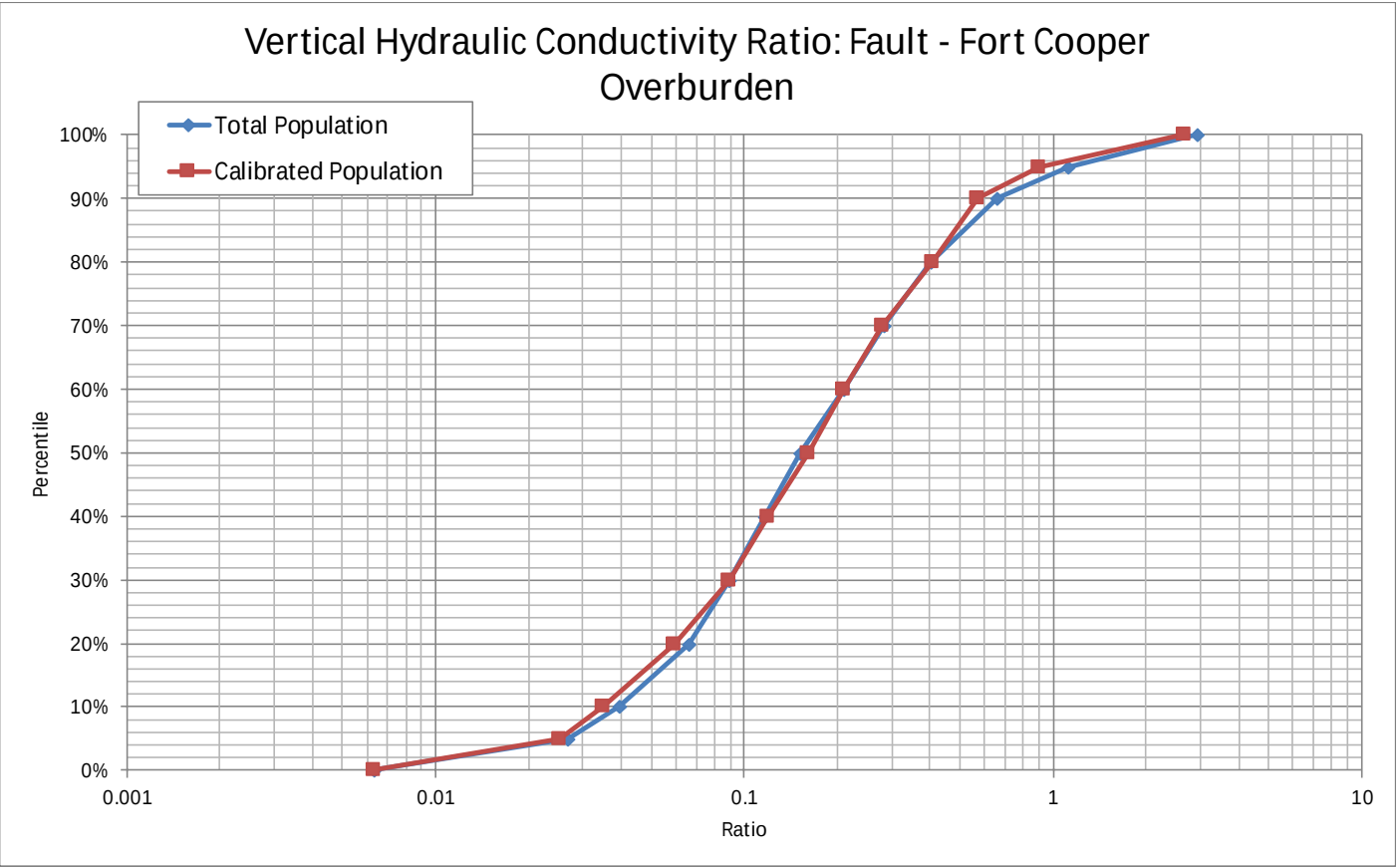
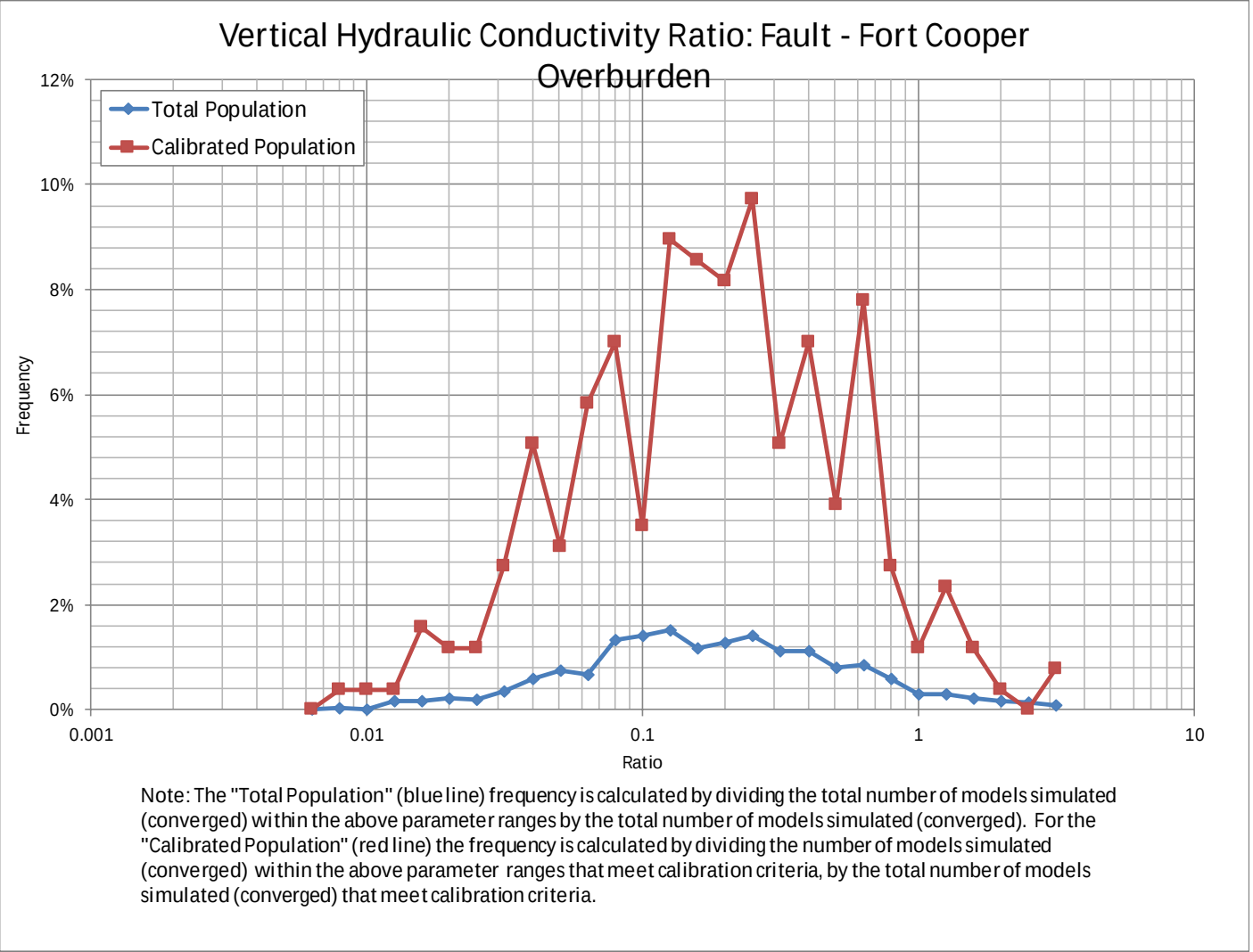


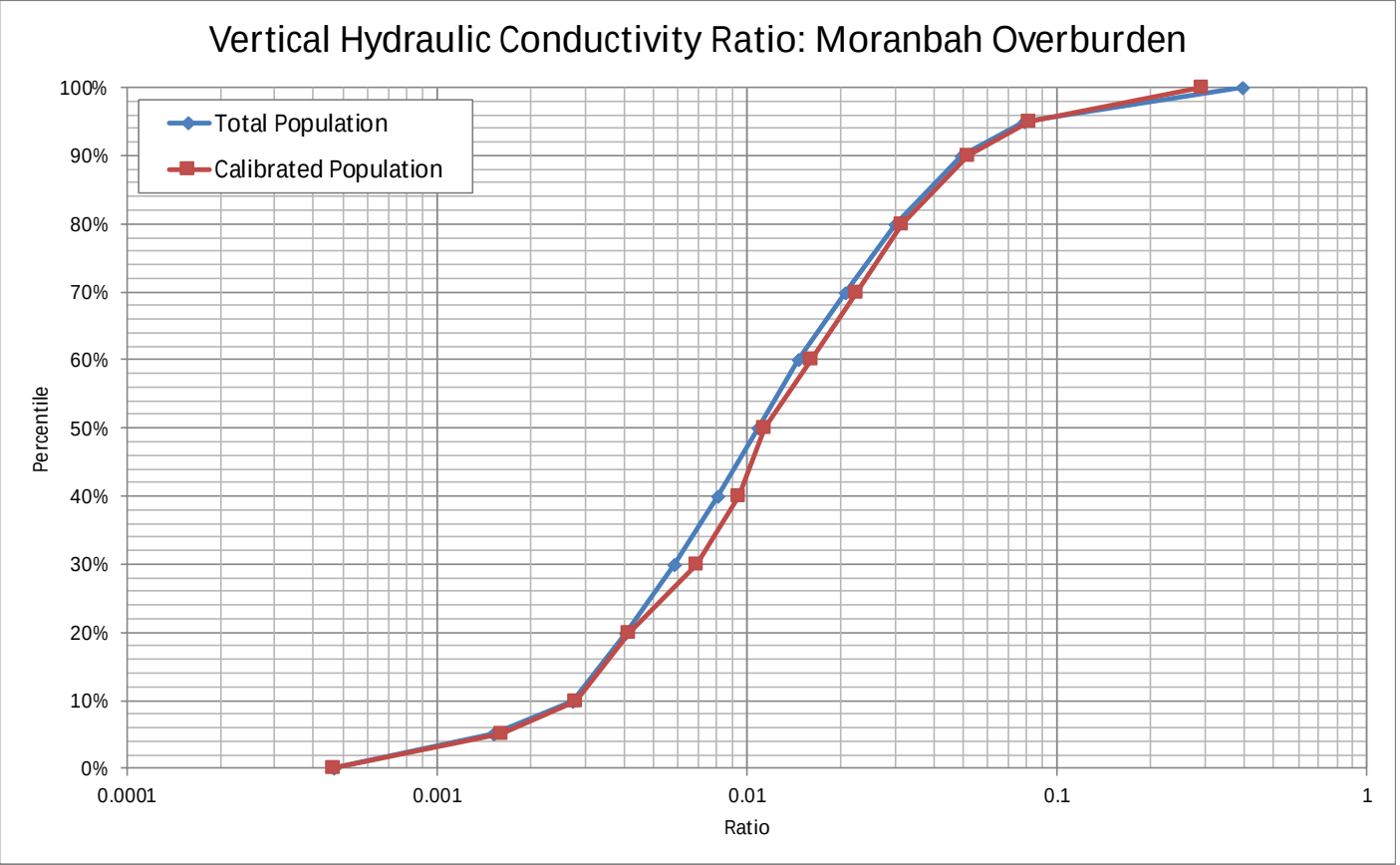
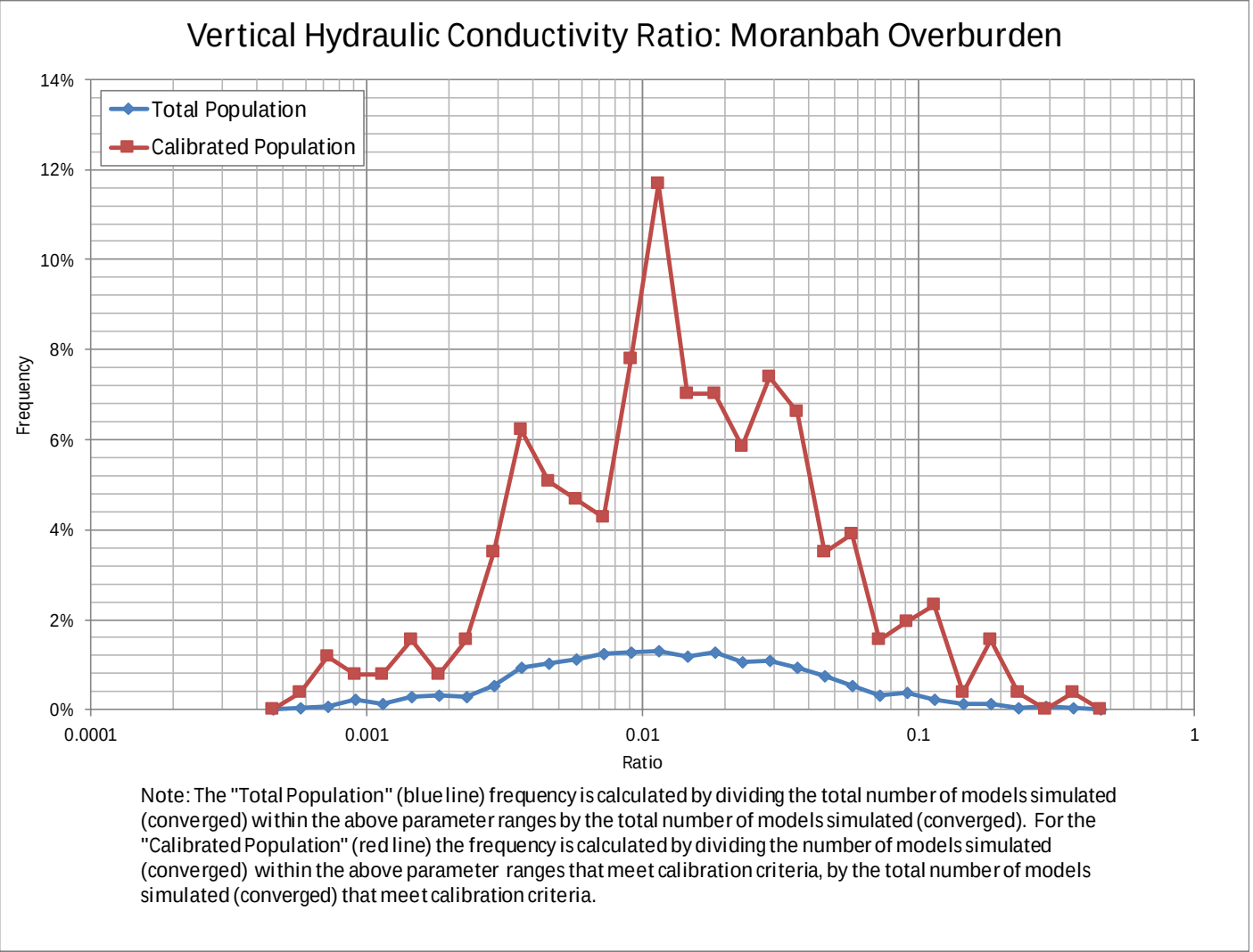
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

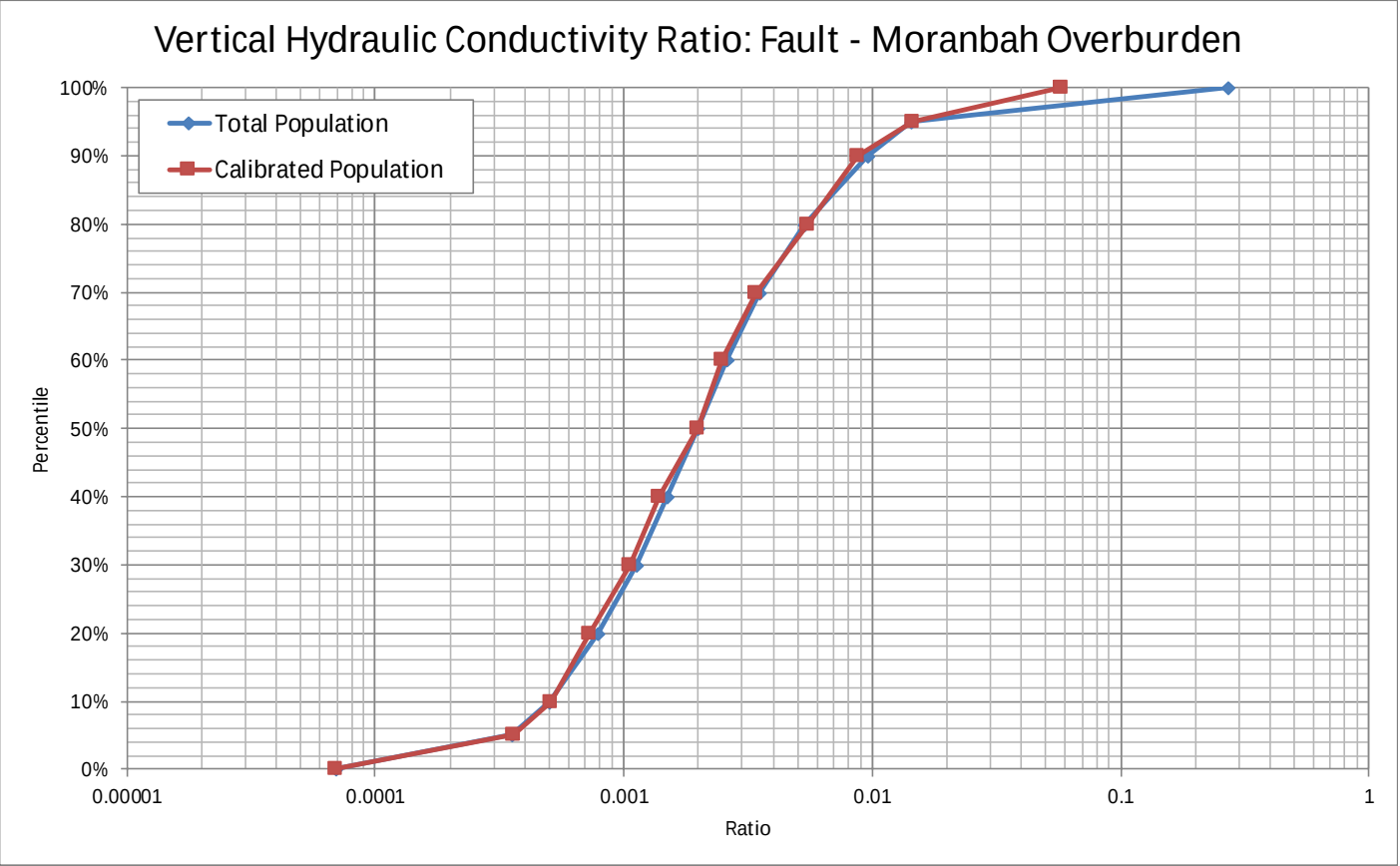
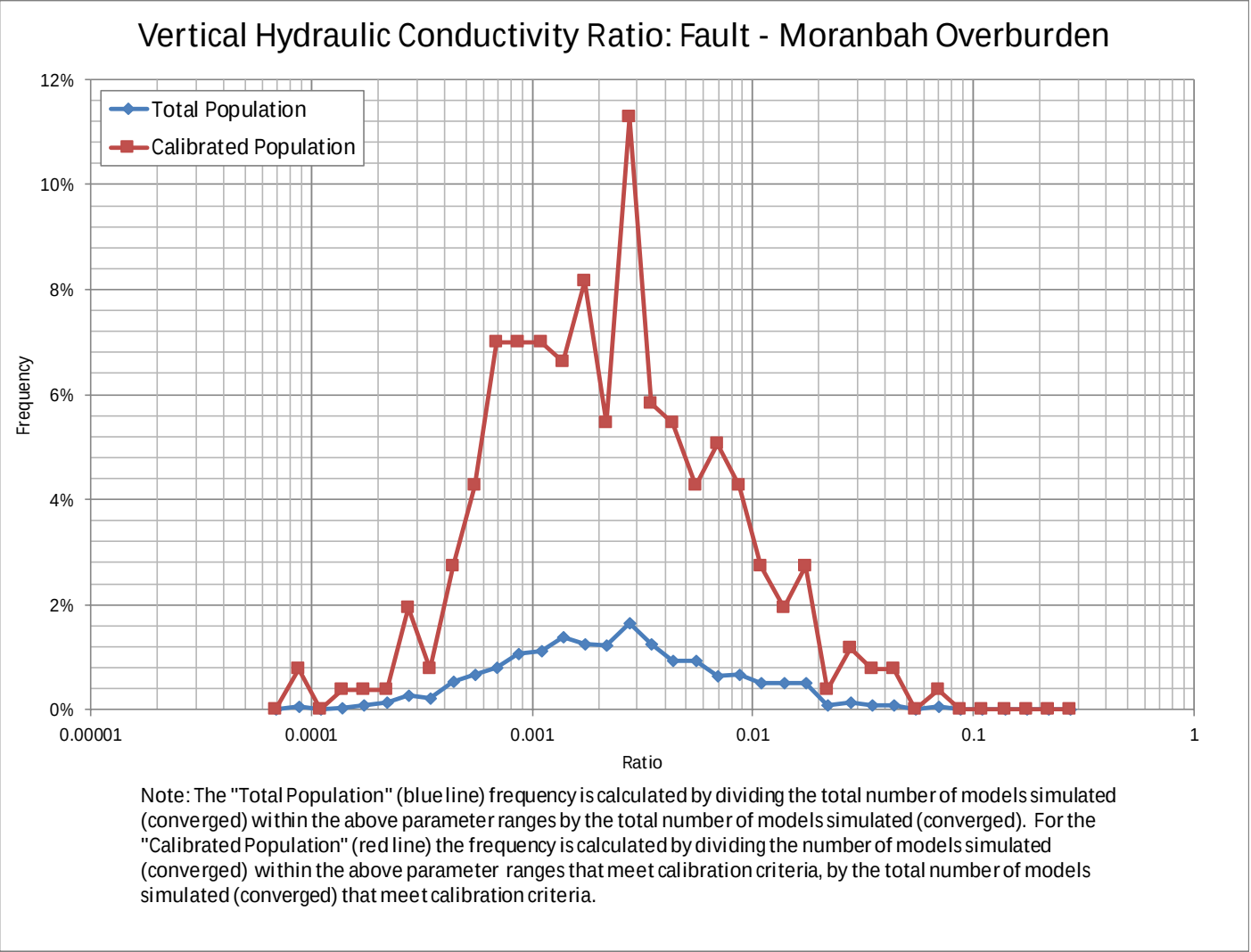
Vertical Hydraulic Conductivity Ratio: Fault - Black Alley Shale

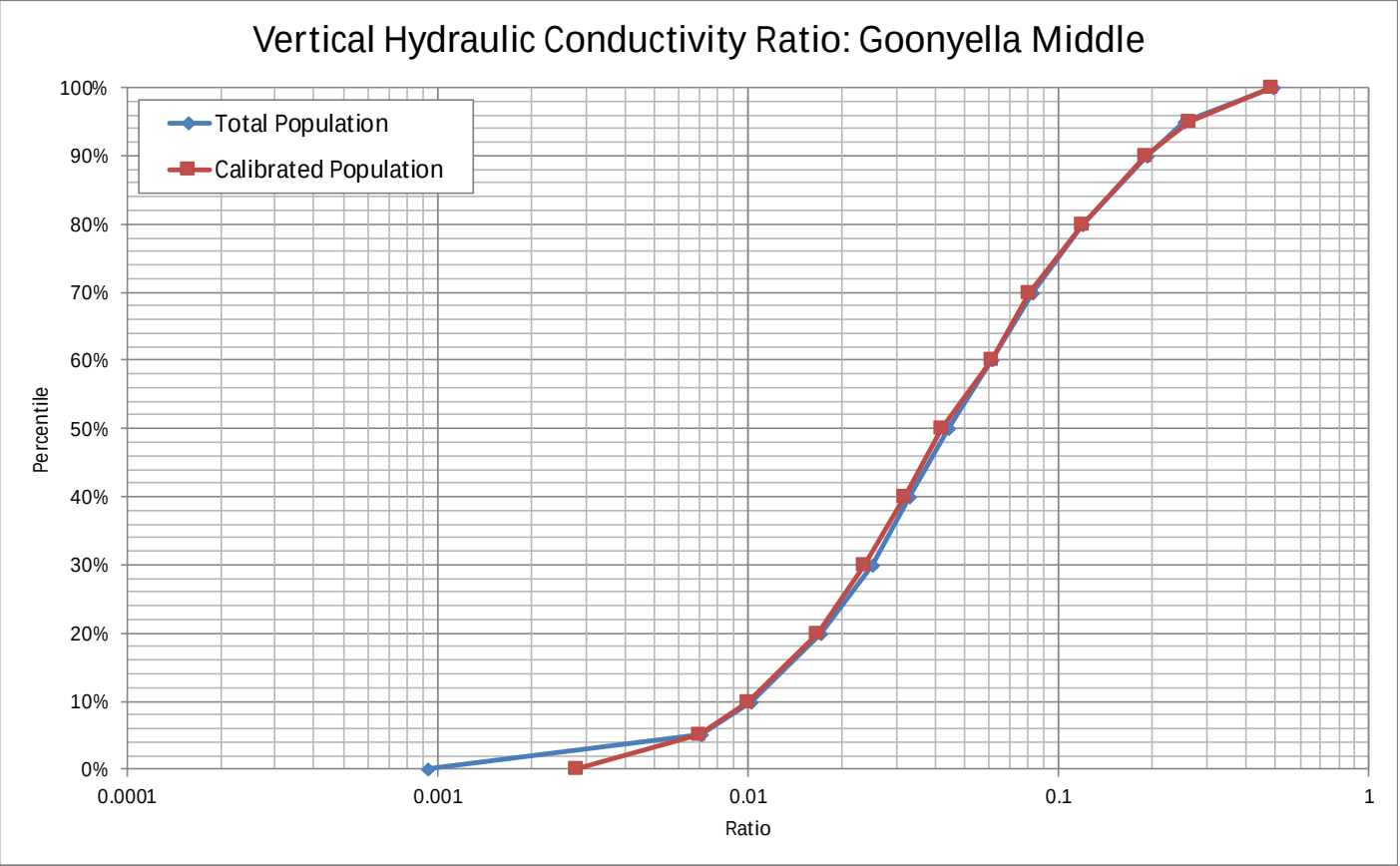
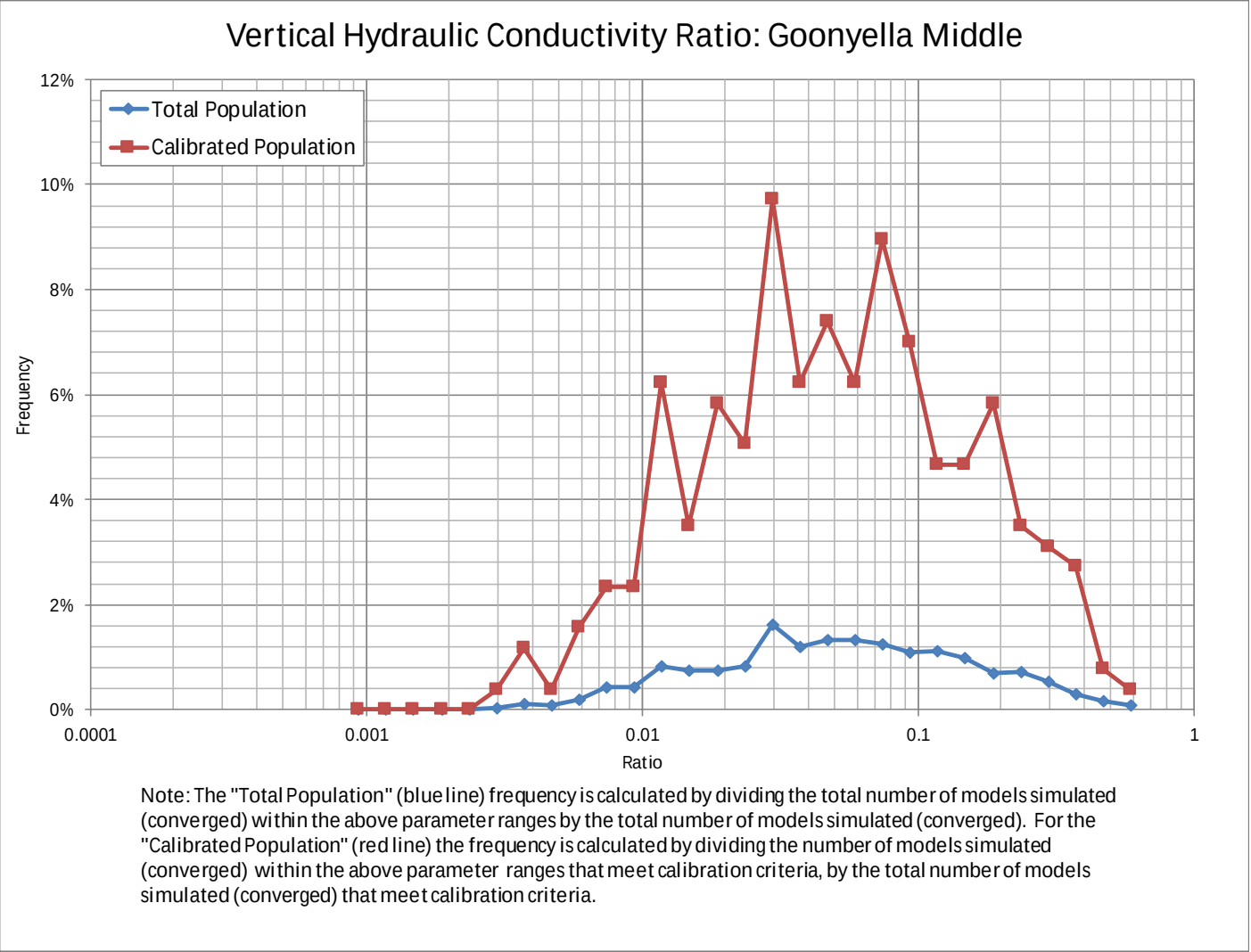


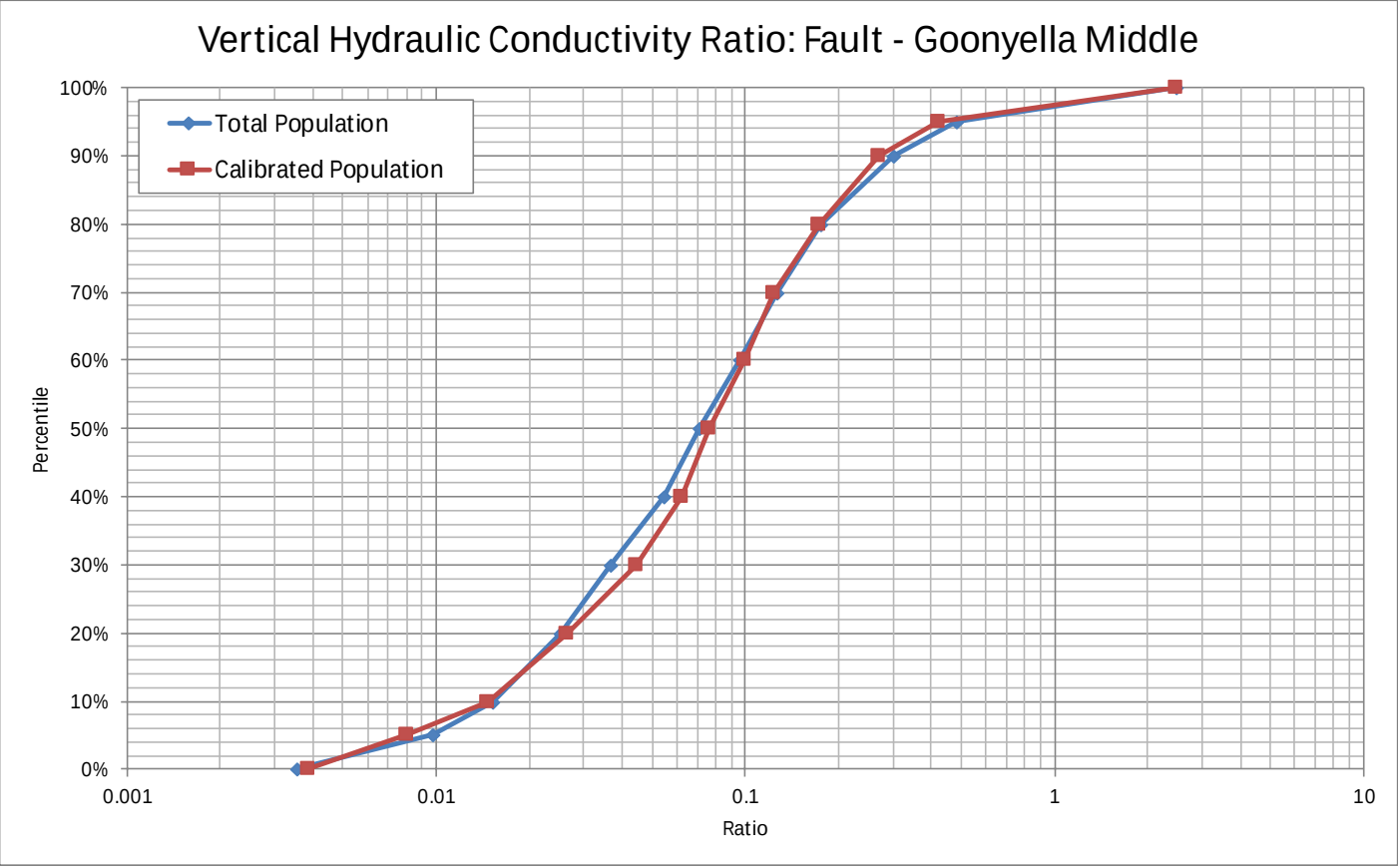
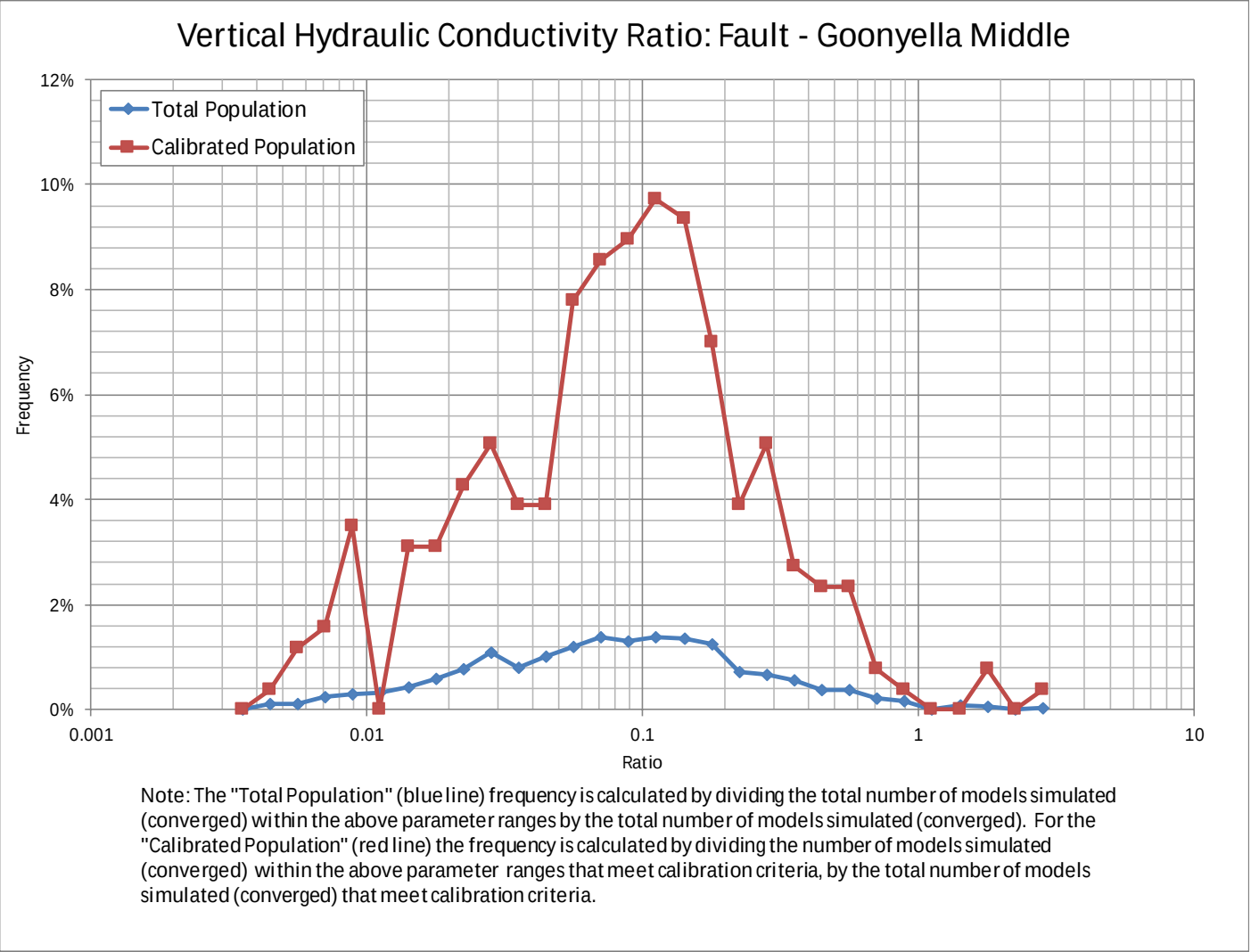


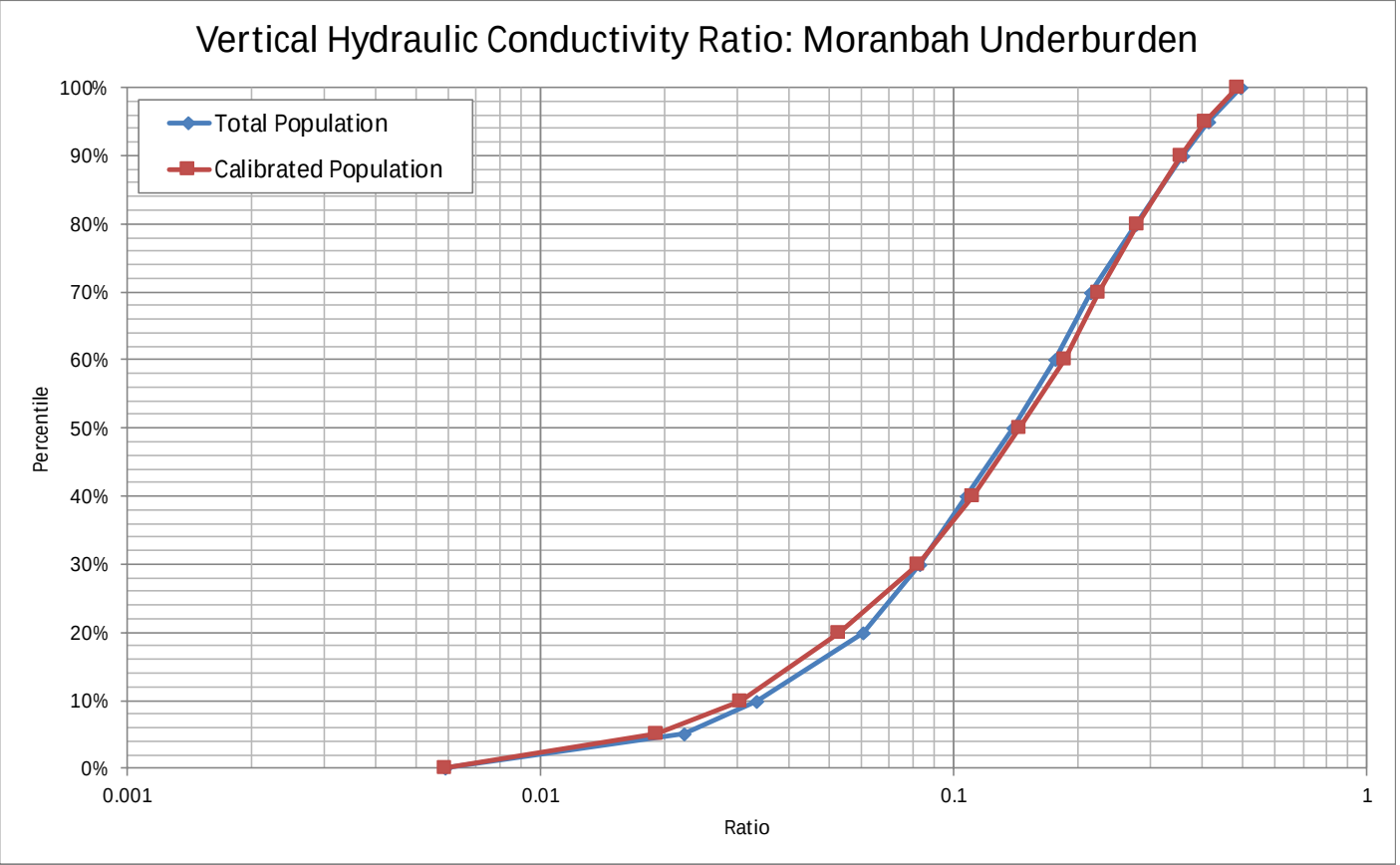
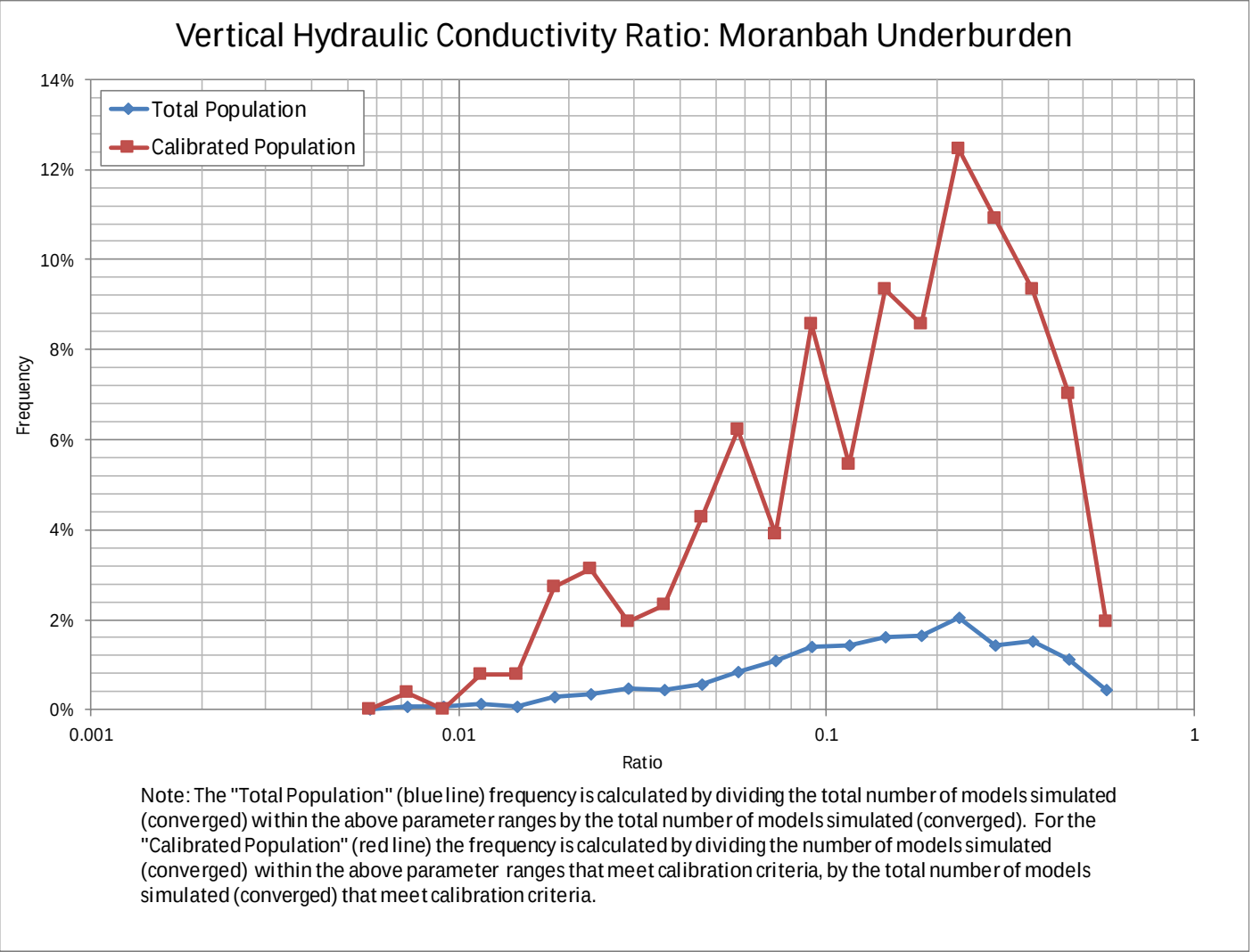


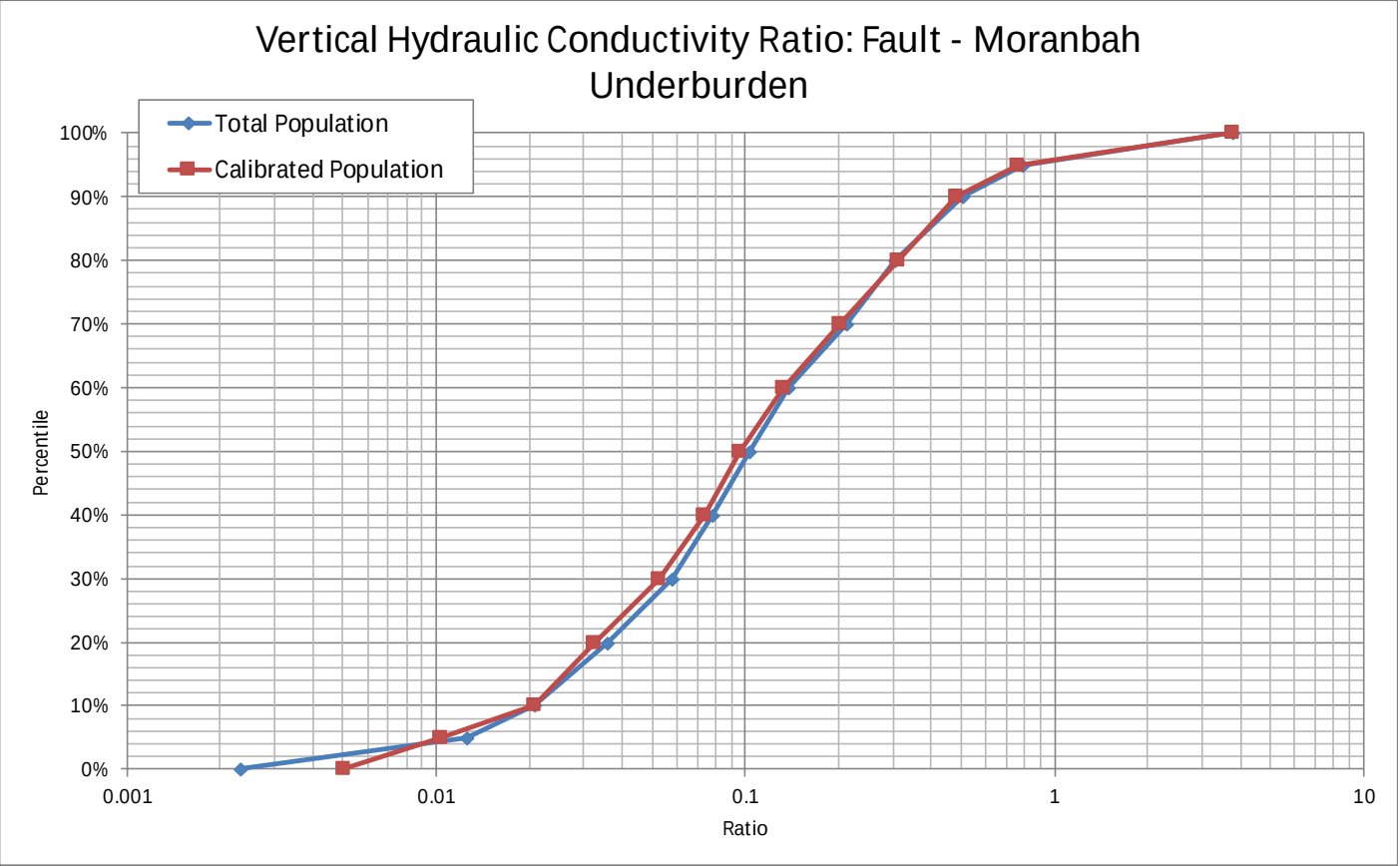
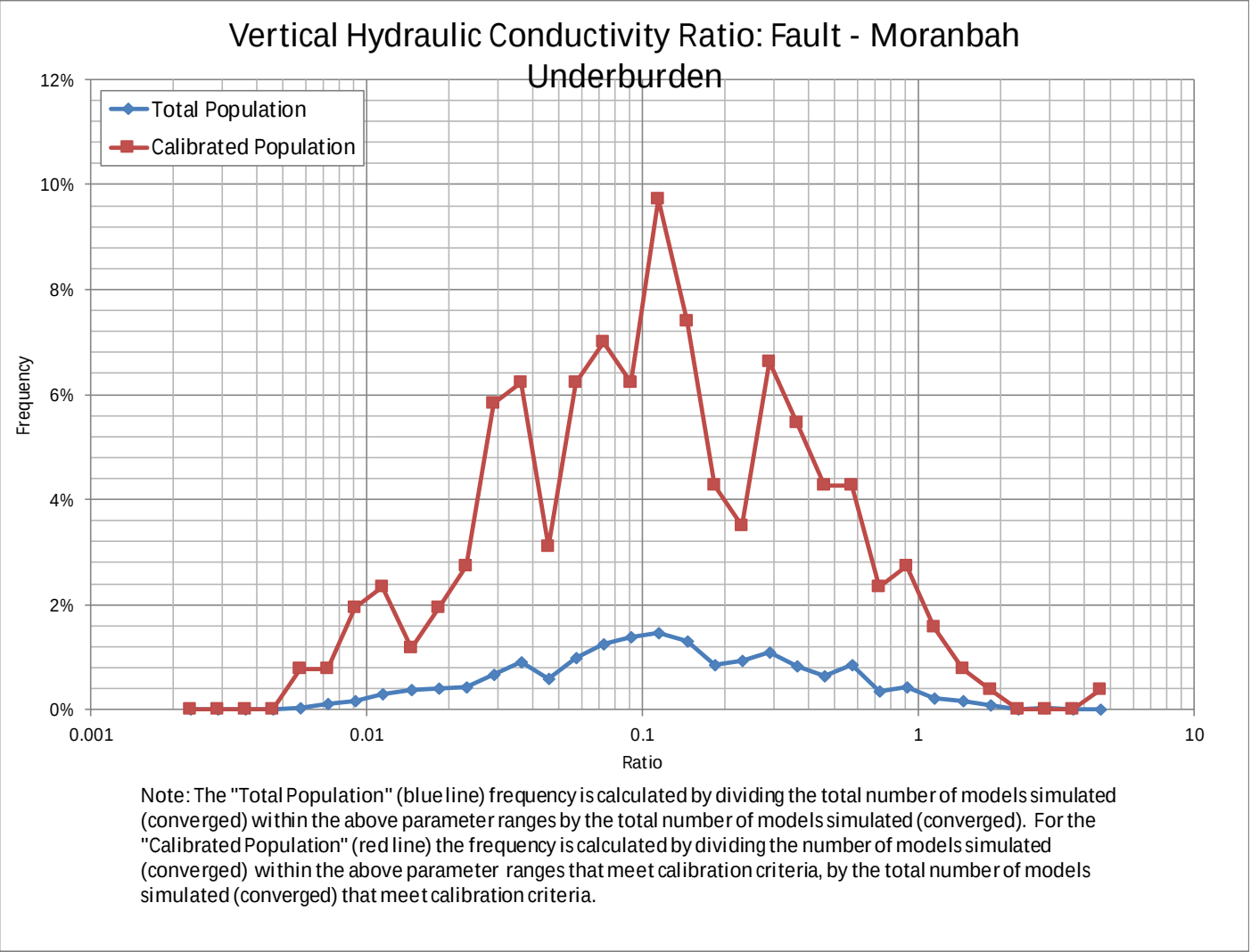


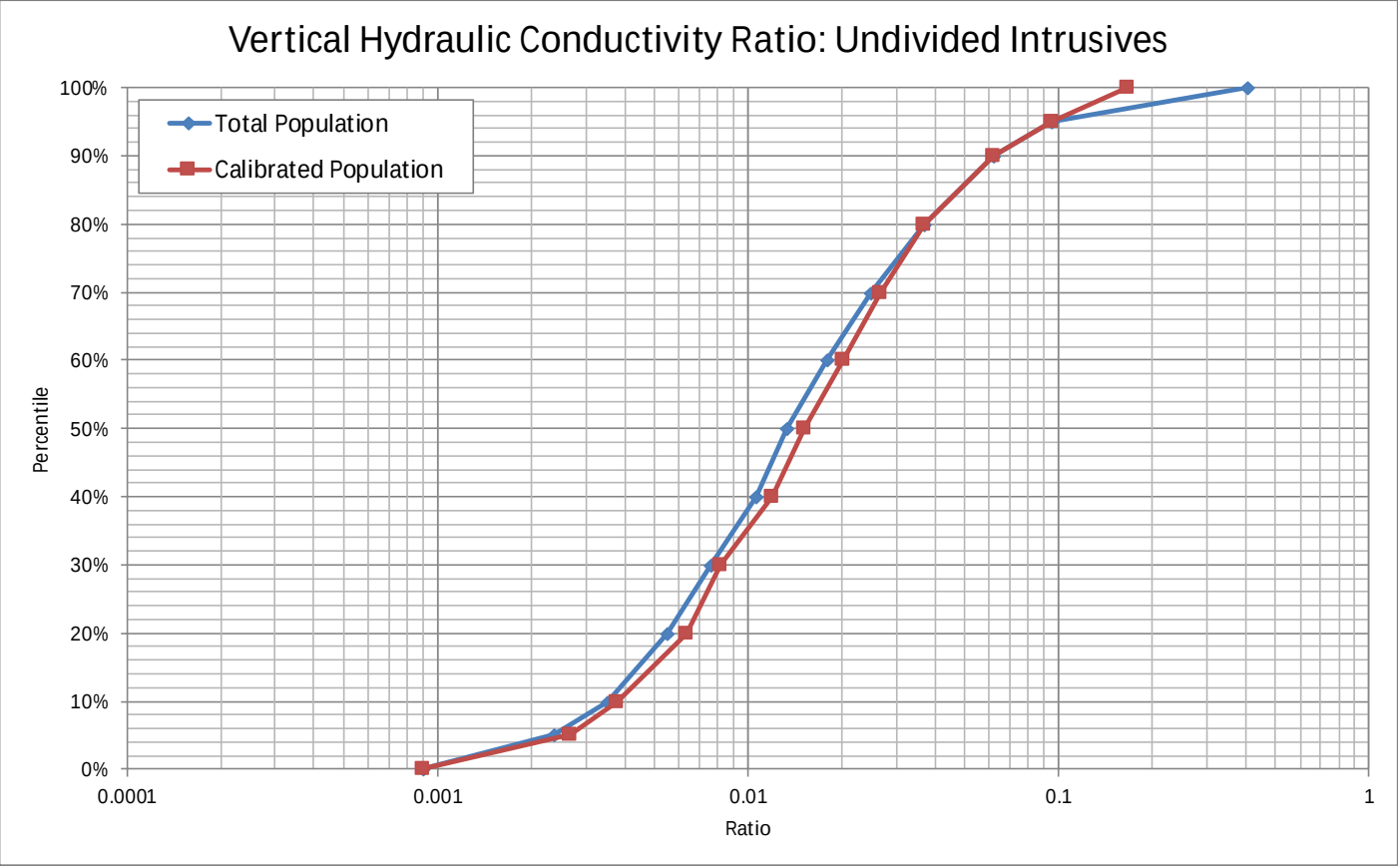
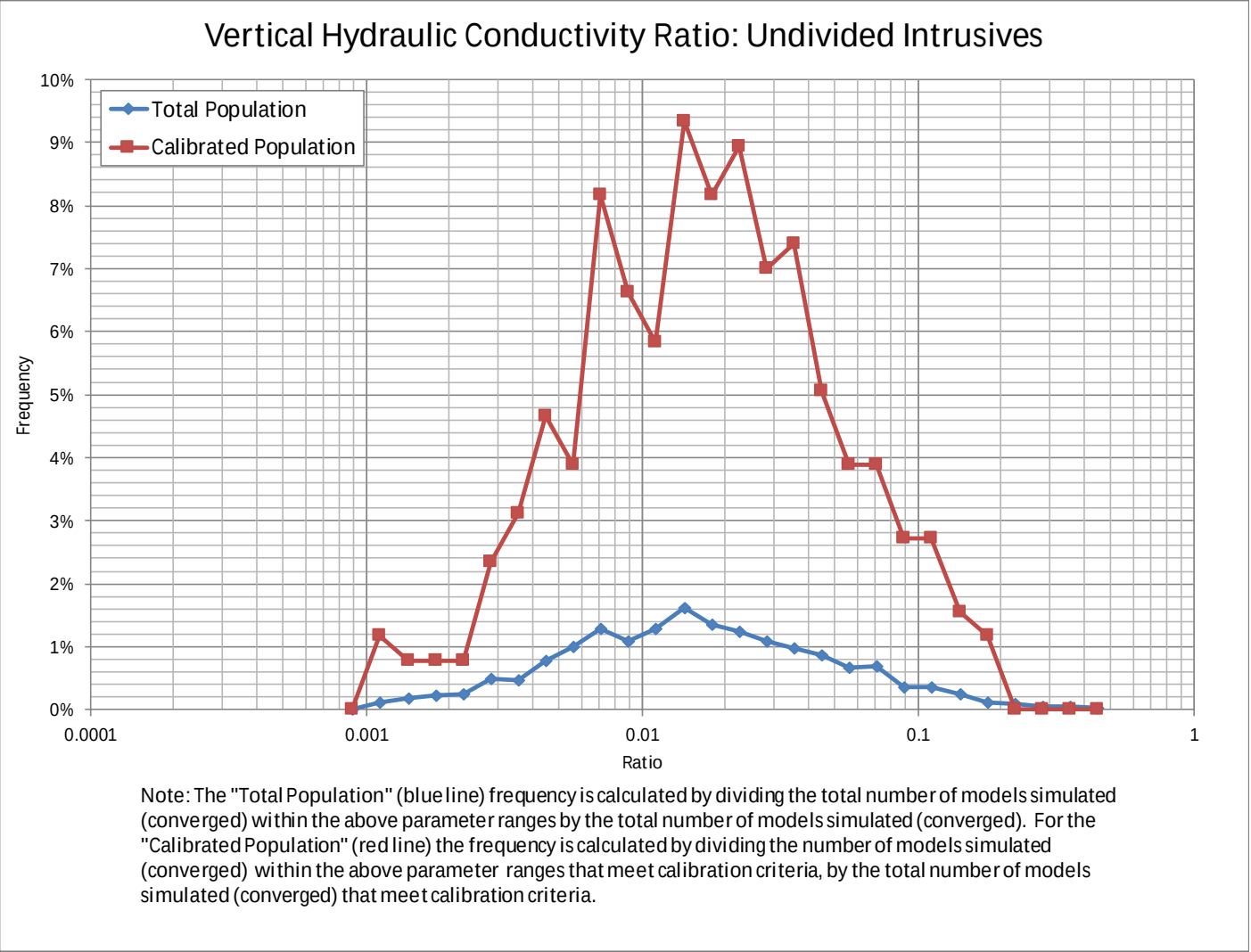


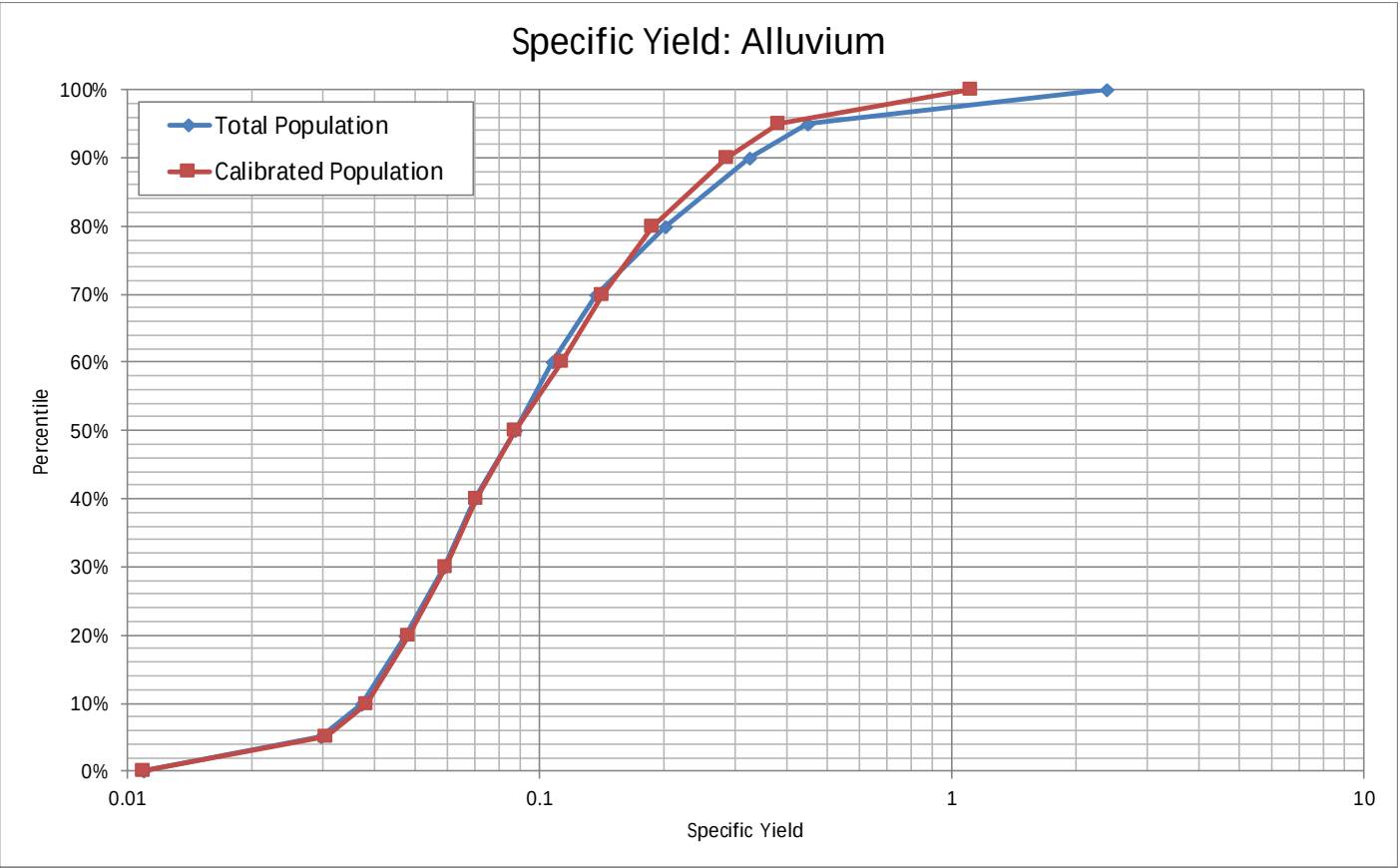
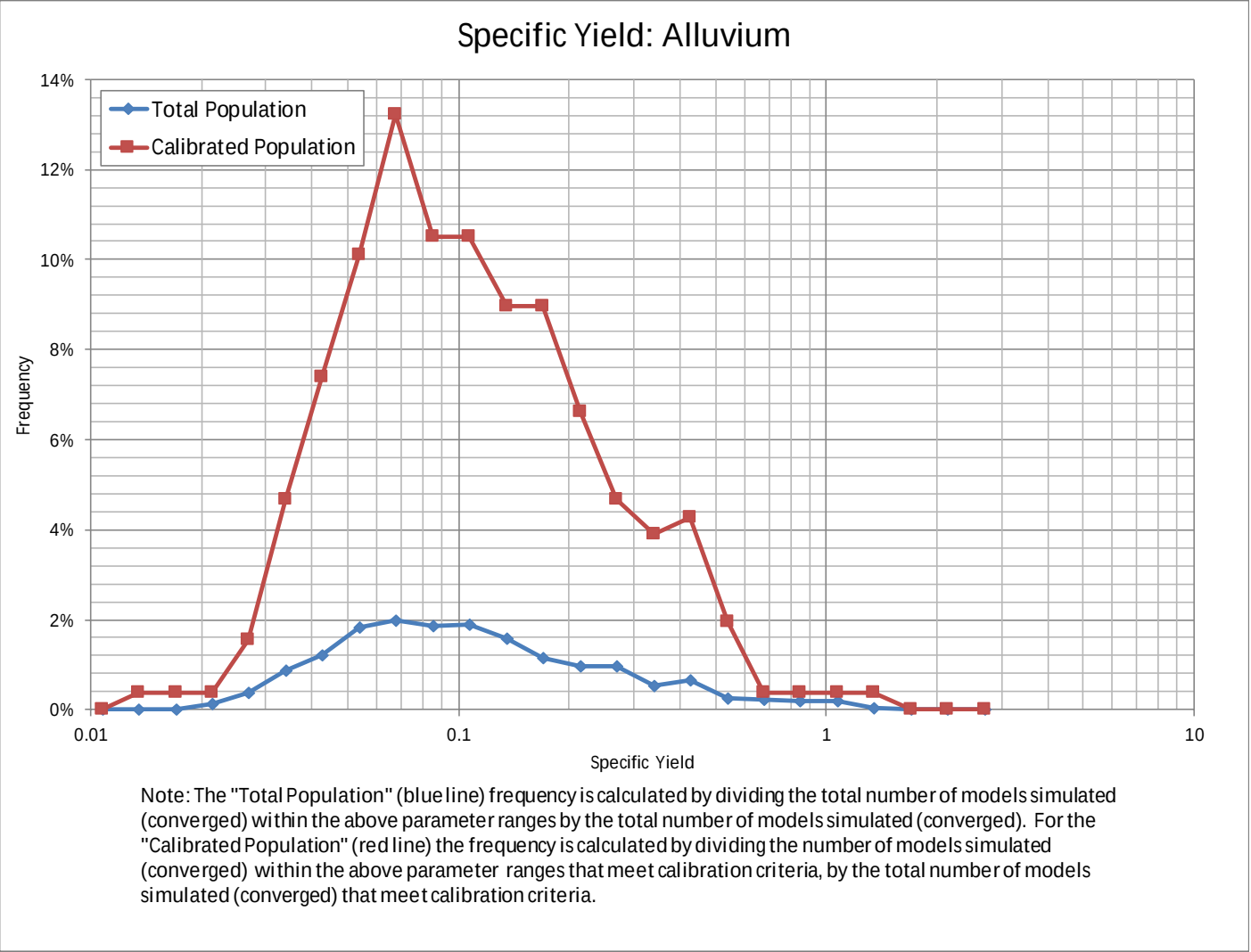


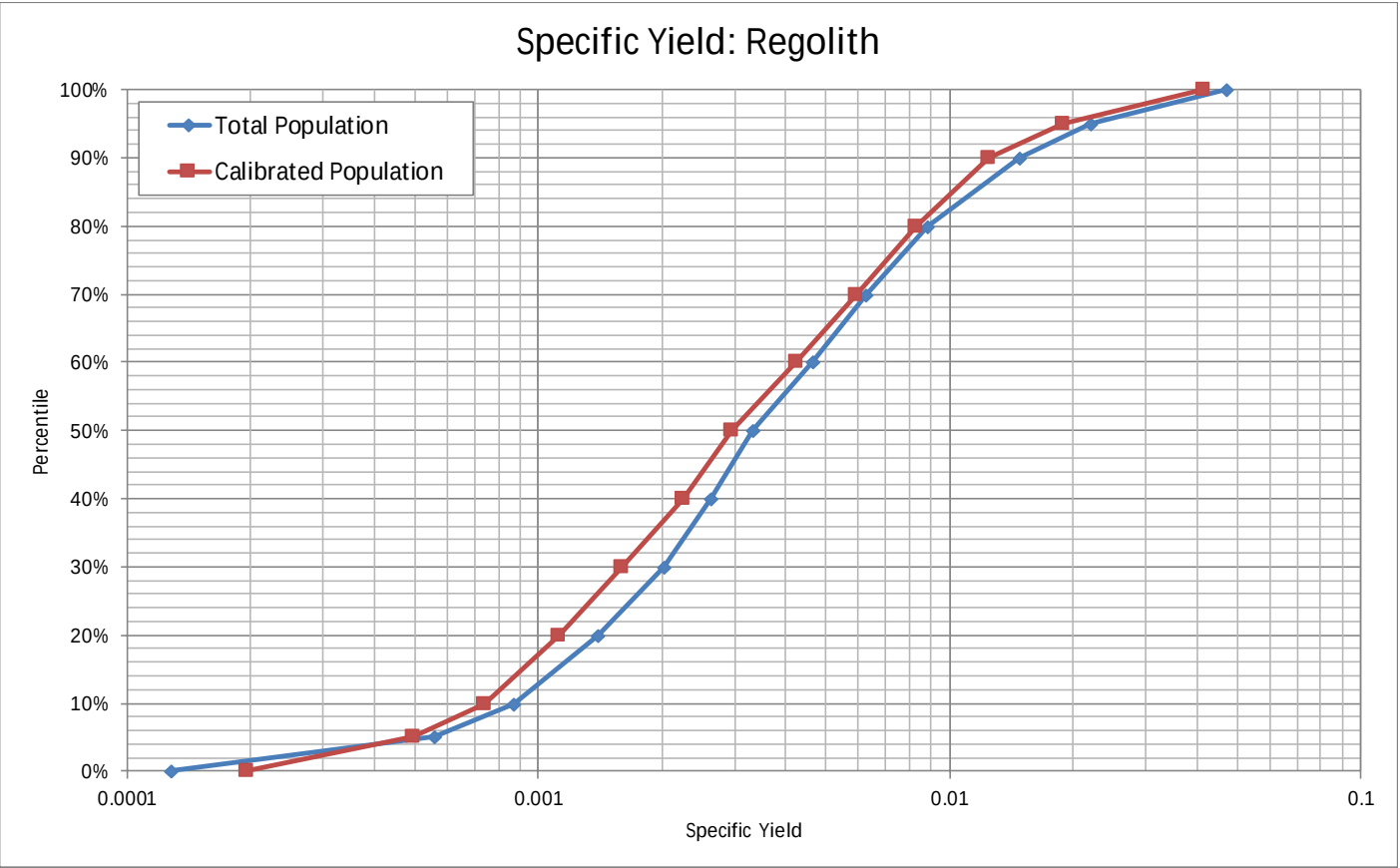
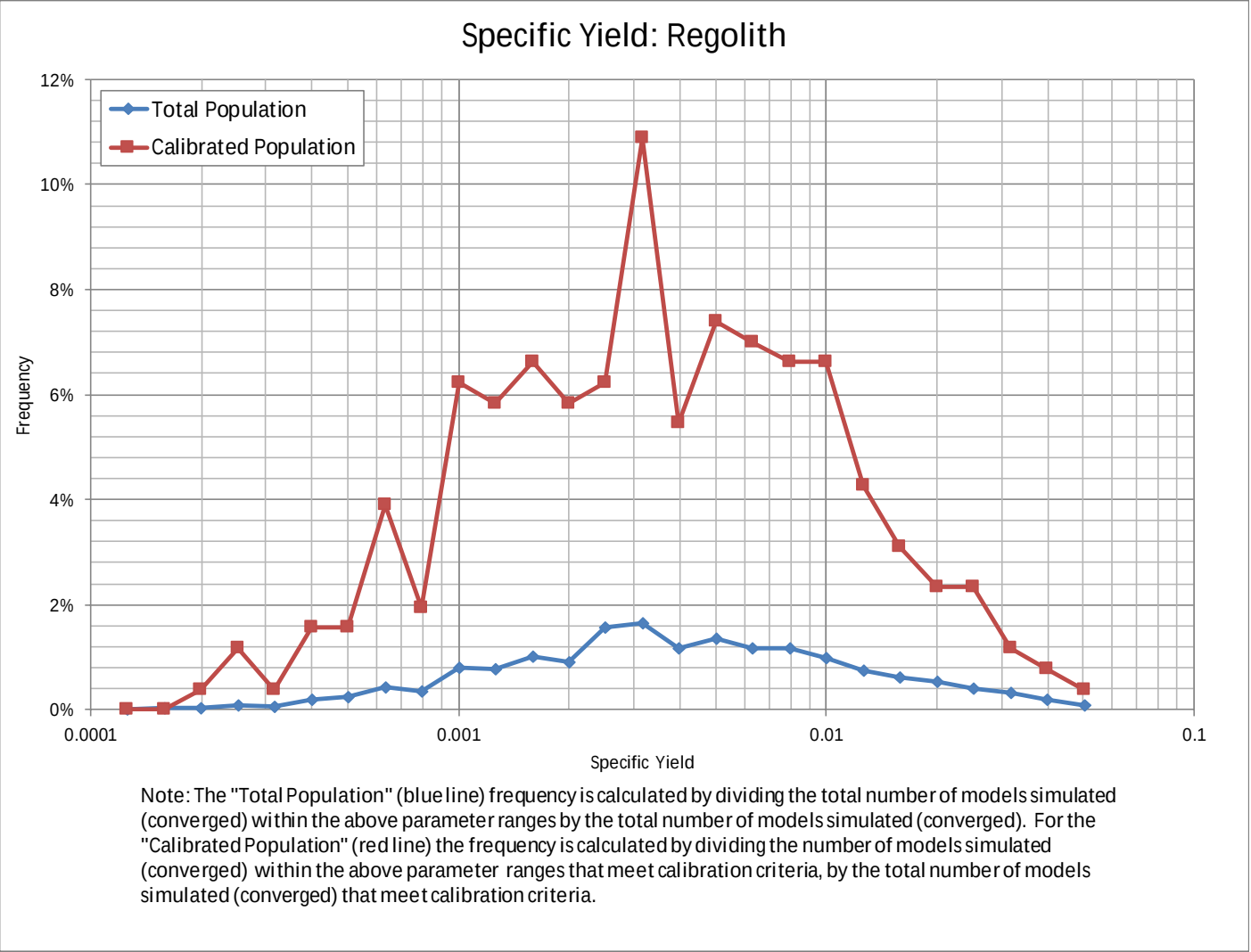


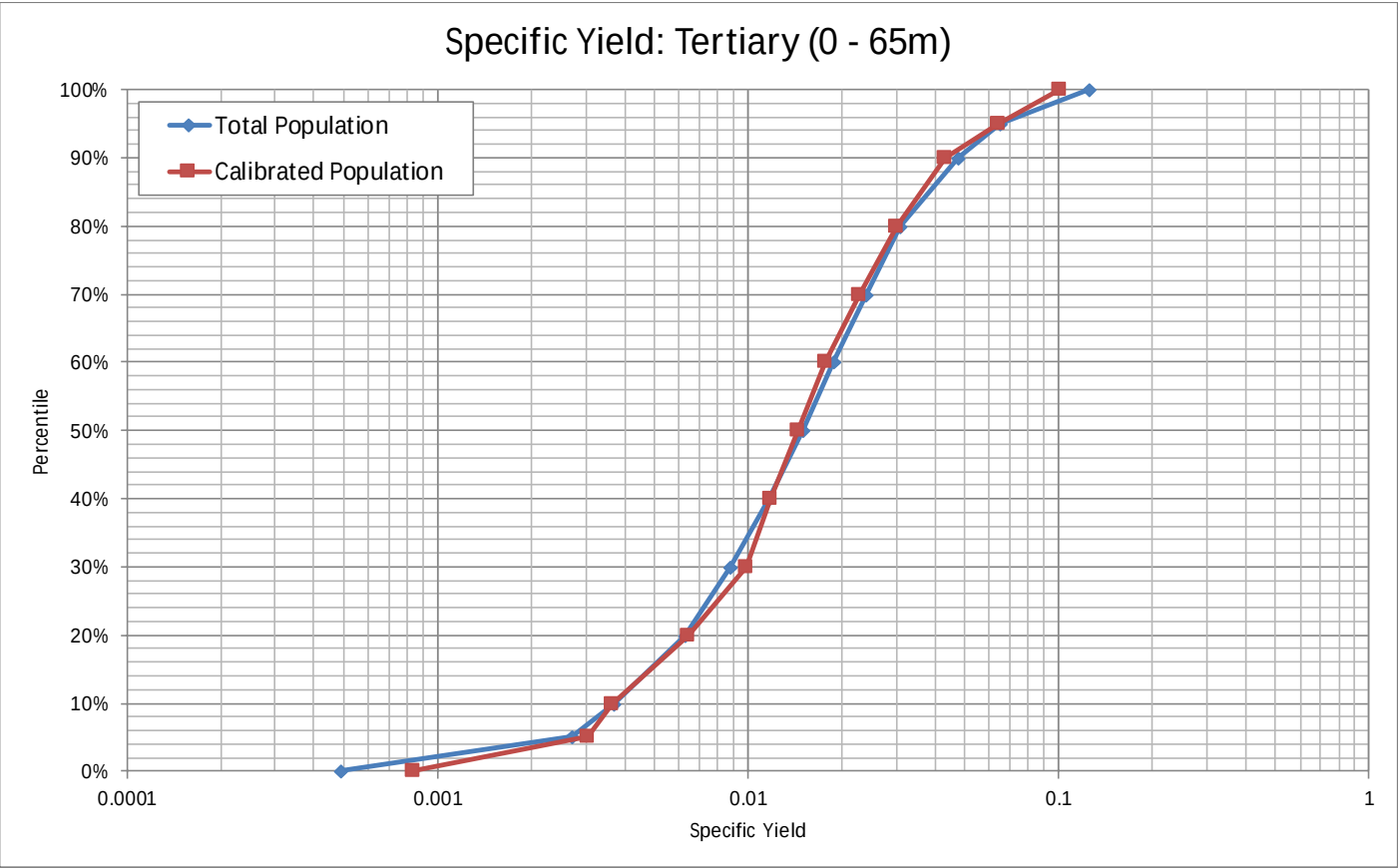
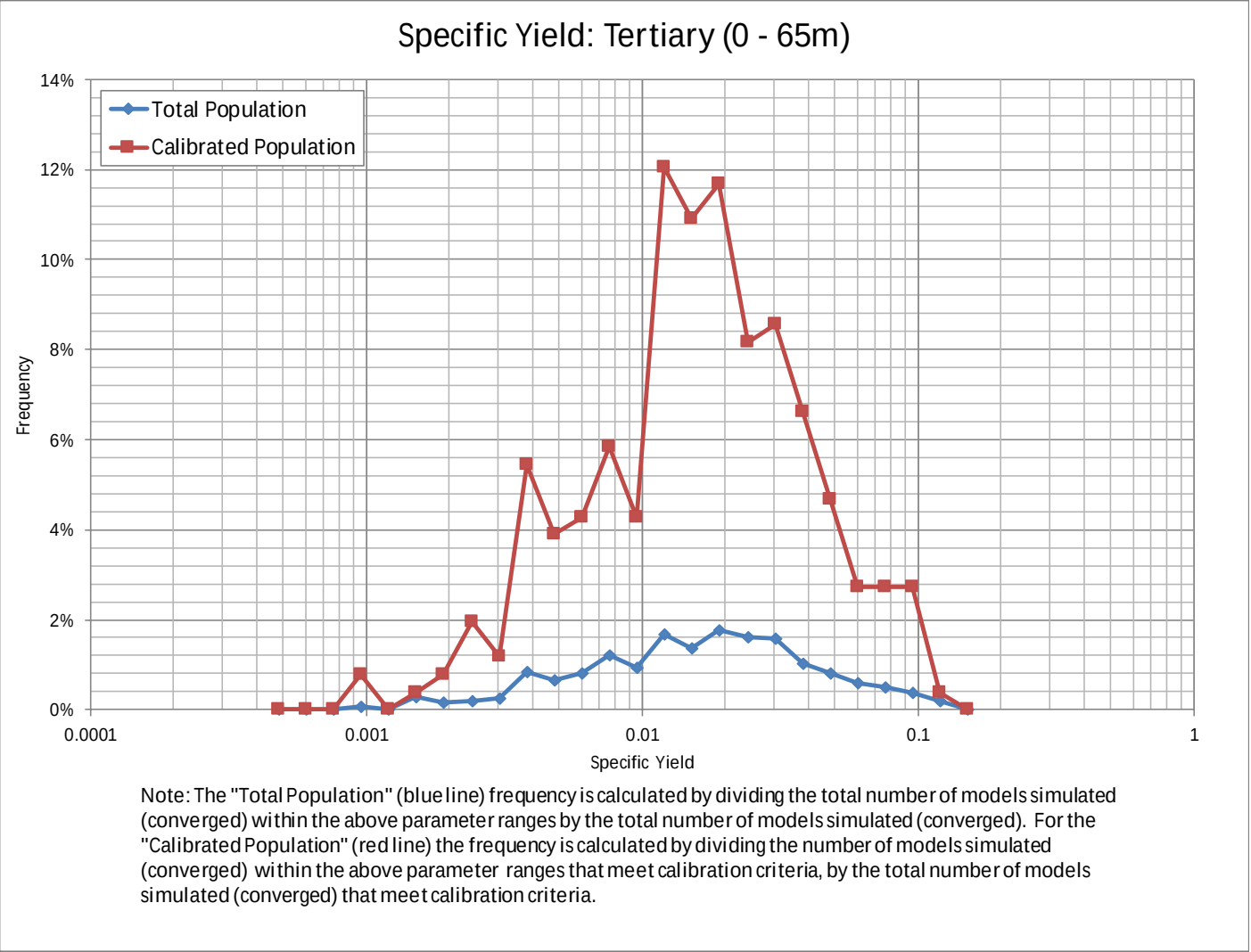


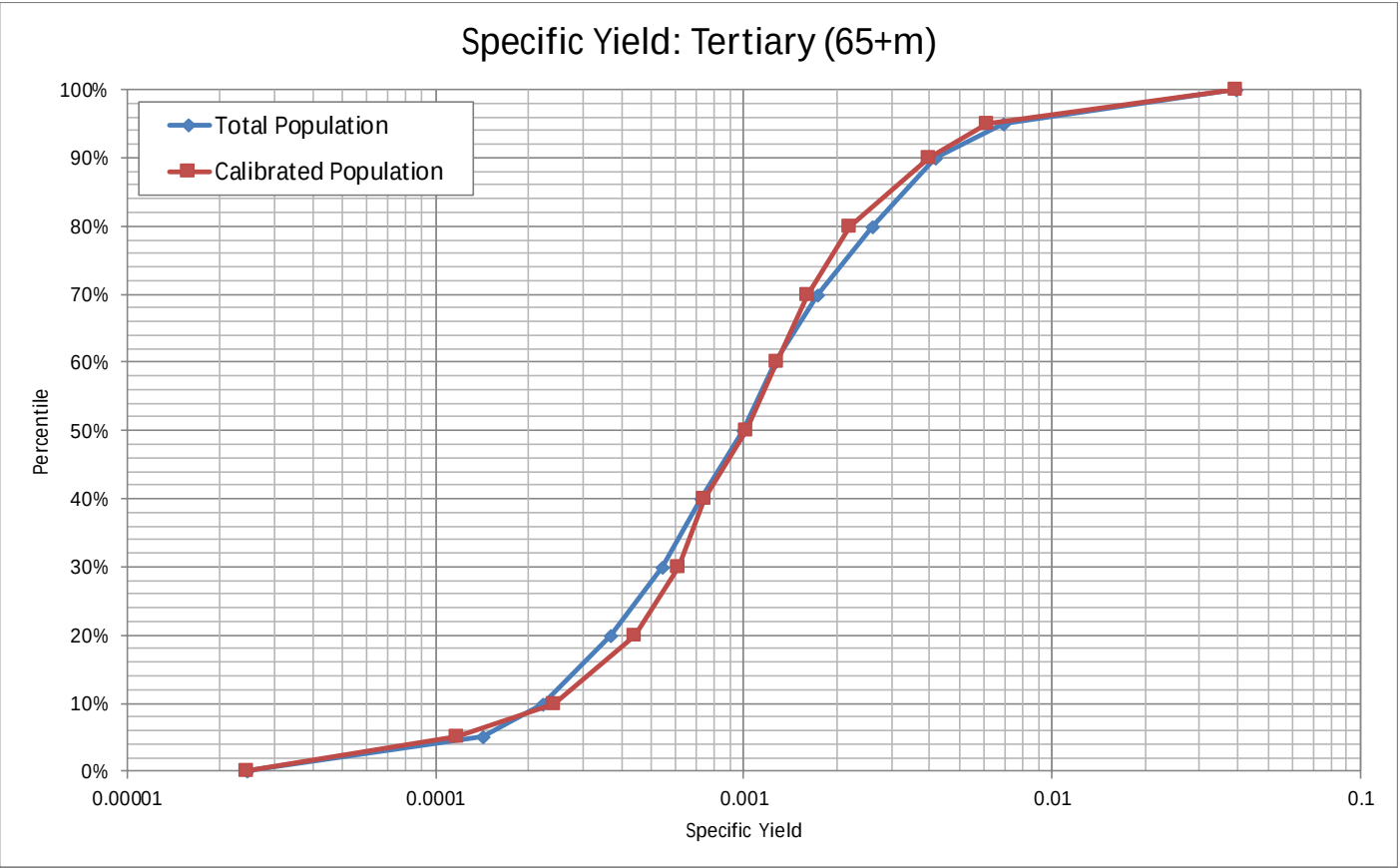
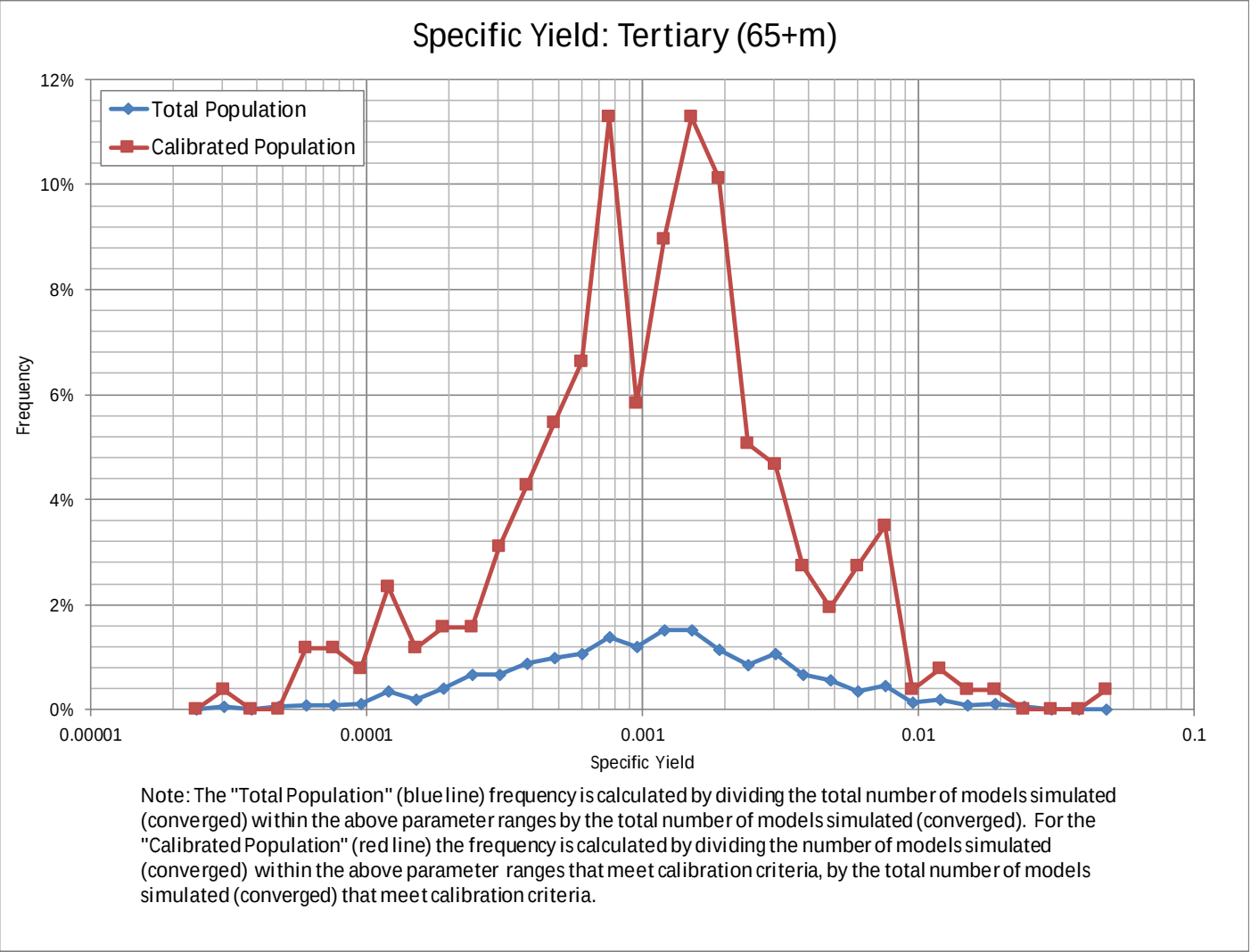


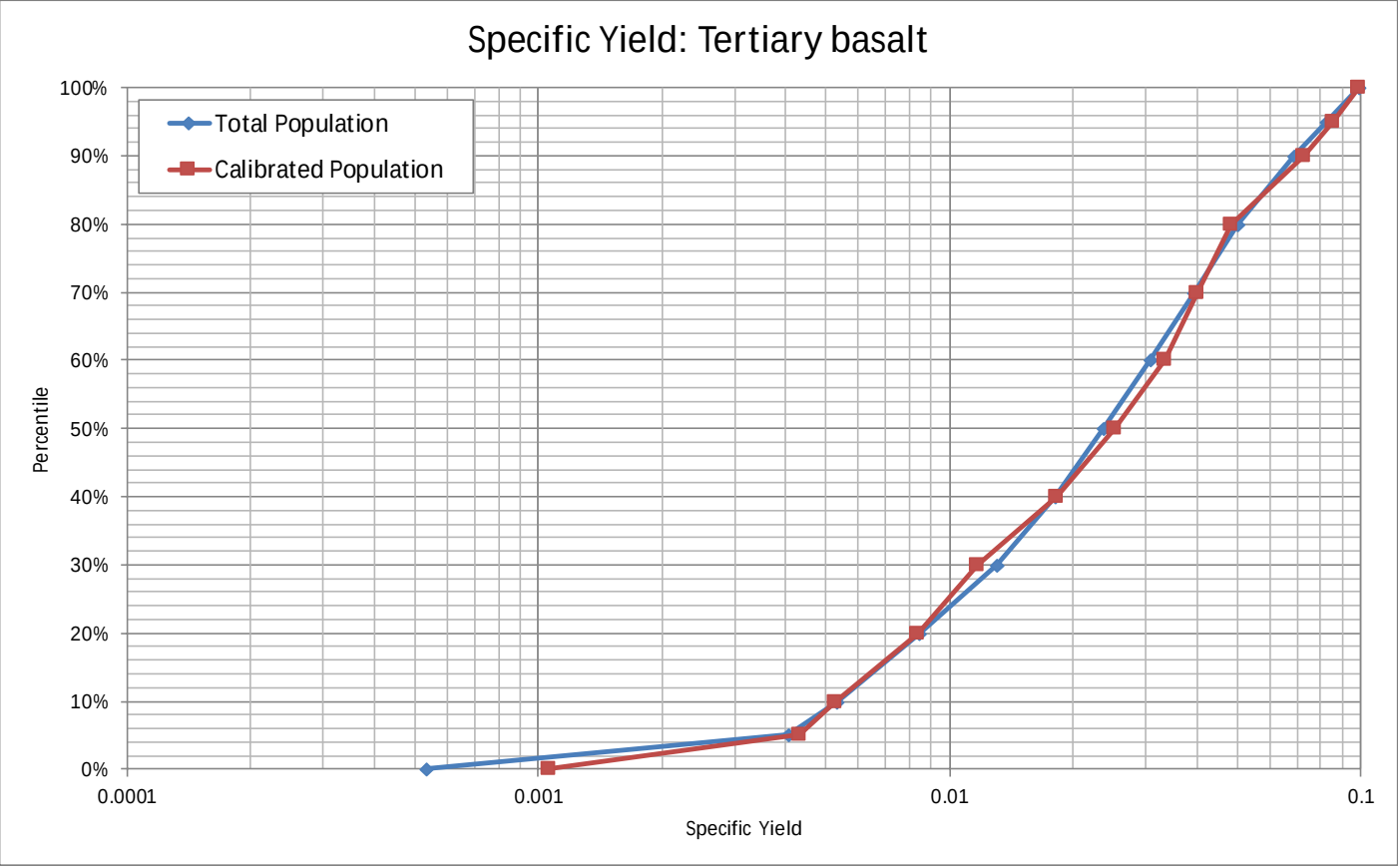
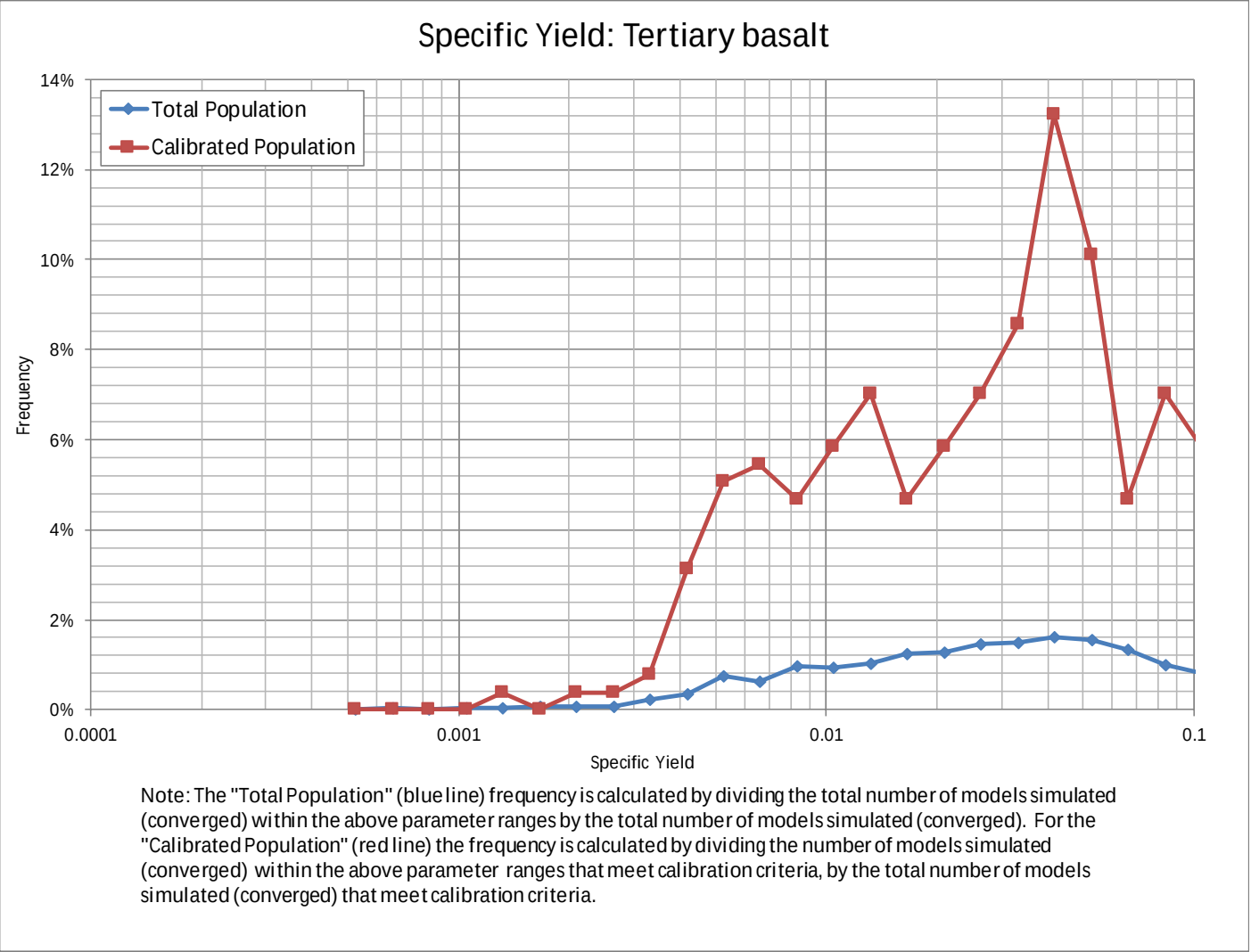


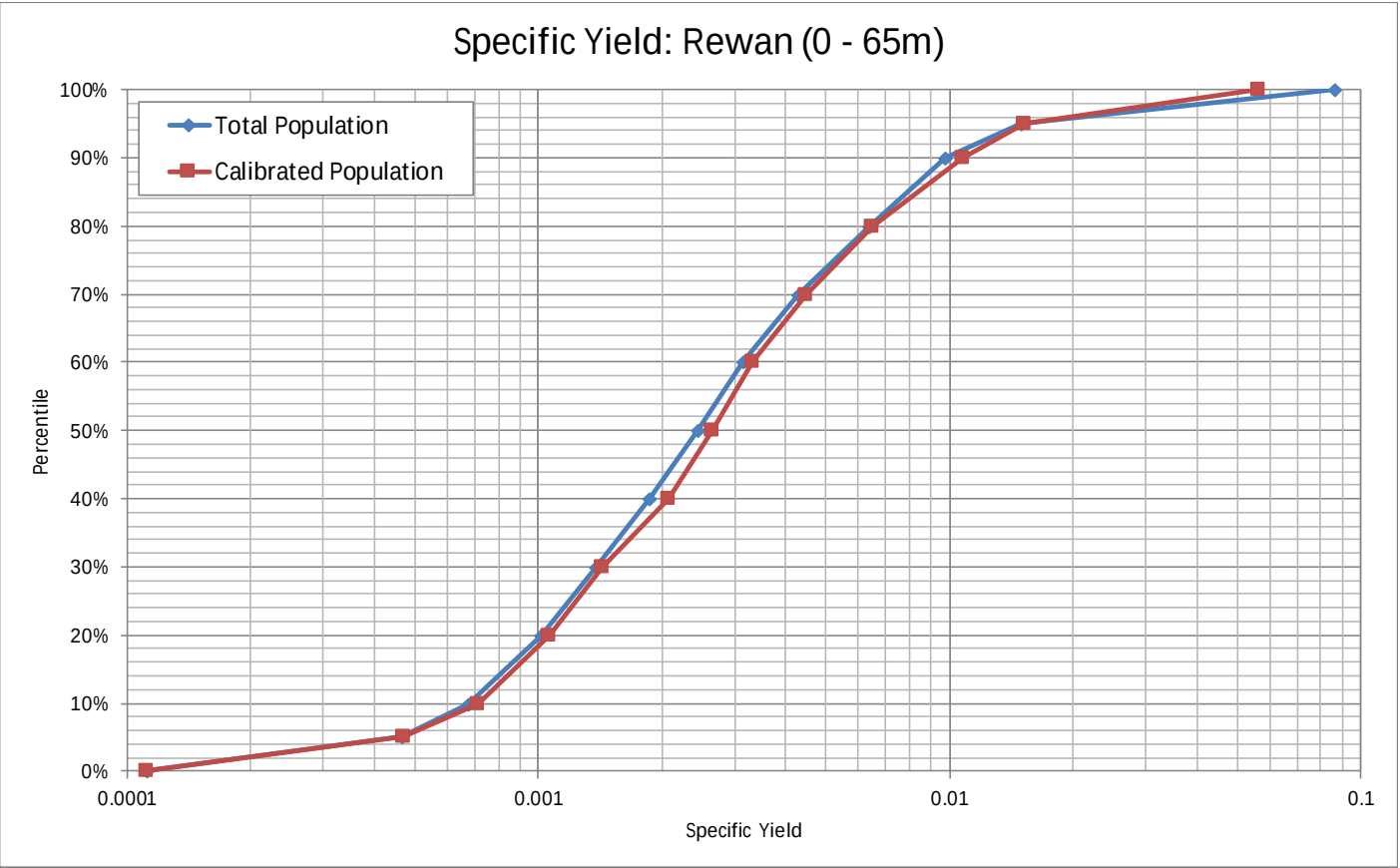
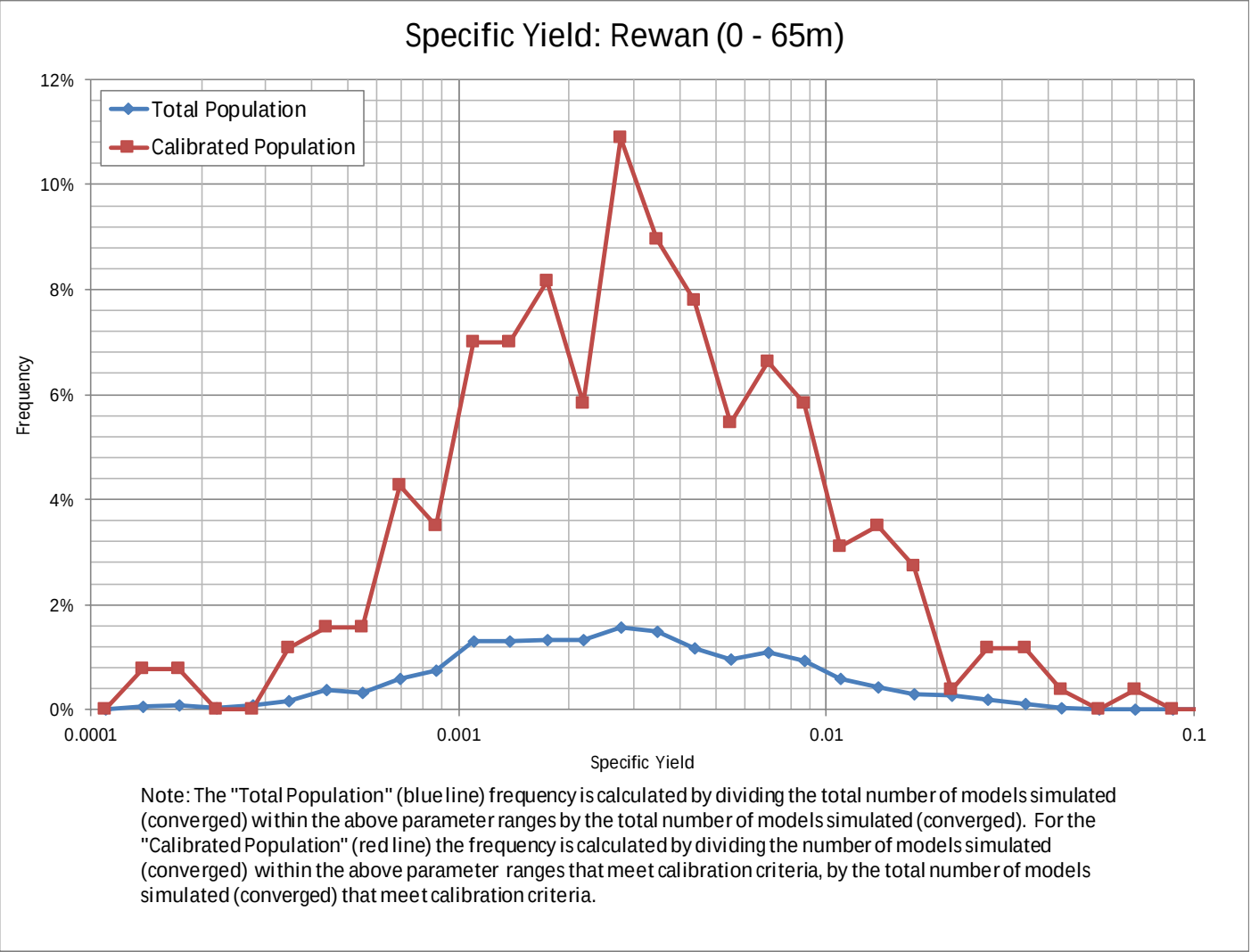


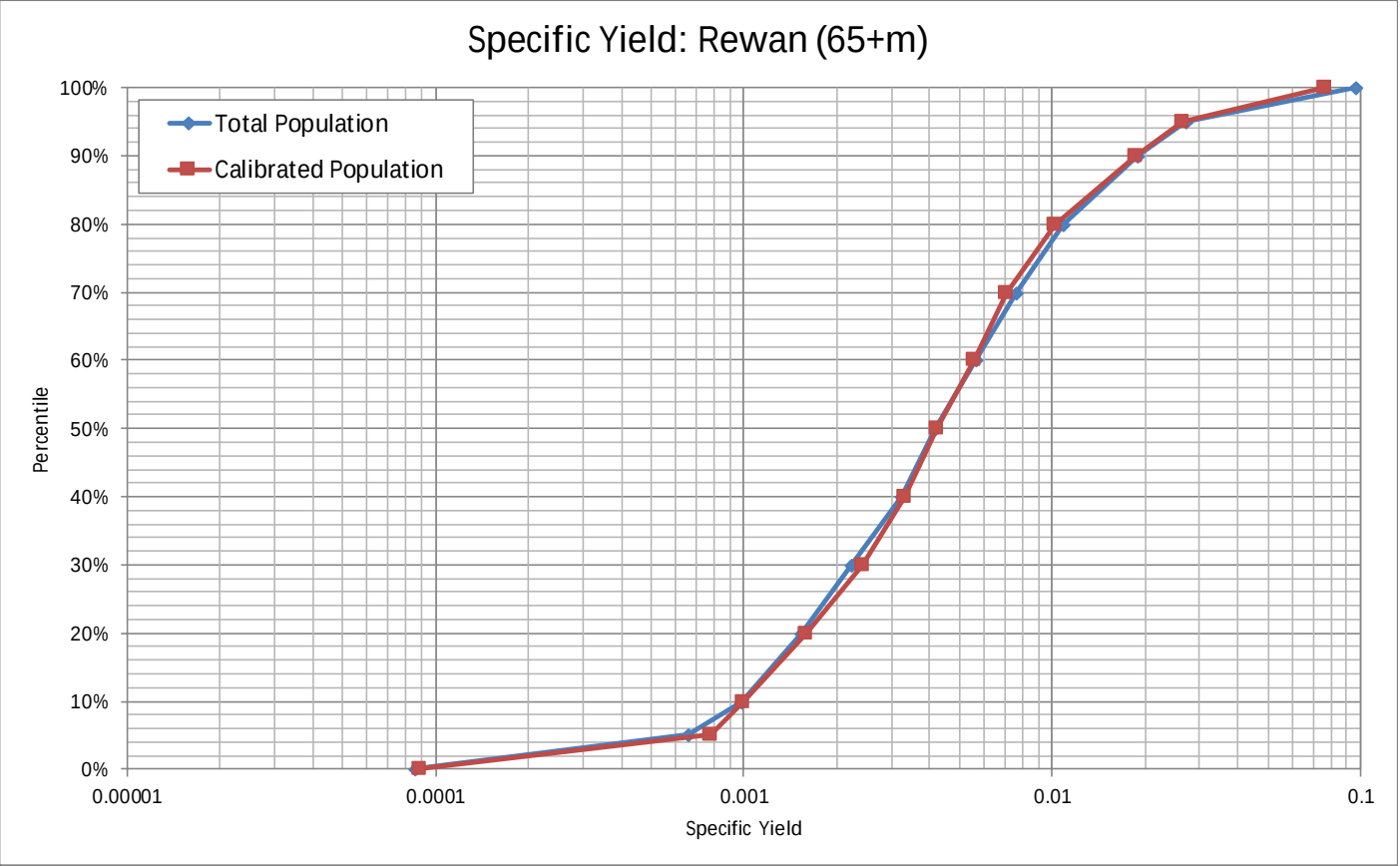
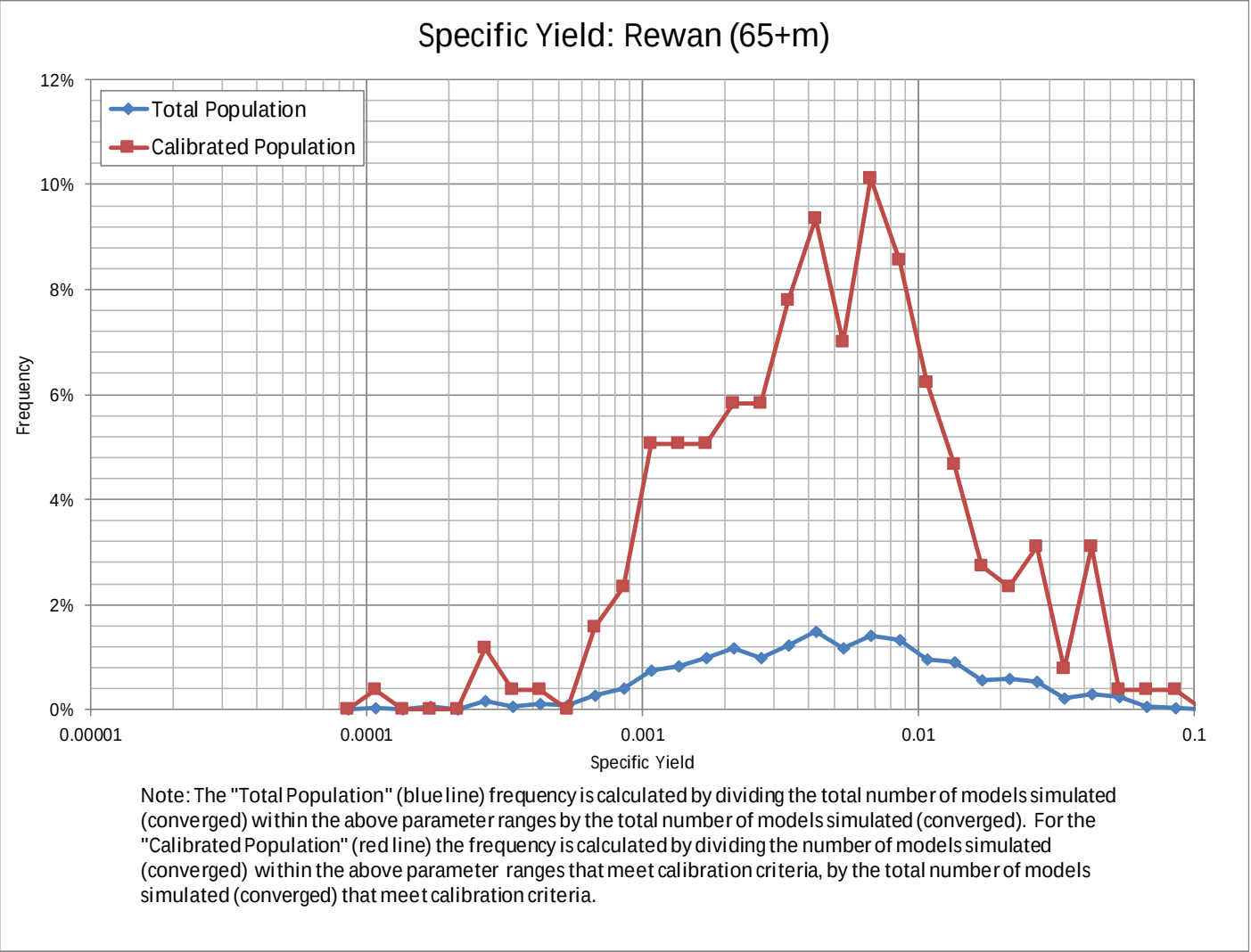




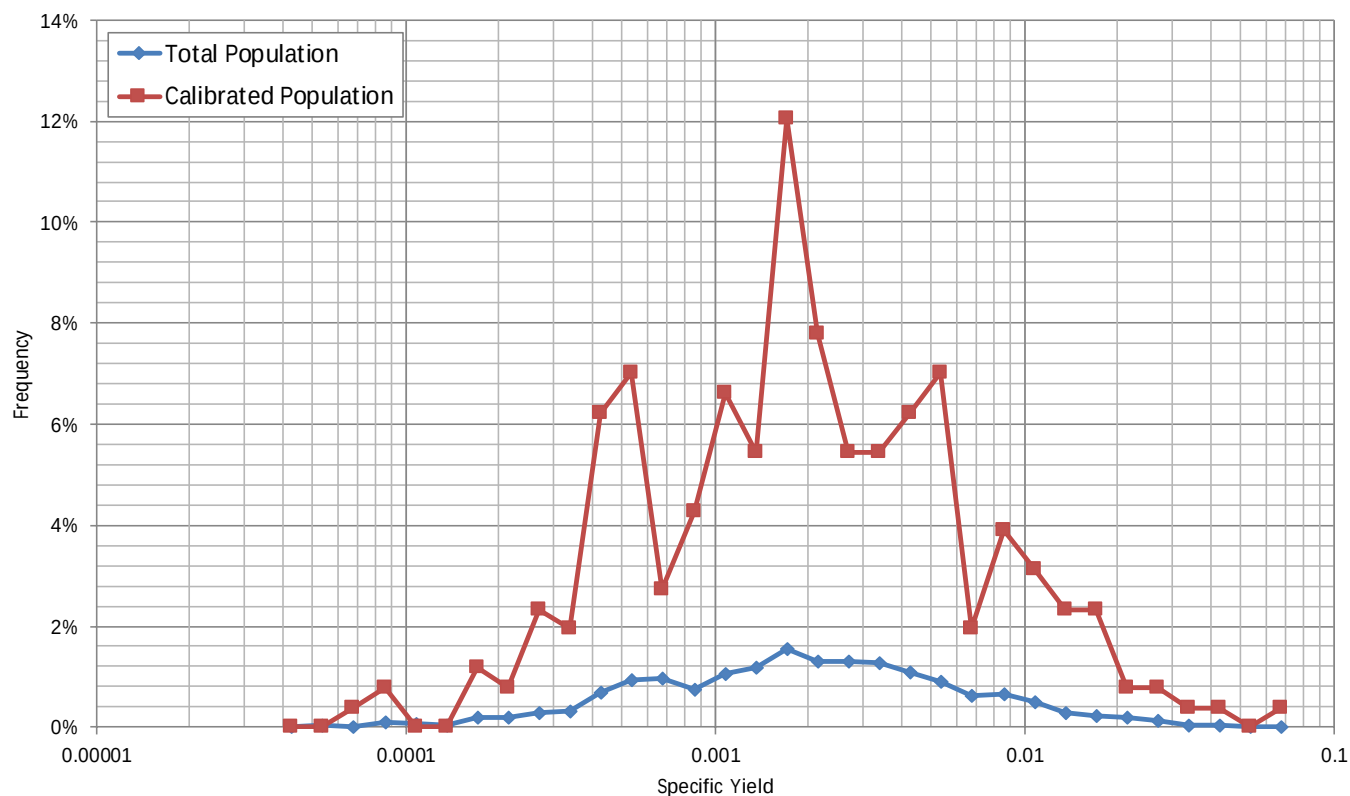






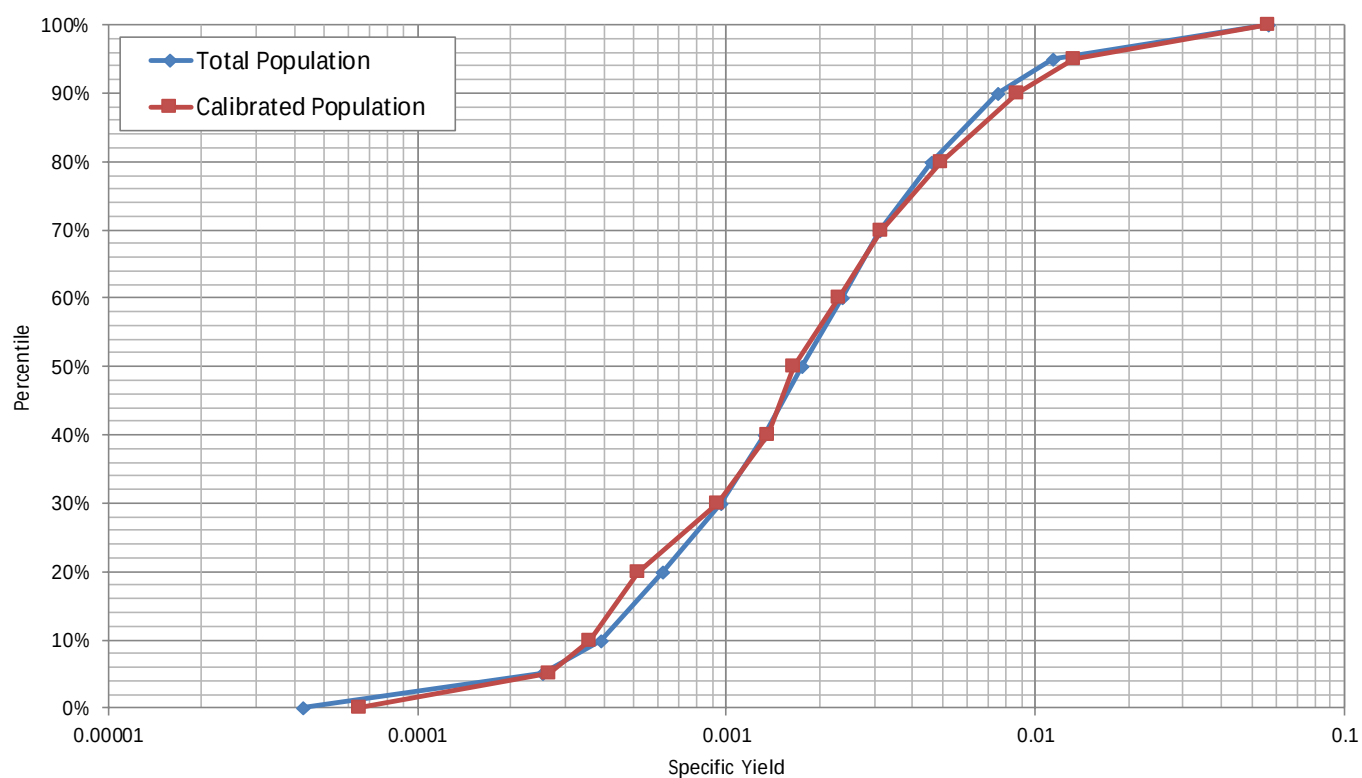


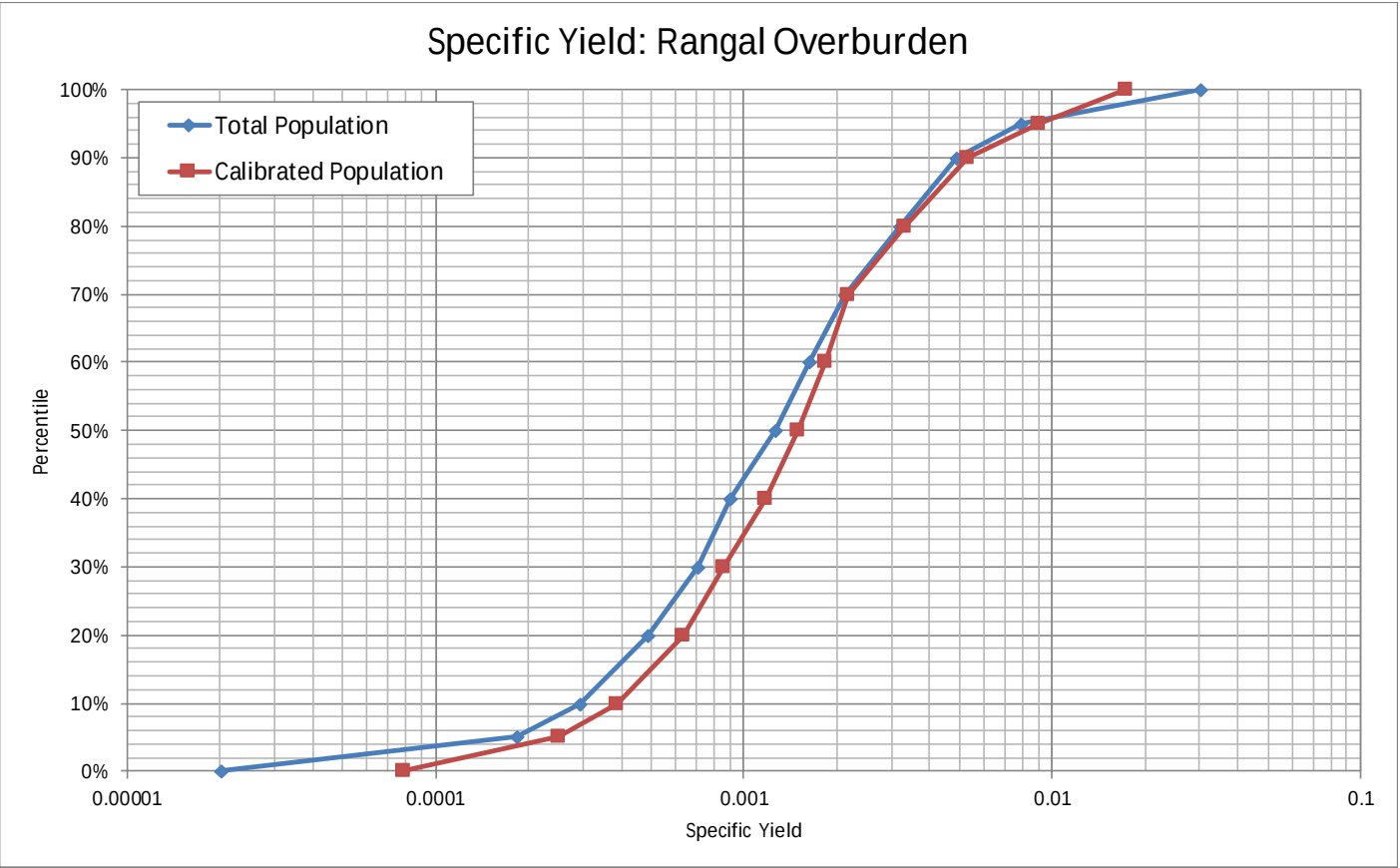
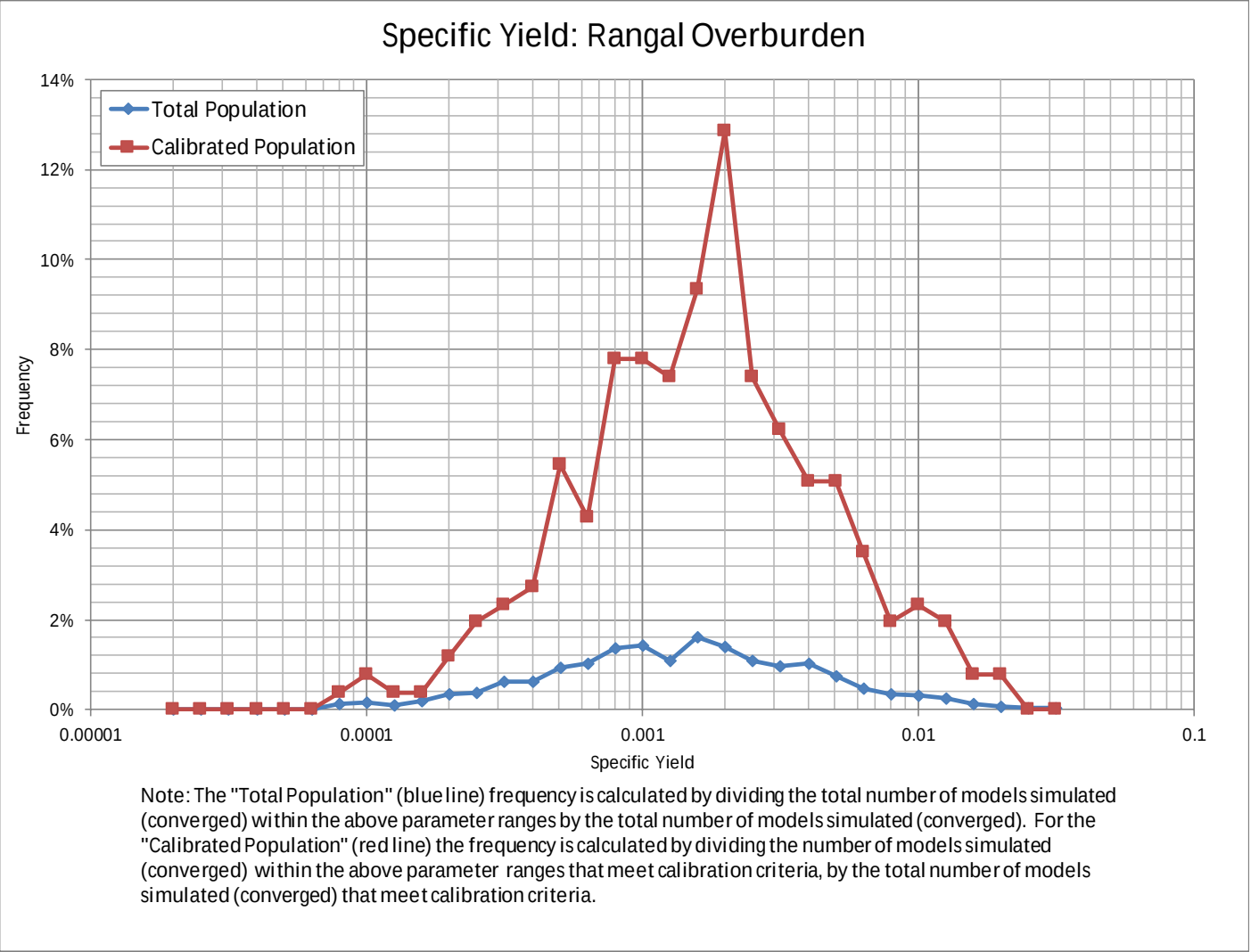
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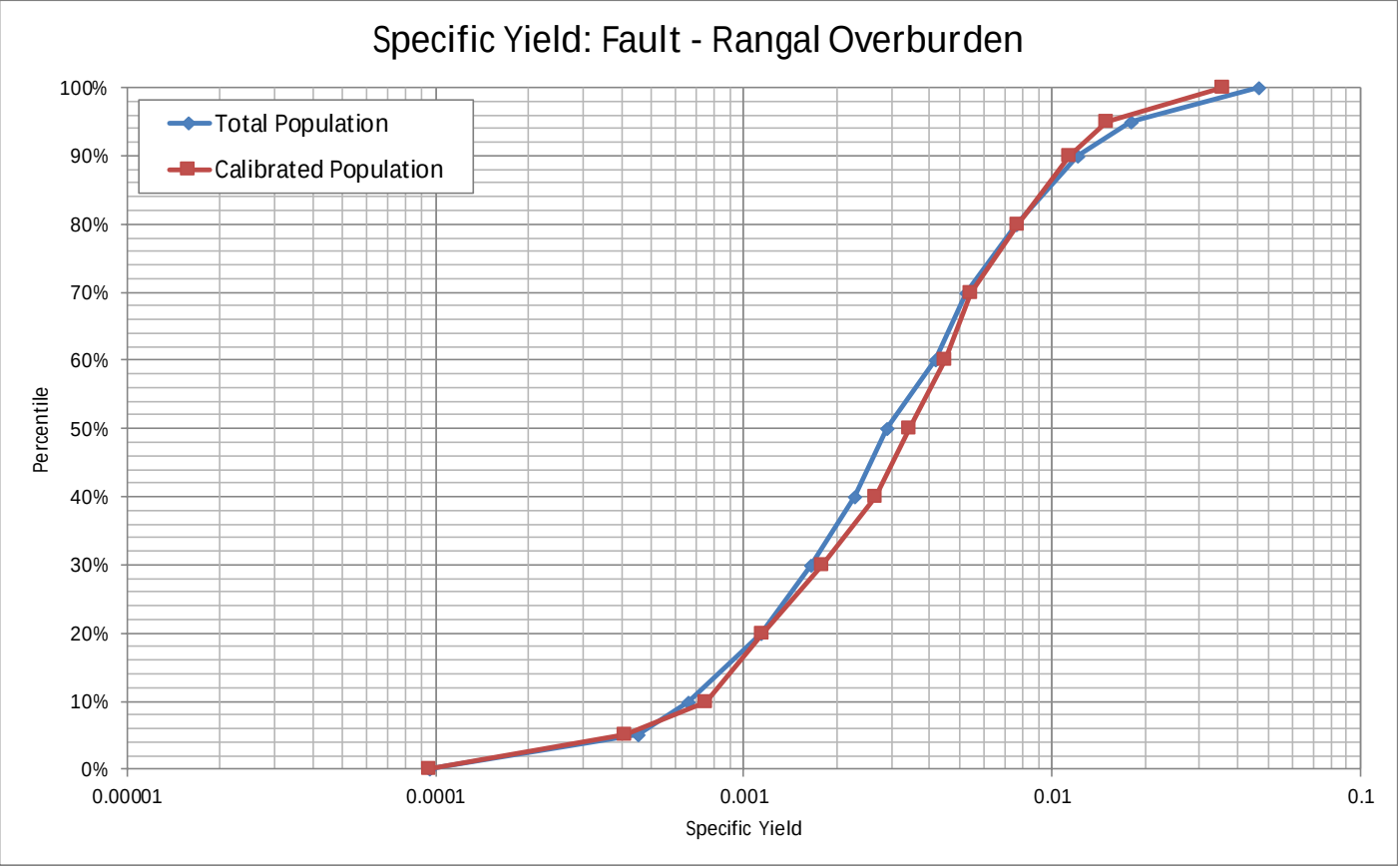
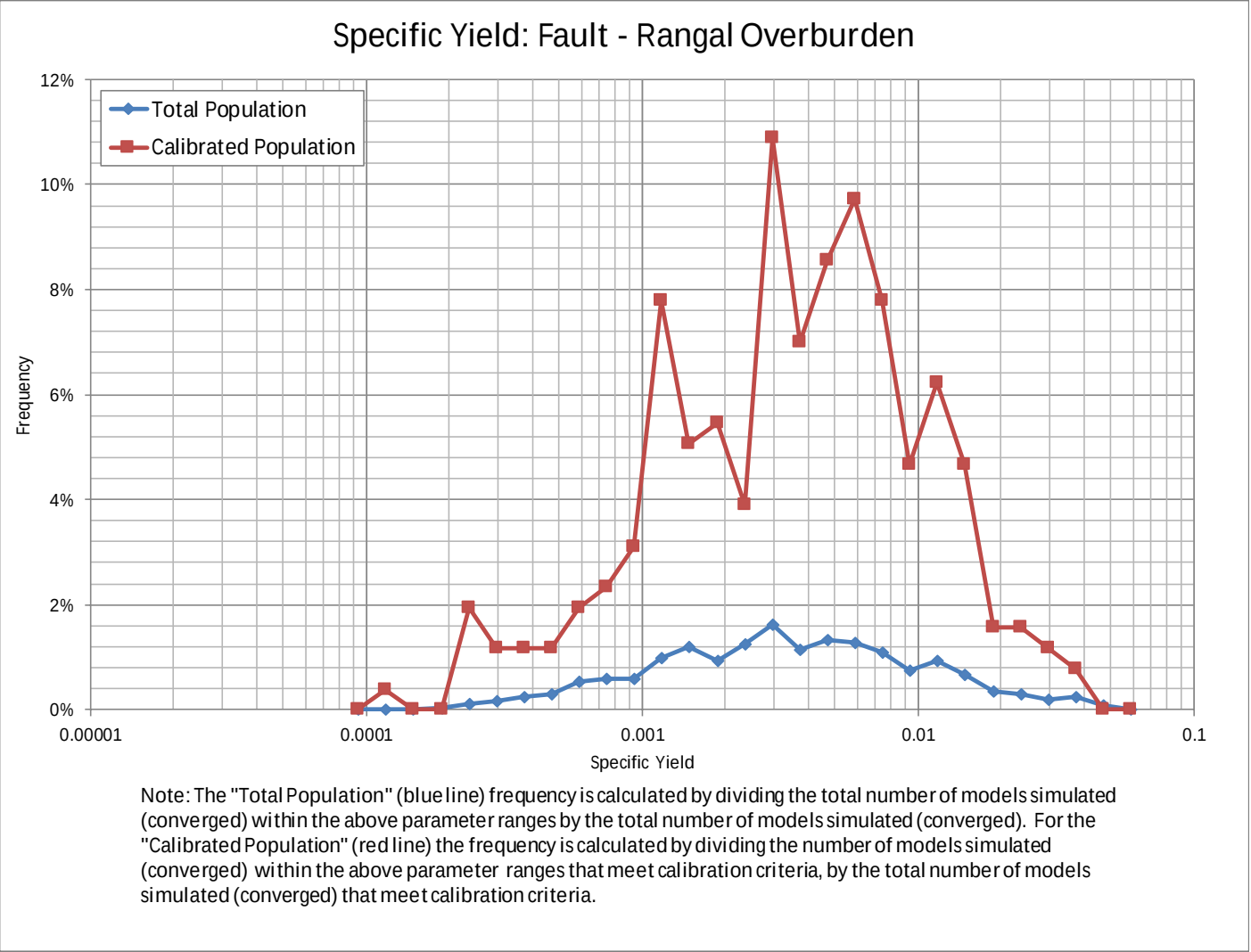


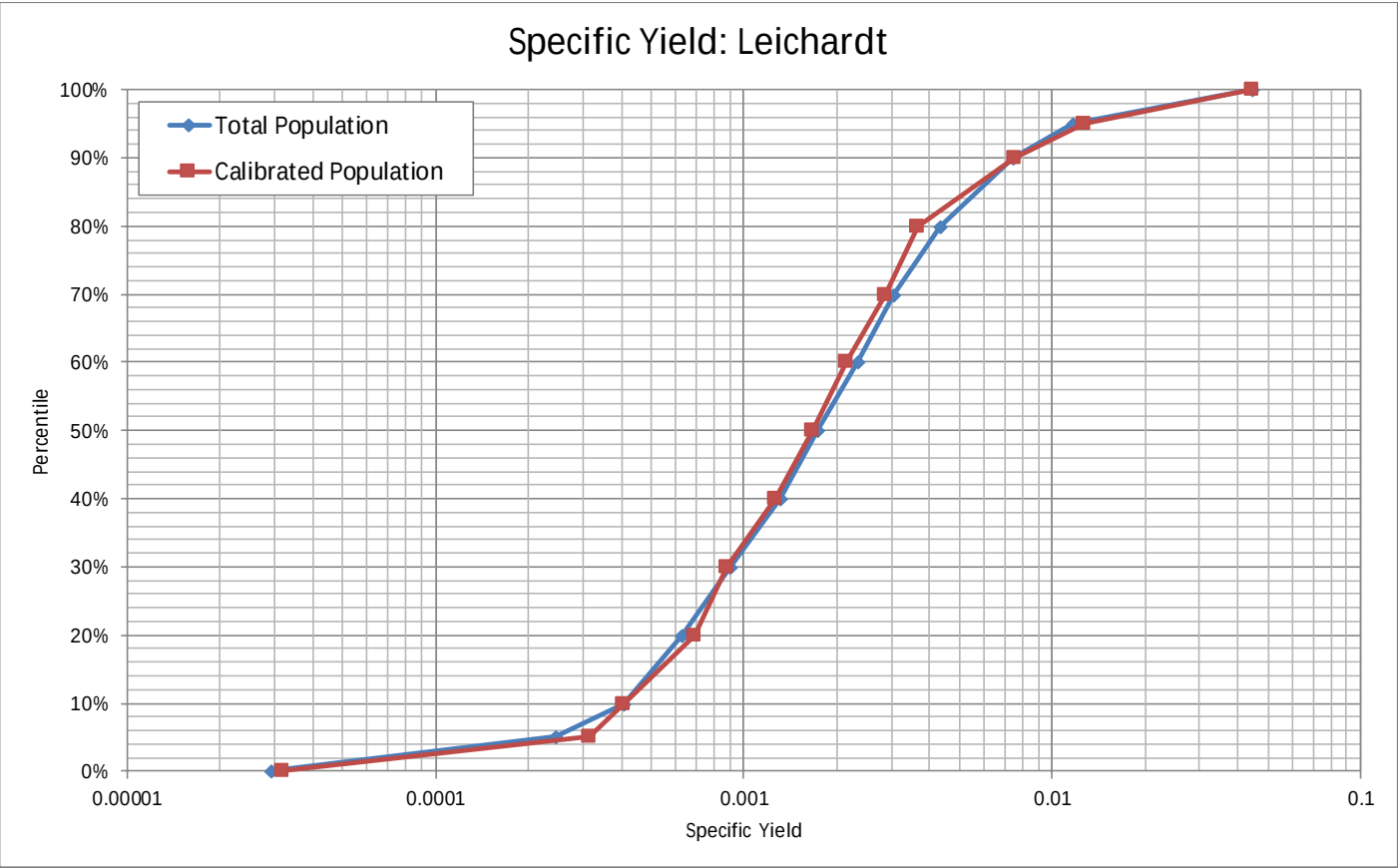
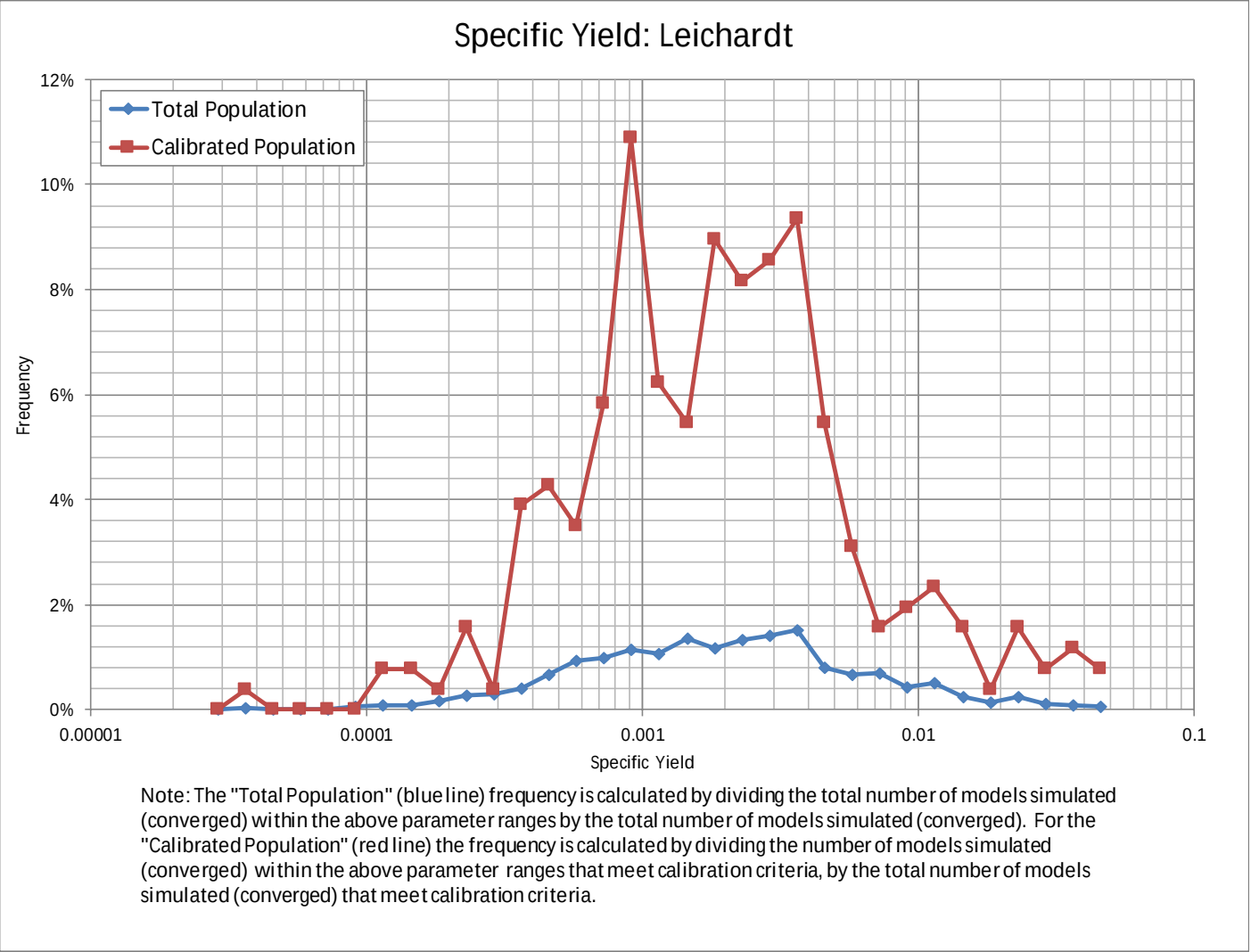
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

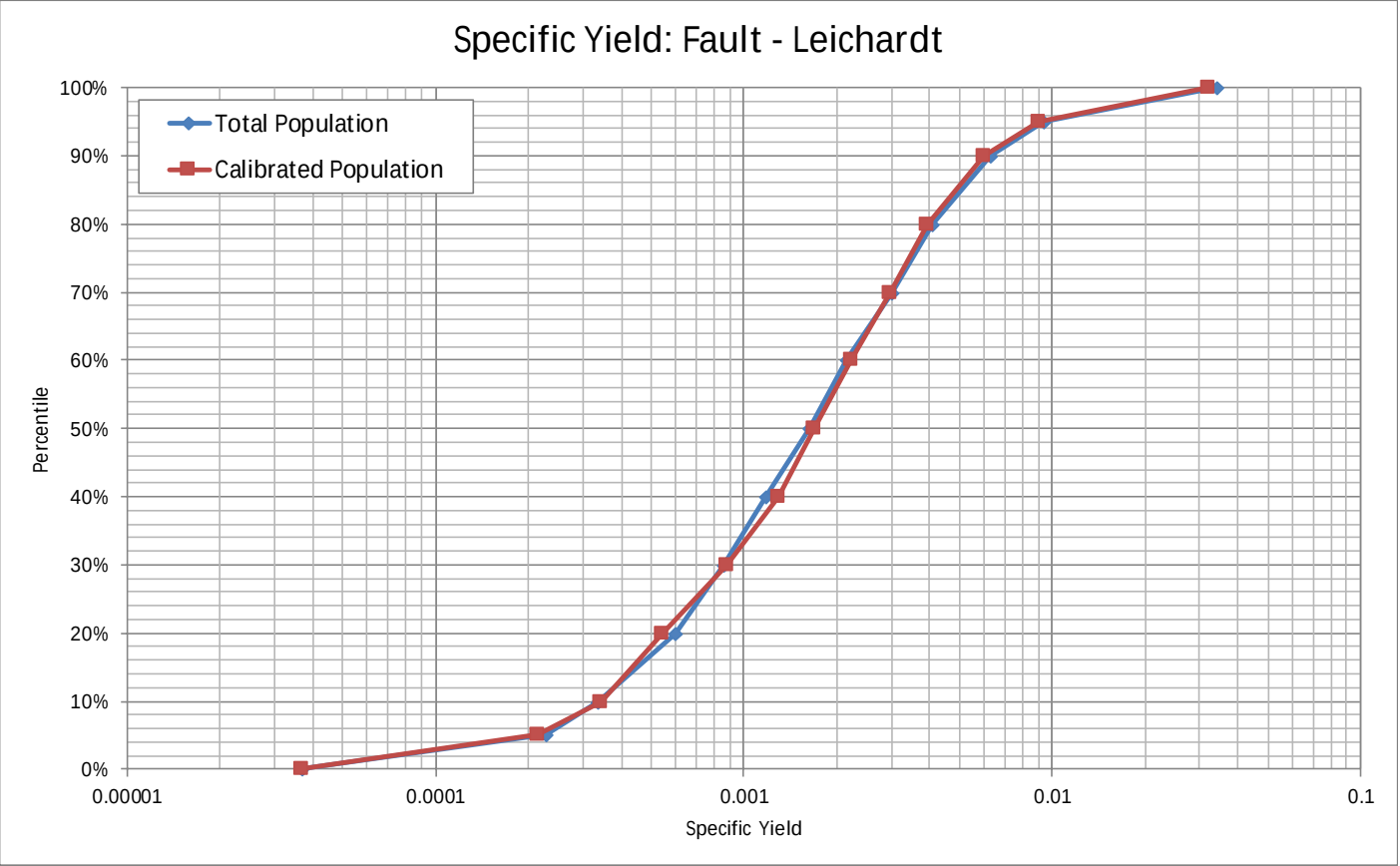
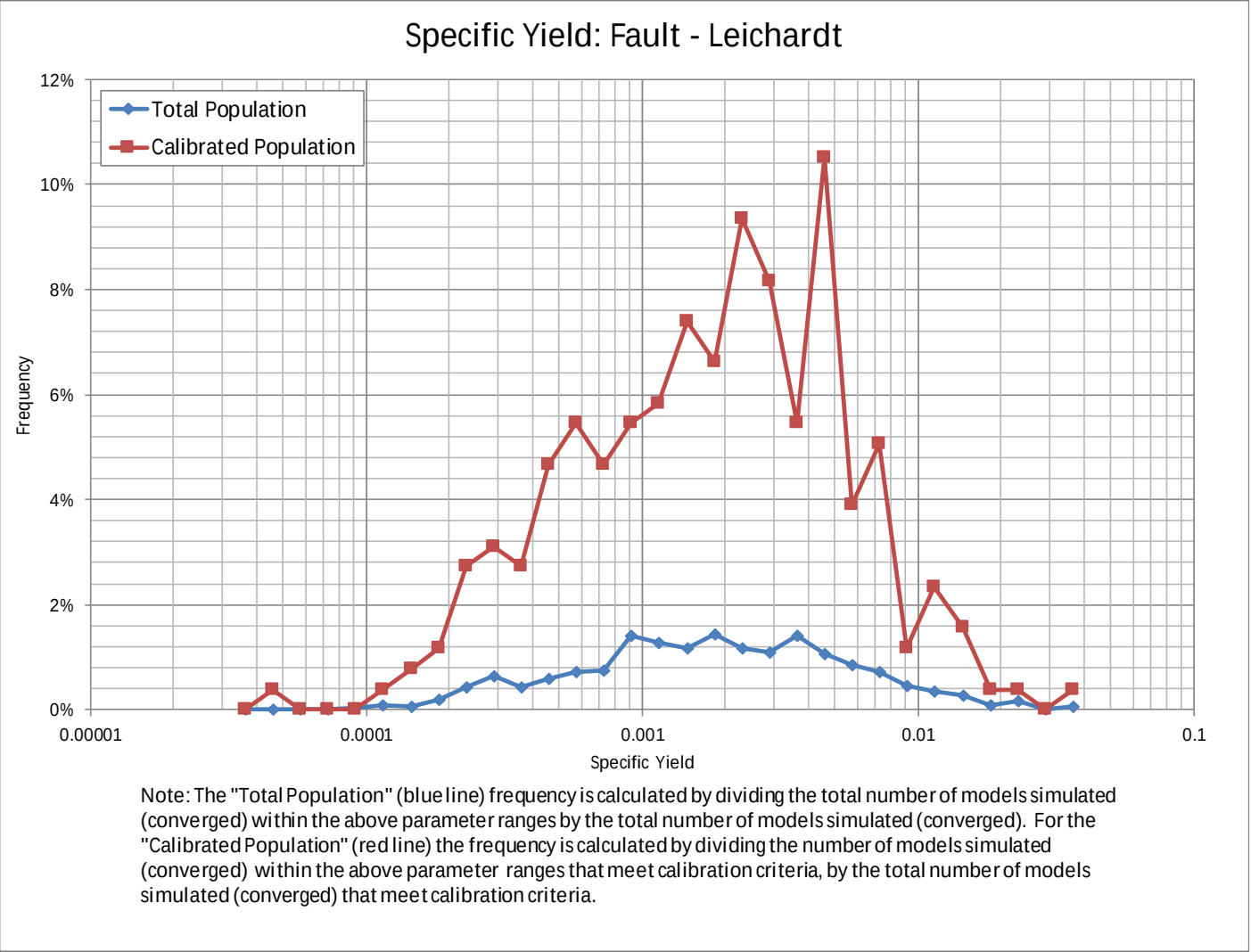
Specific Yield: Fault - Rewan

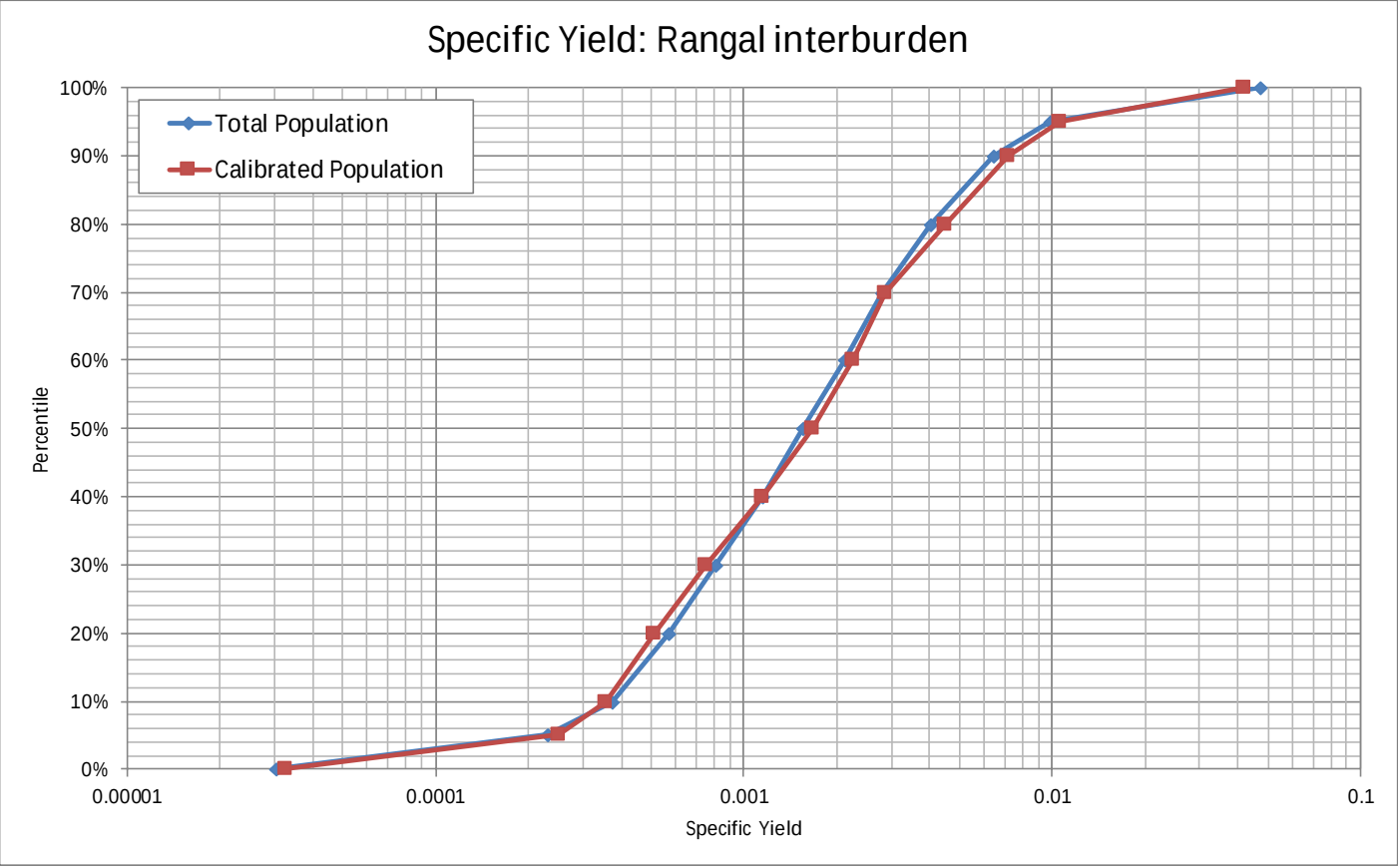
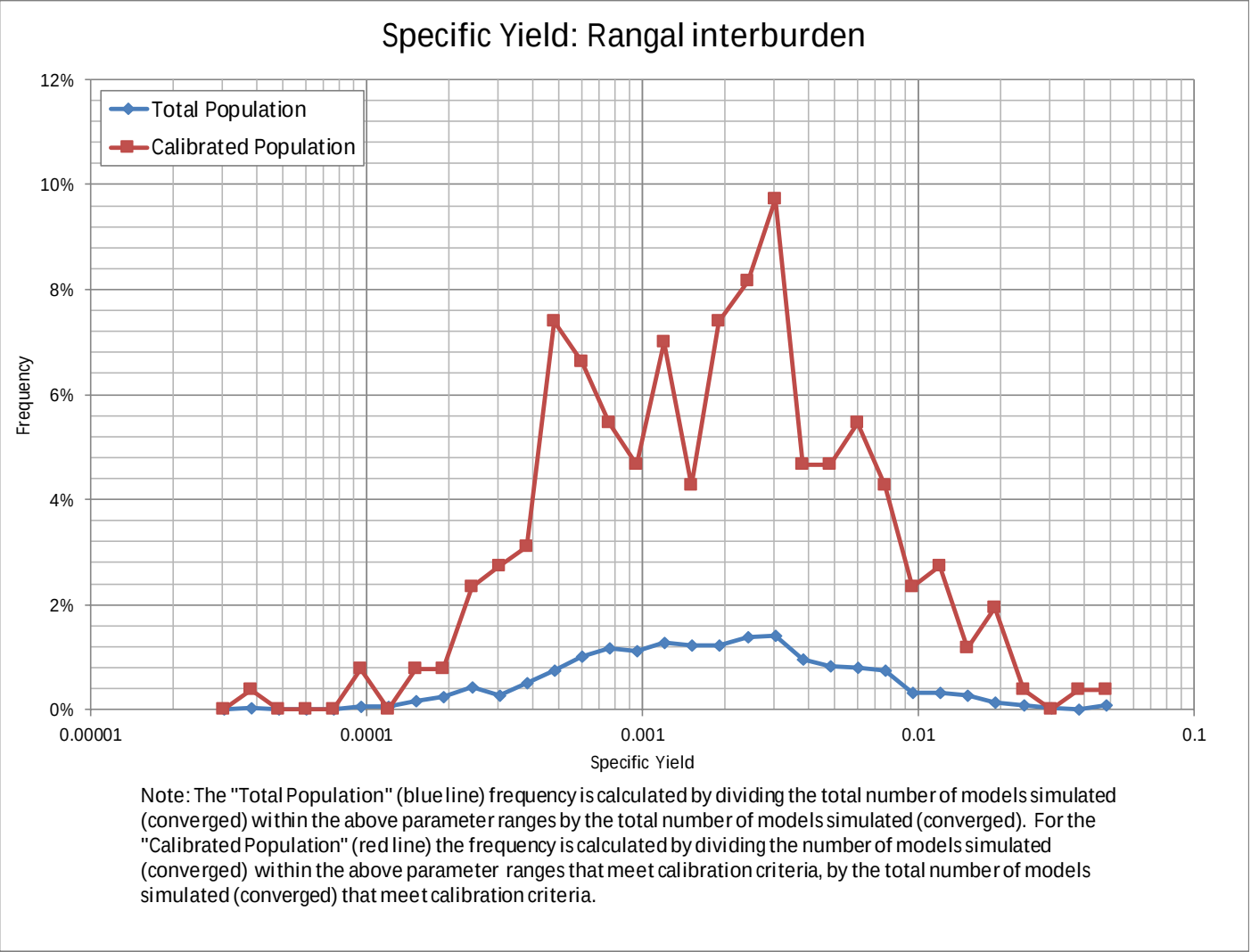




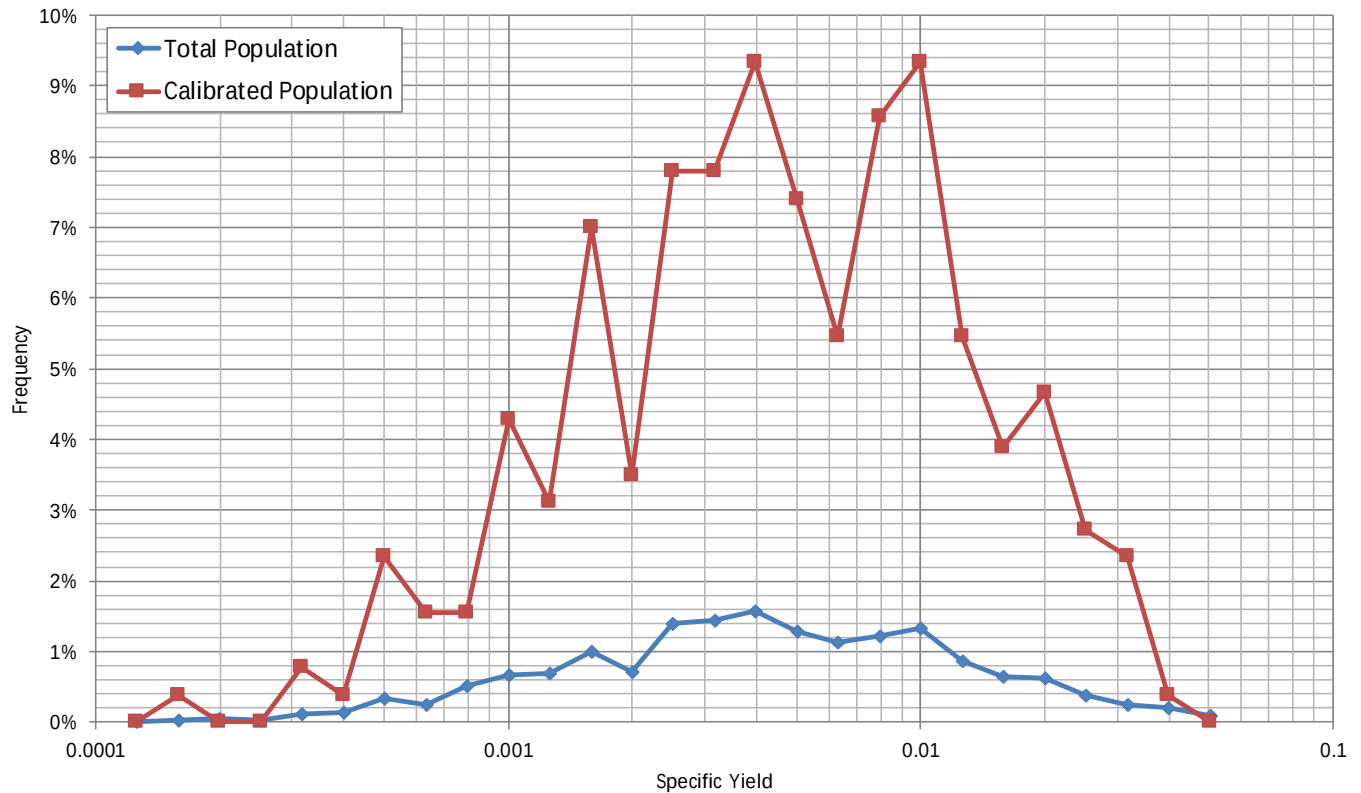






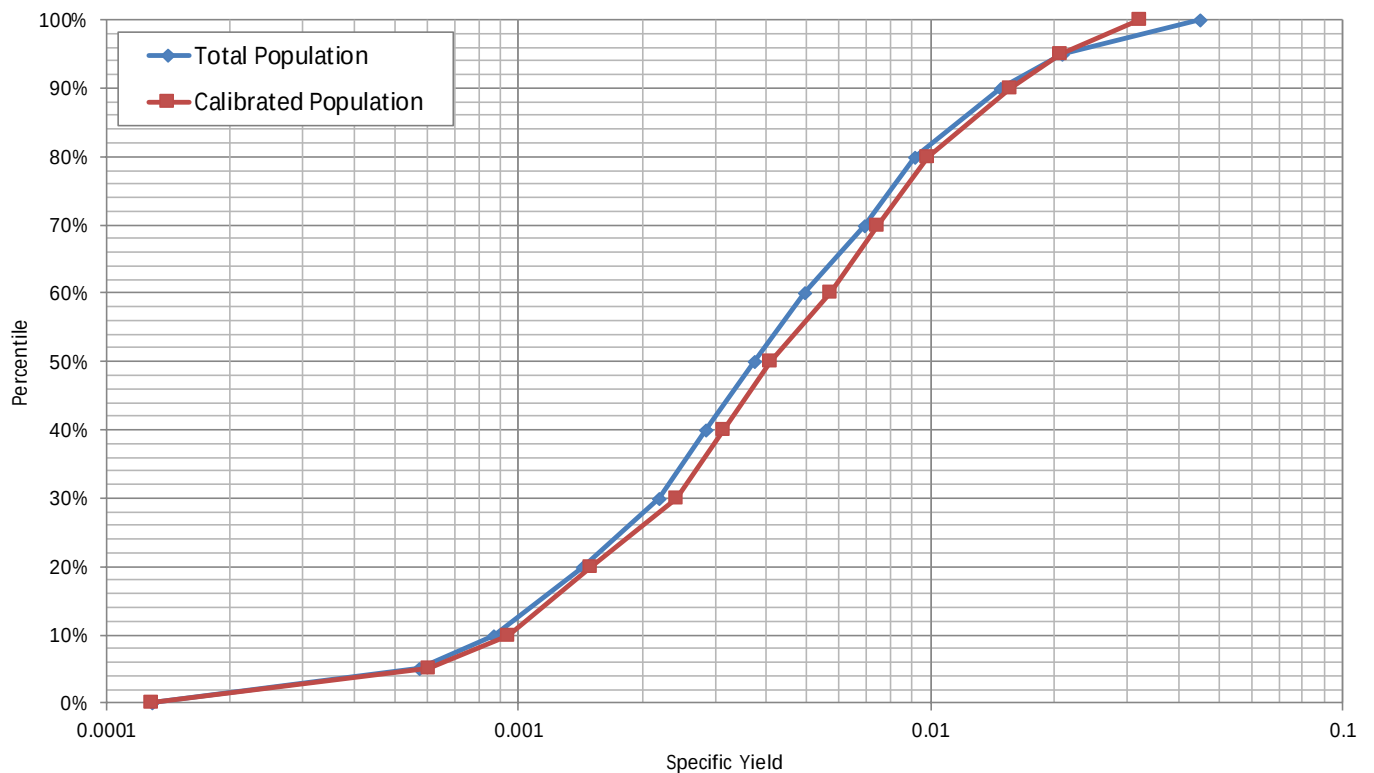


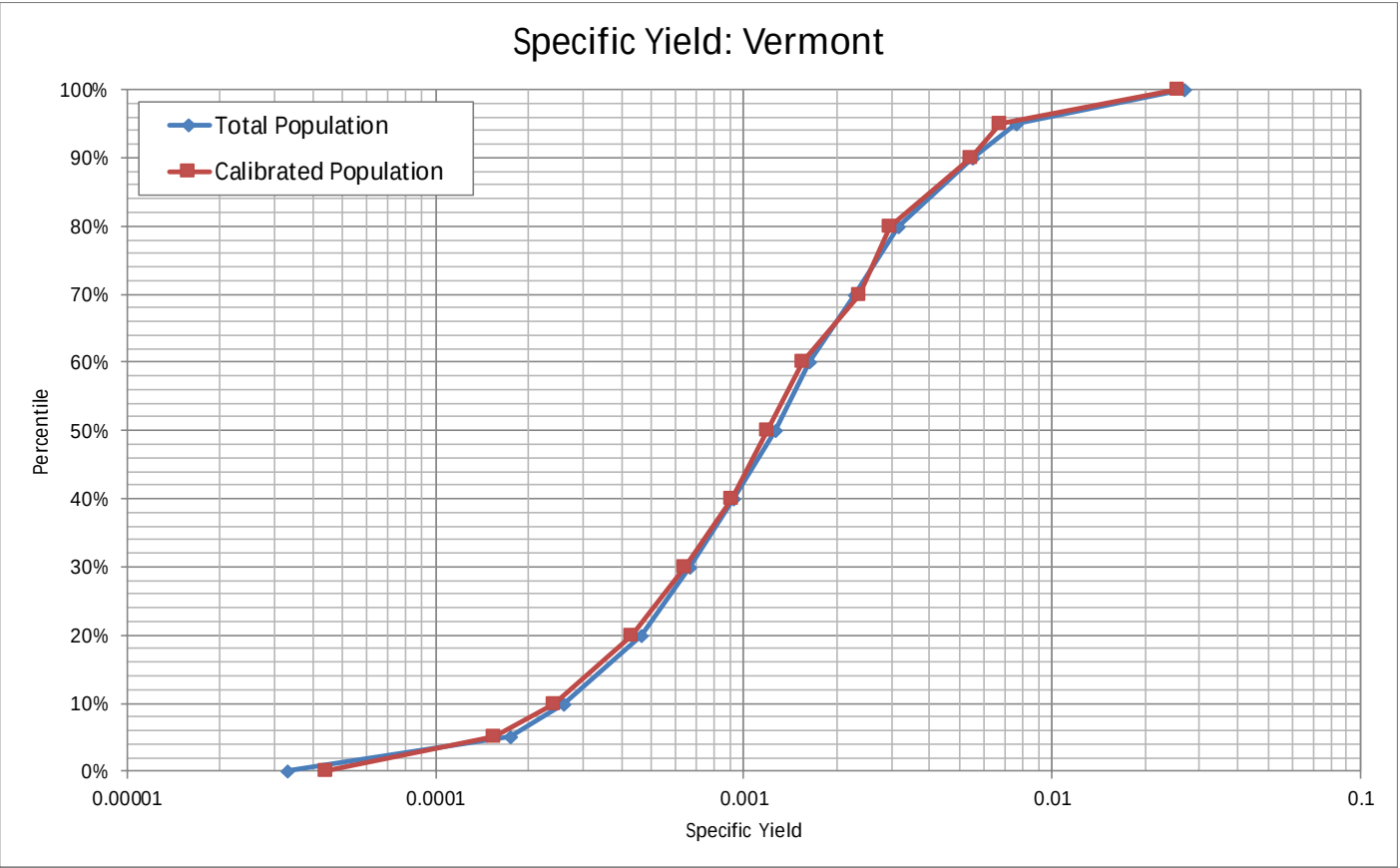
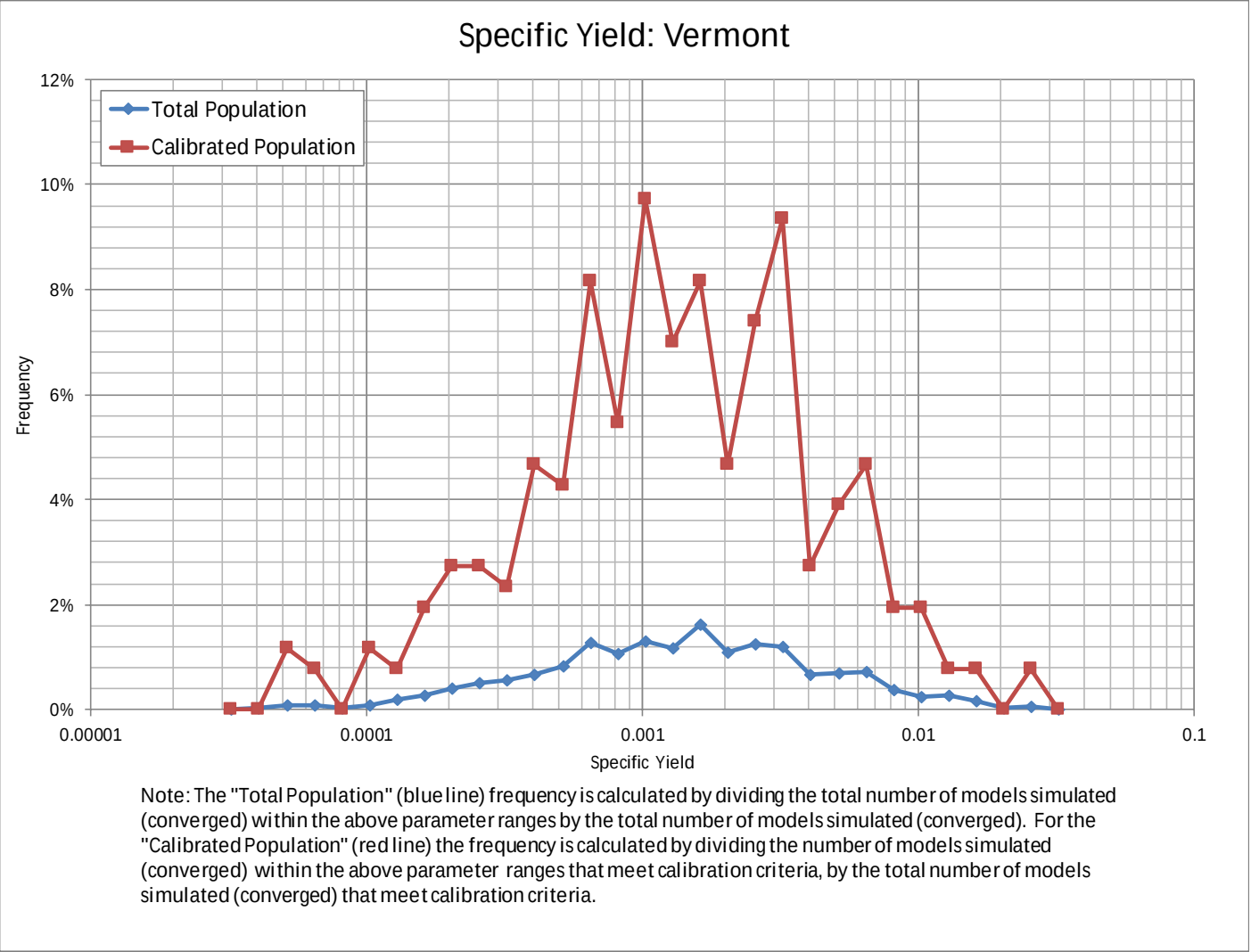
Specific Yield: Fault - Rangal interburden

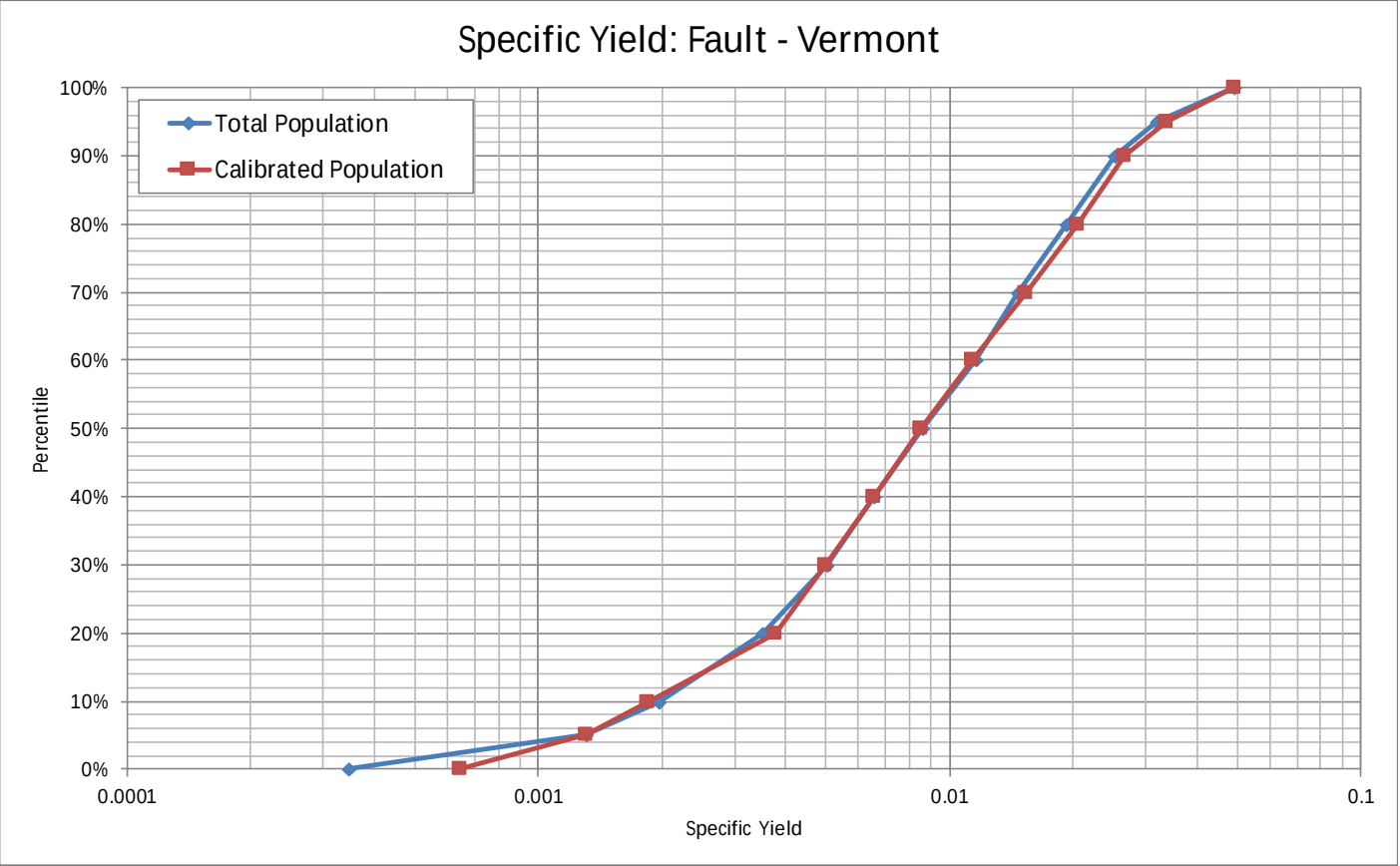
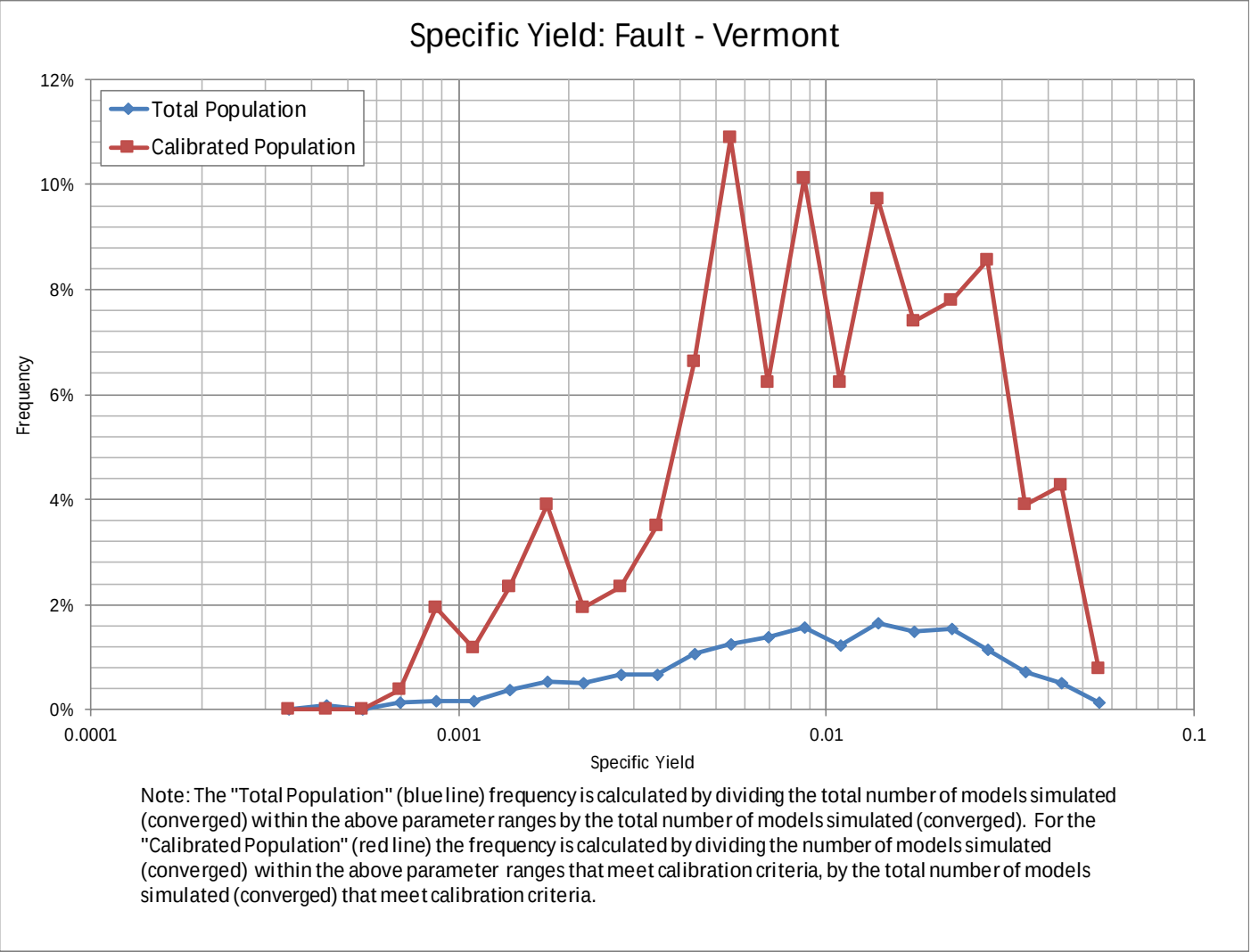


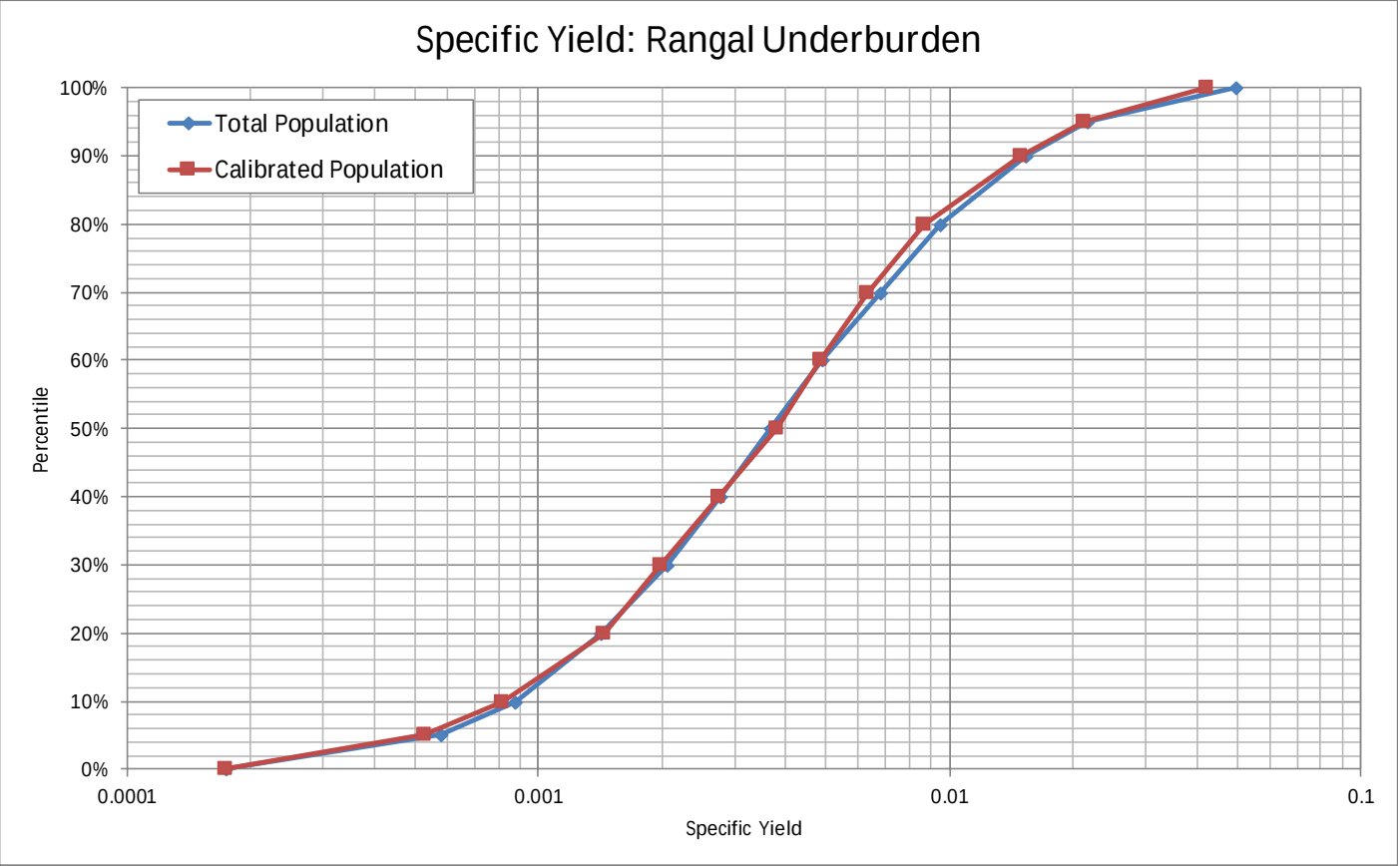
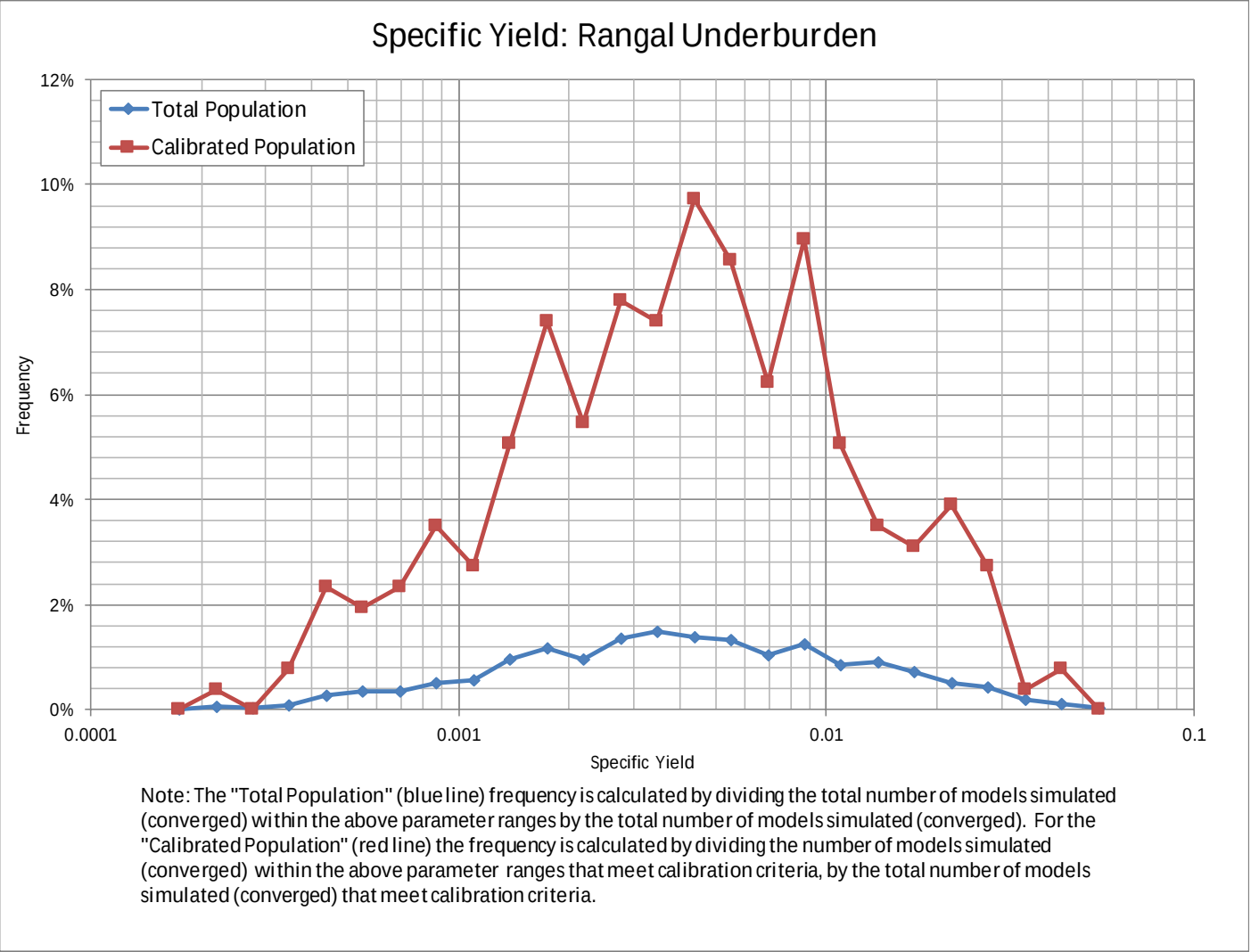
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Specific Yield: Fault - Rangal interburden

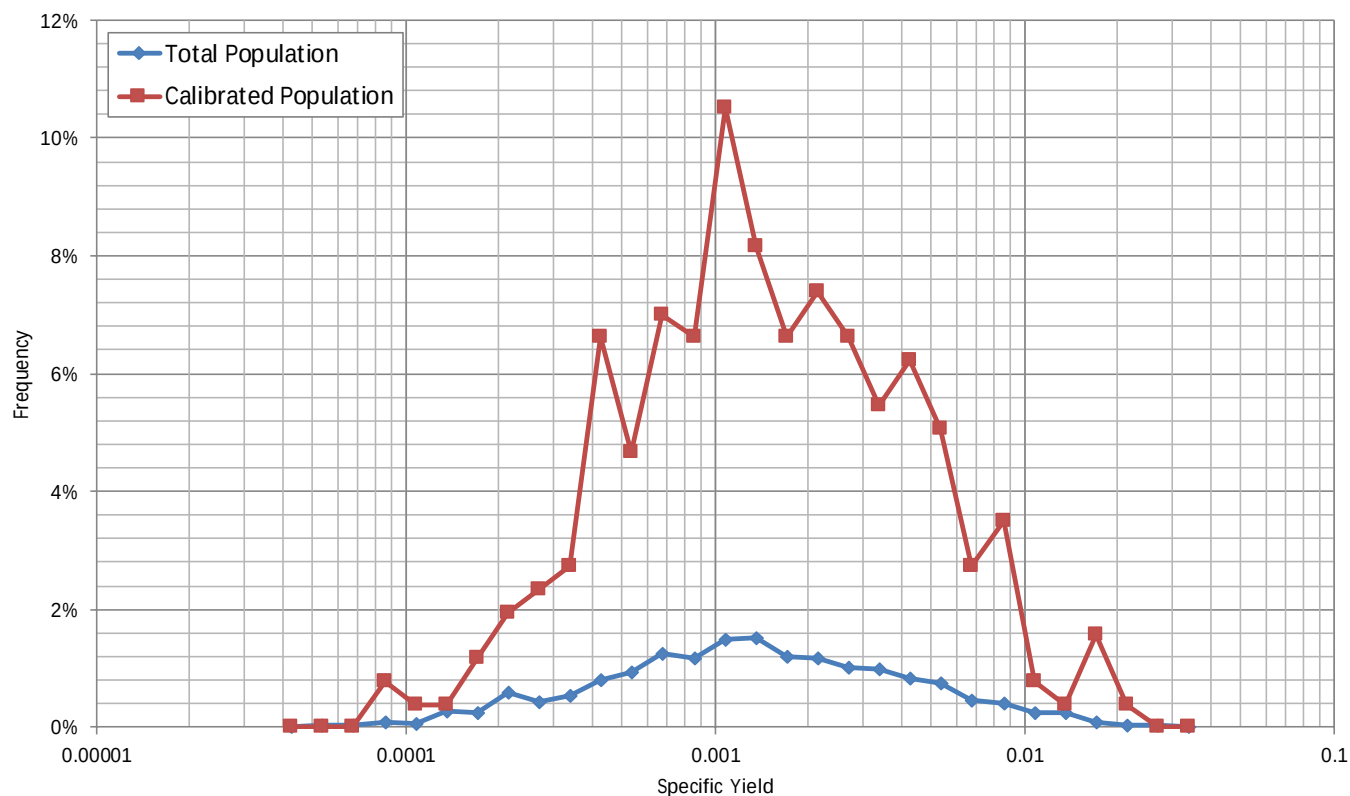






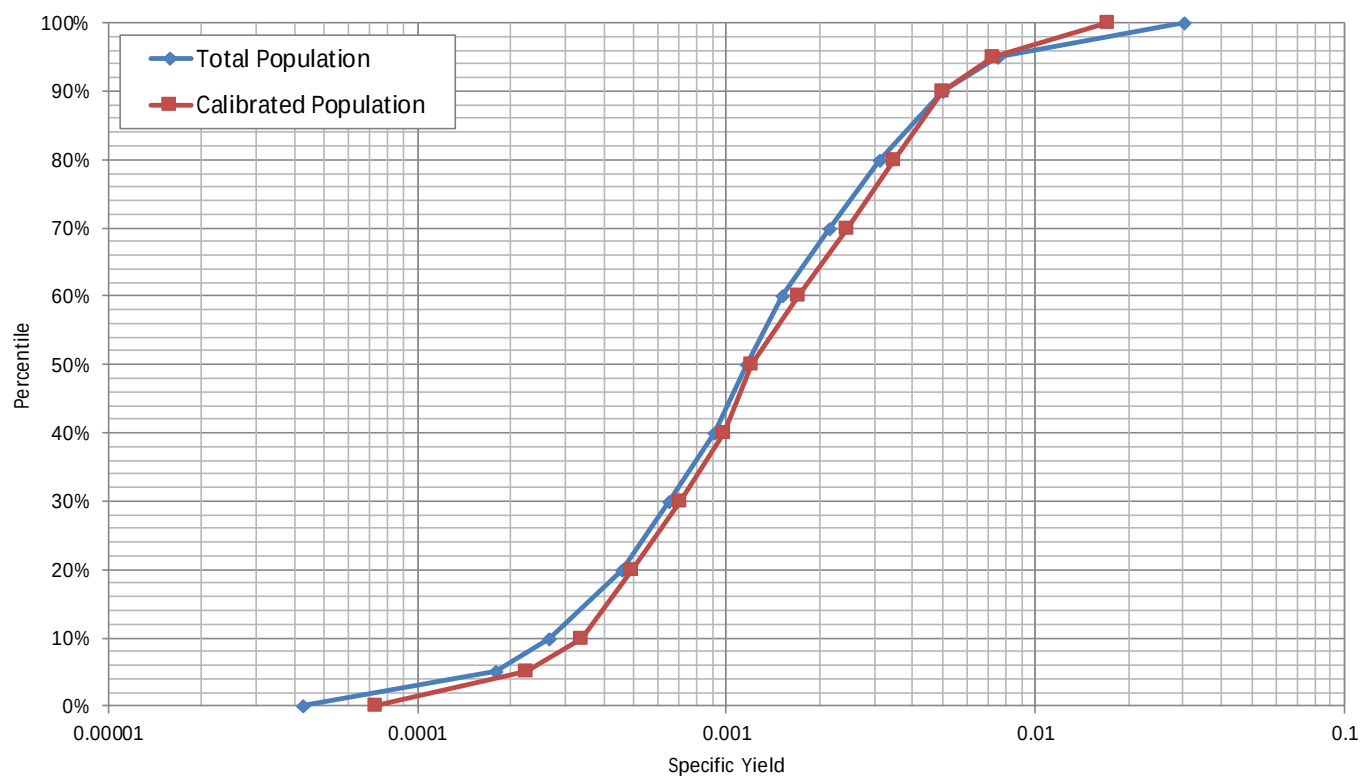


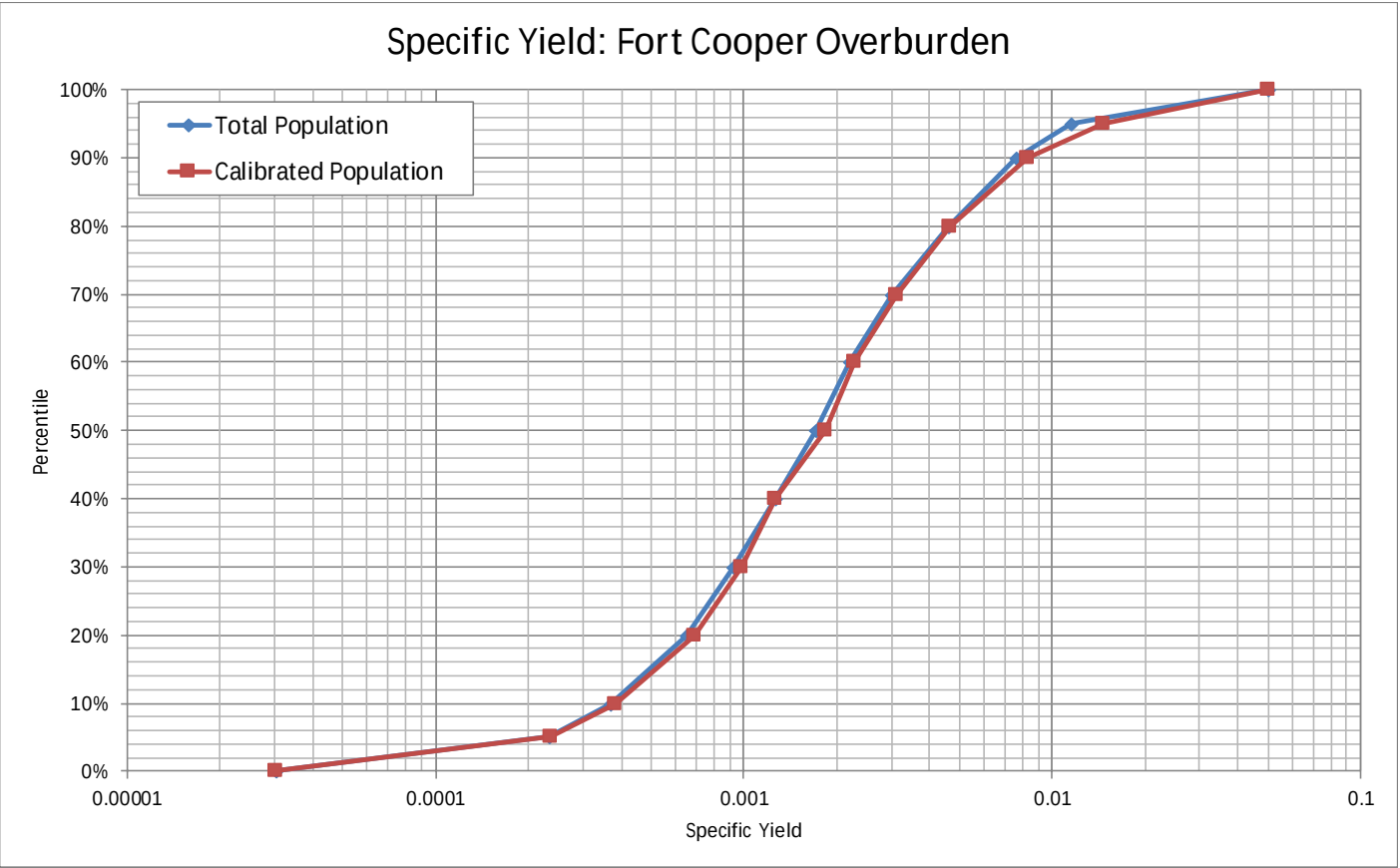
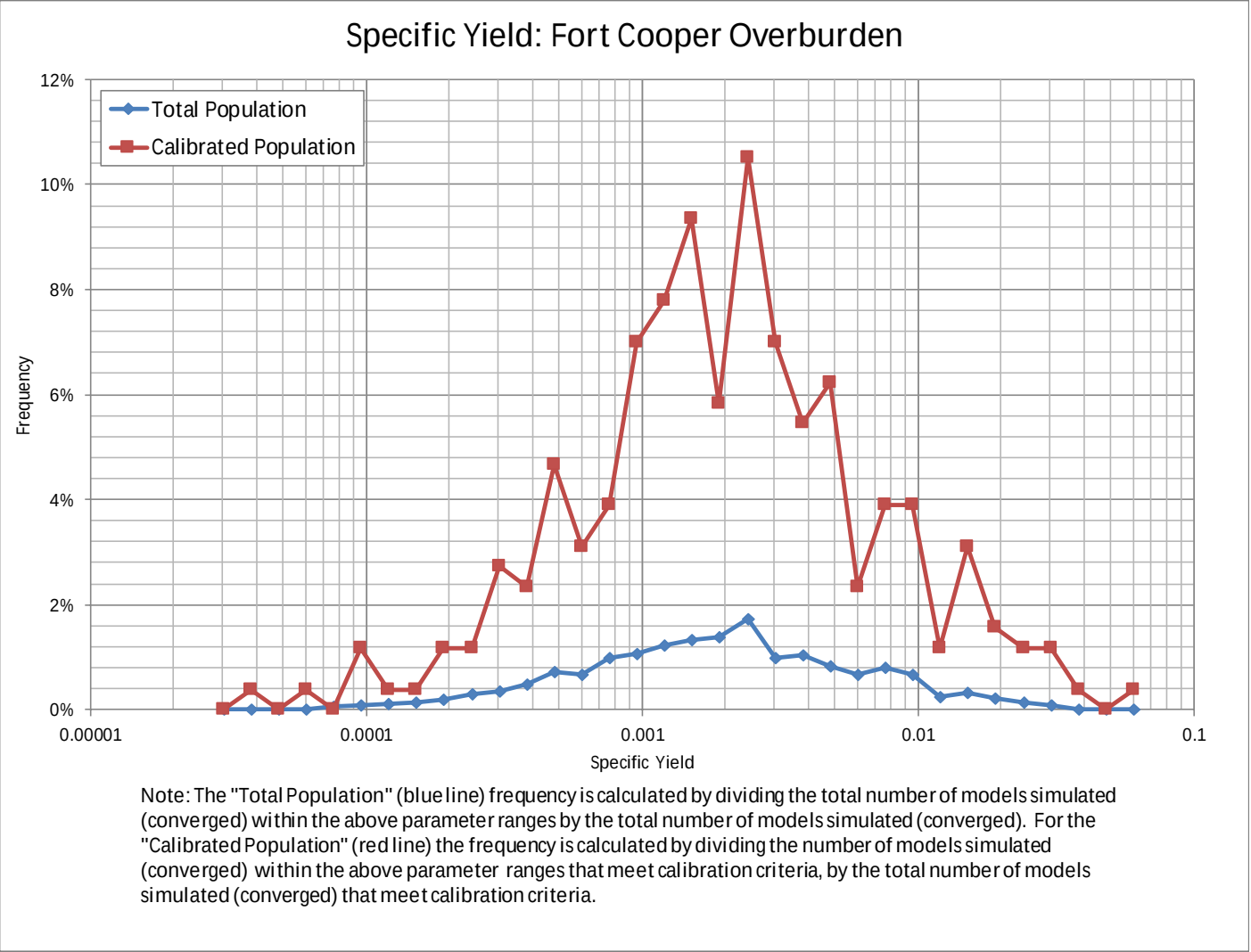
Specific Yield: Fault - Rangal Underburden

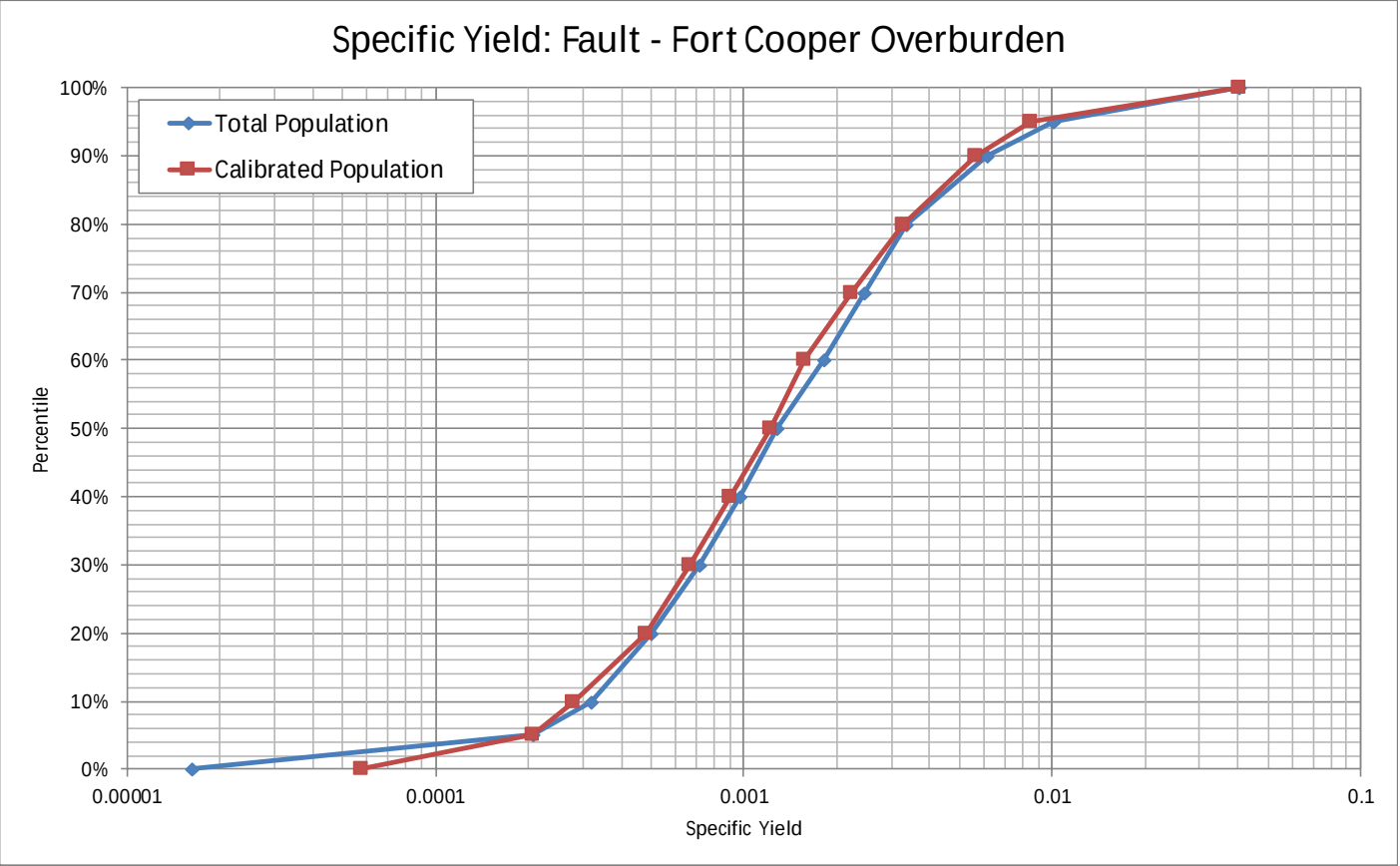
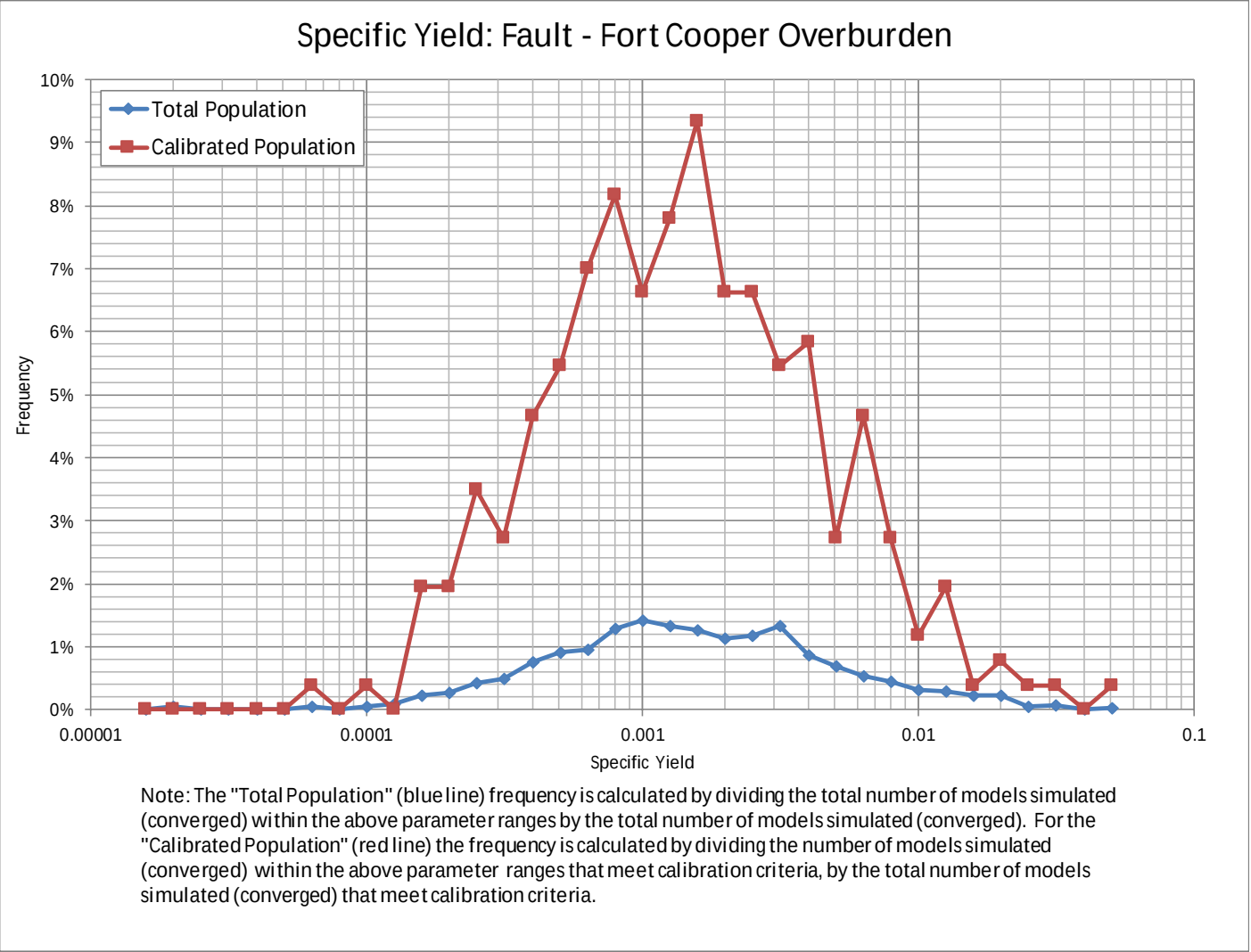


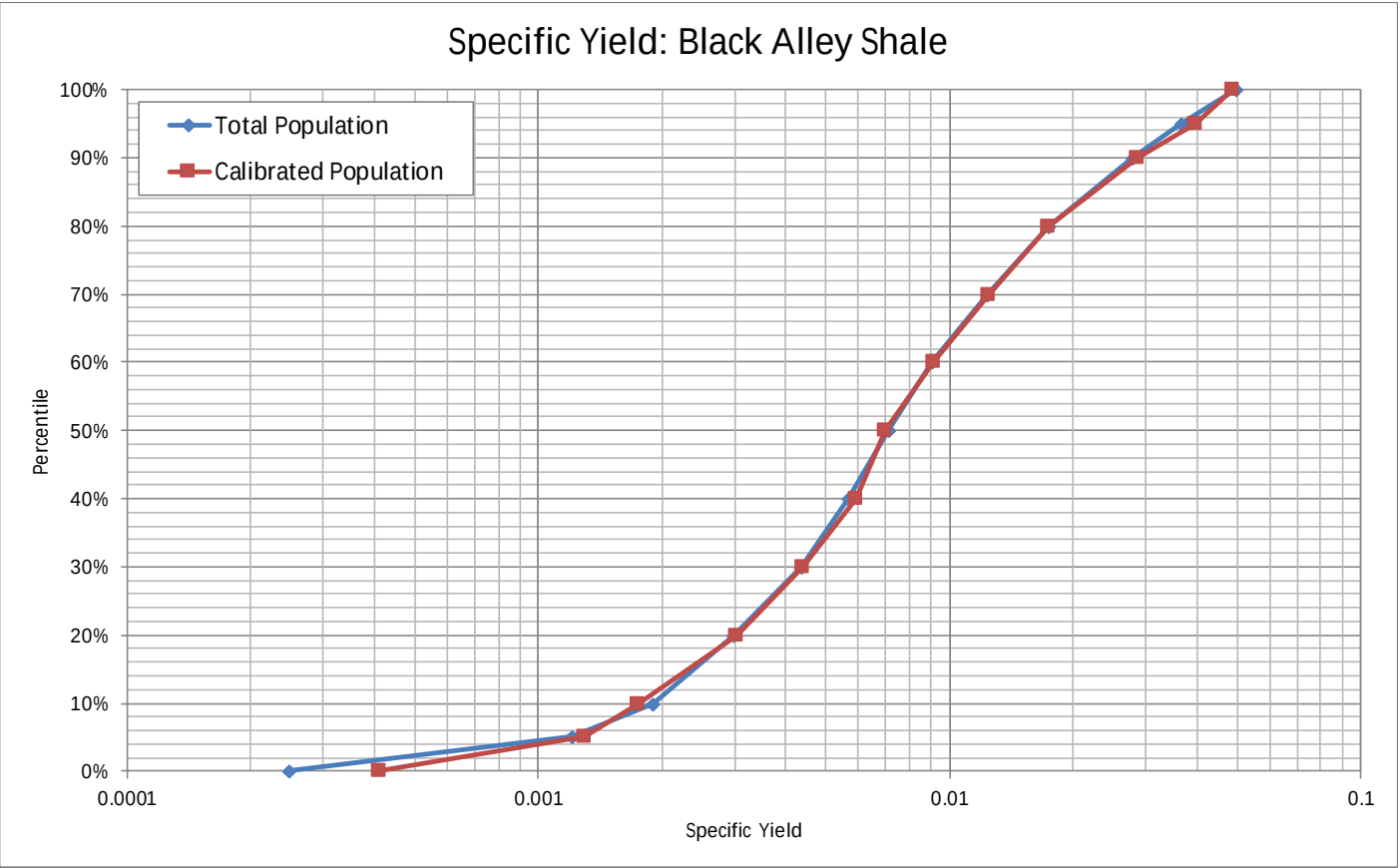
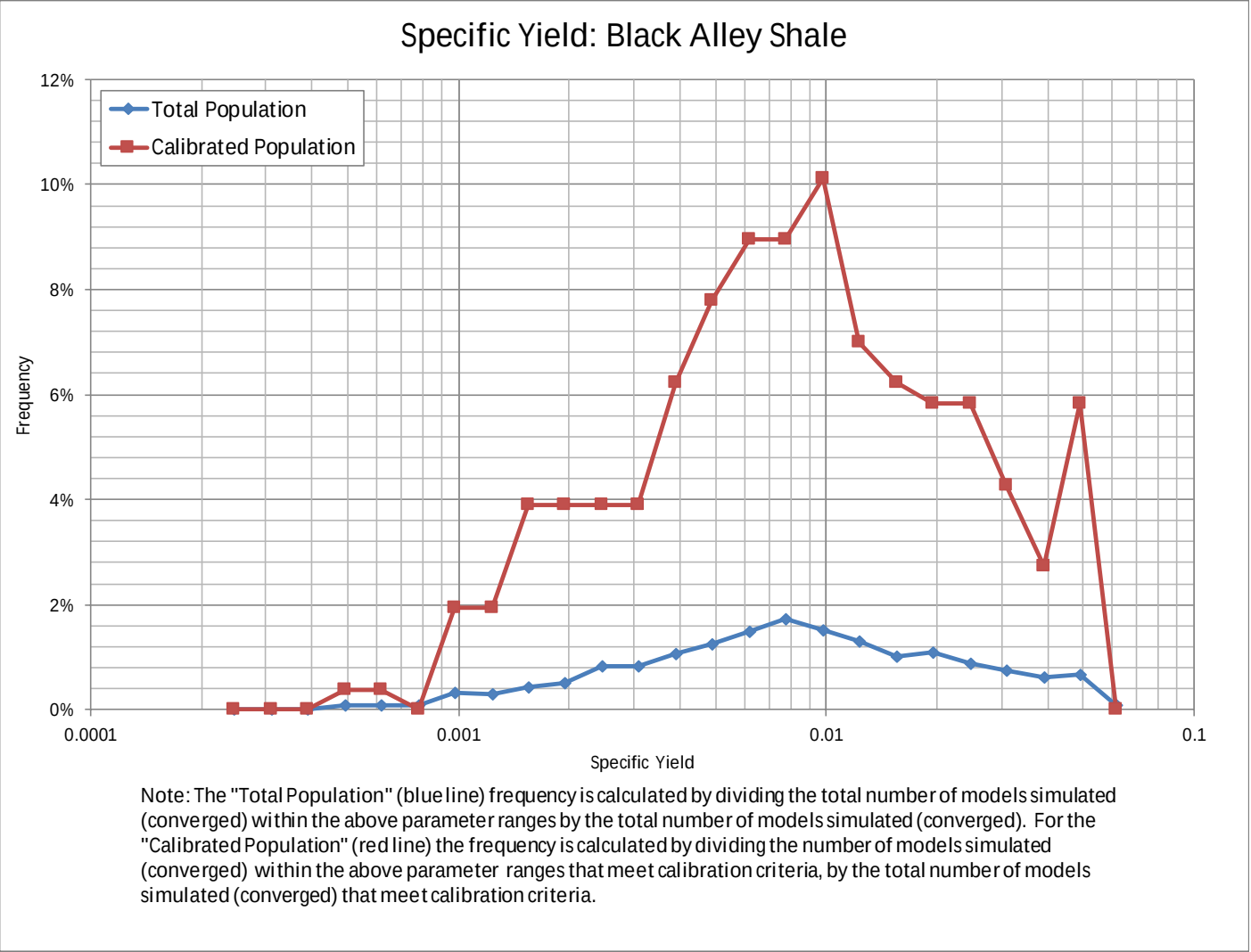
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

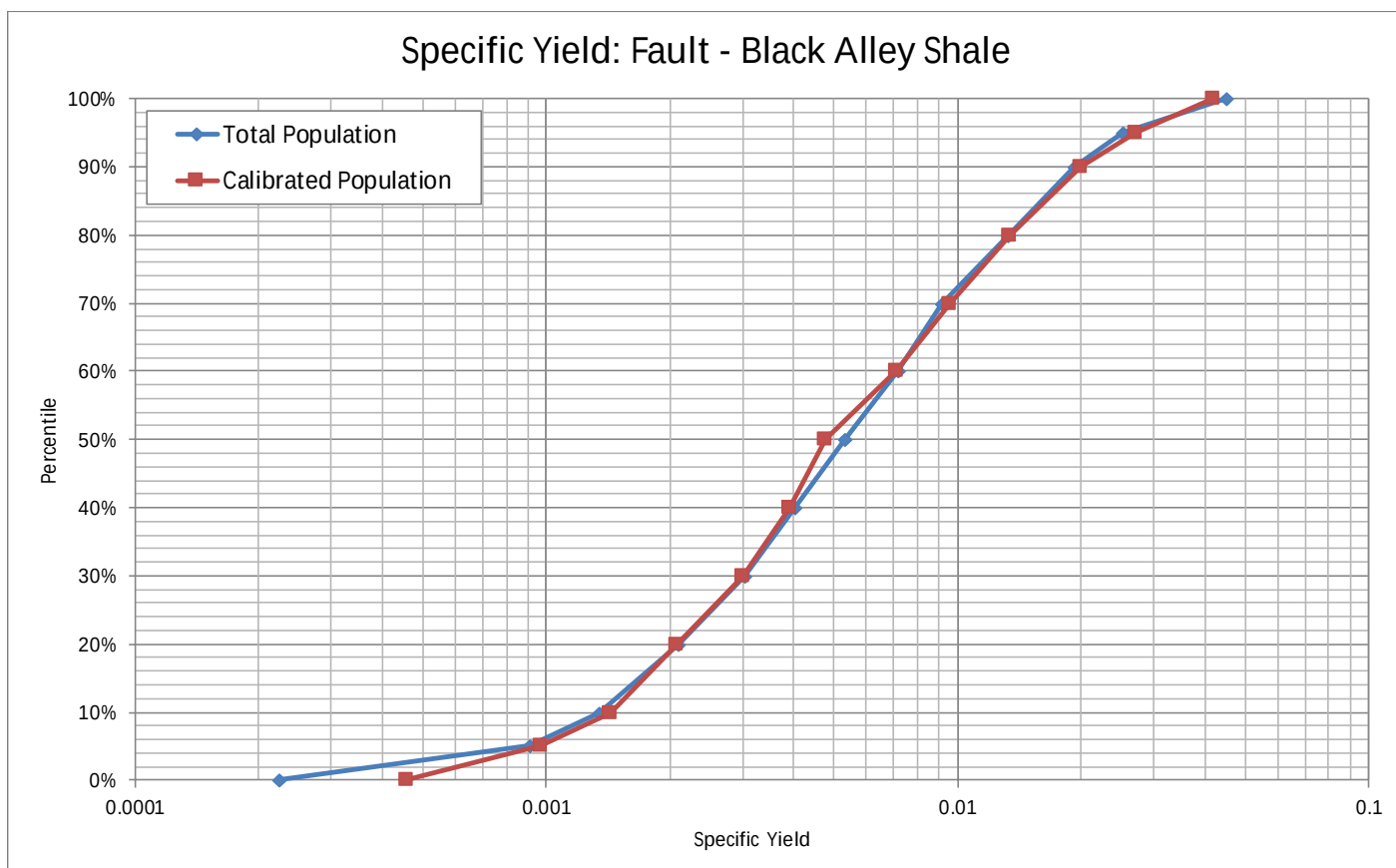
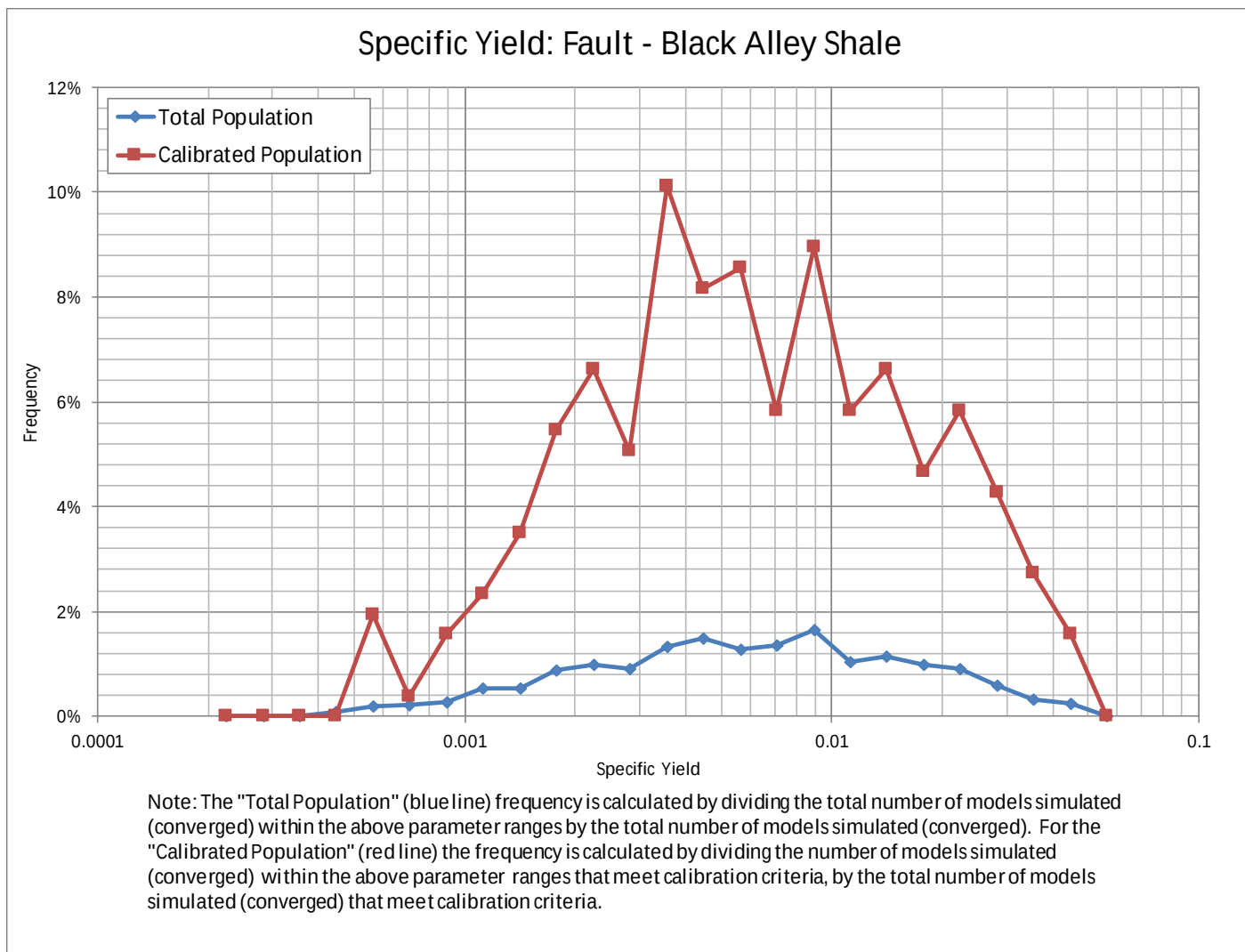
Specific Yield: Fault - Rangal Underburden

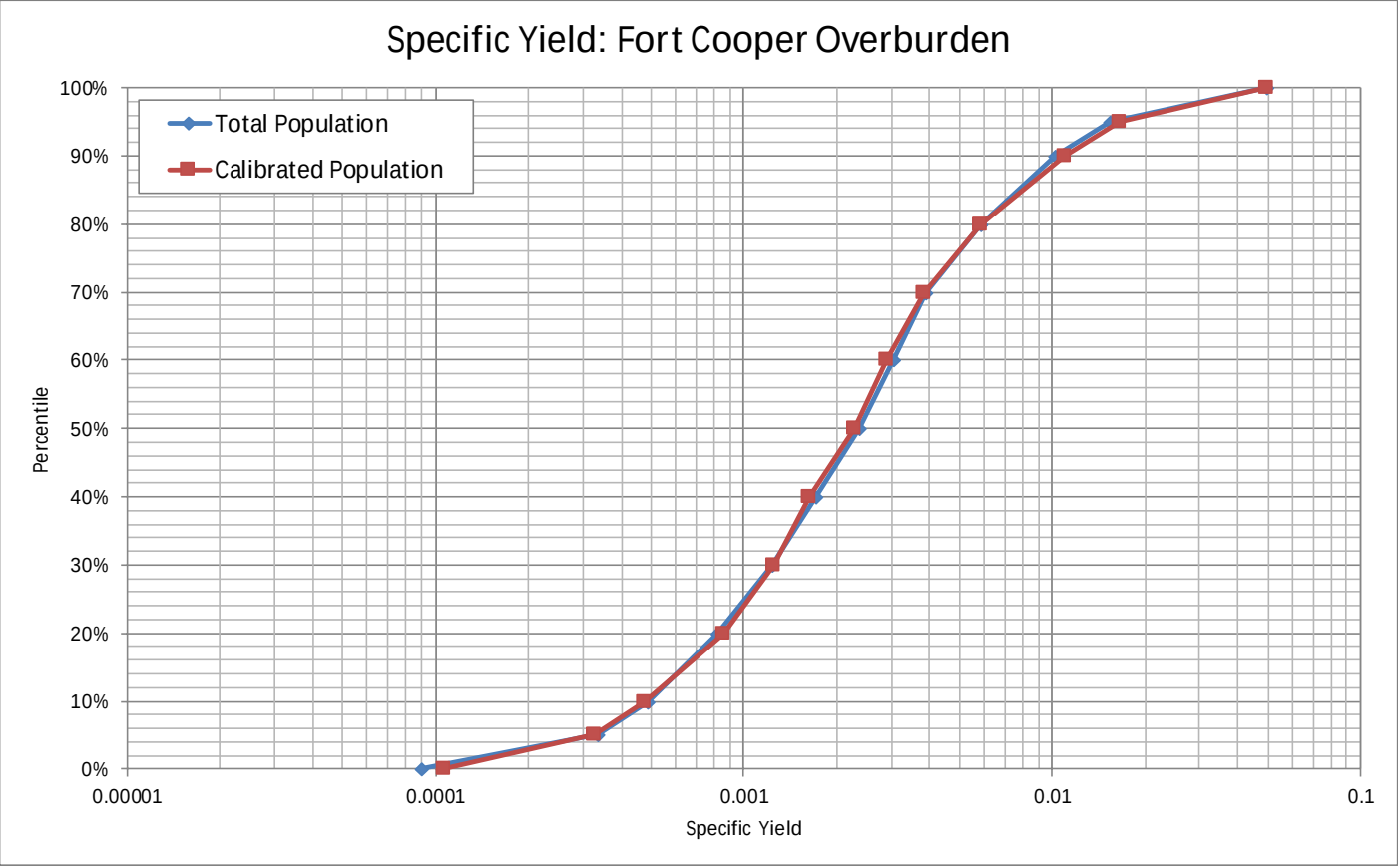
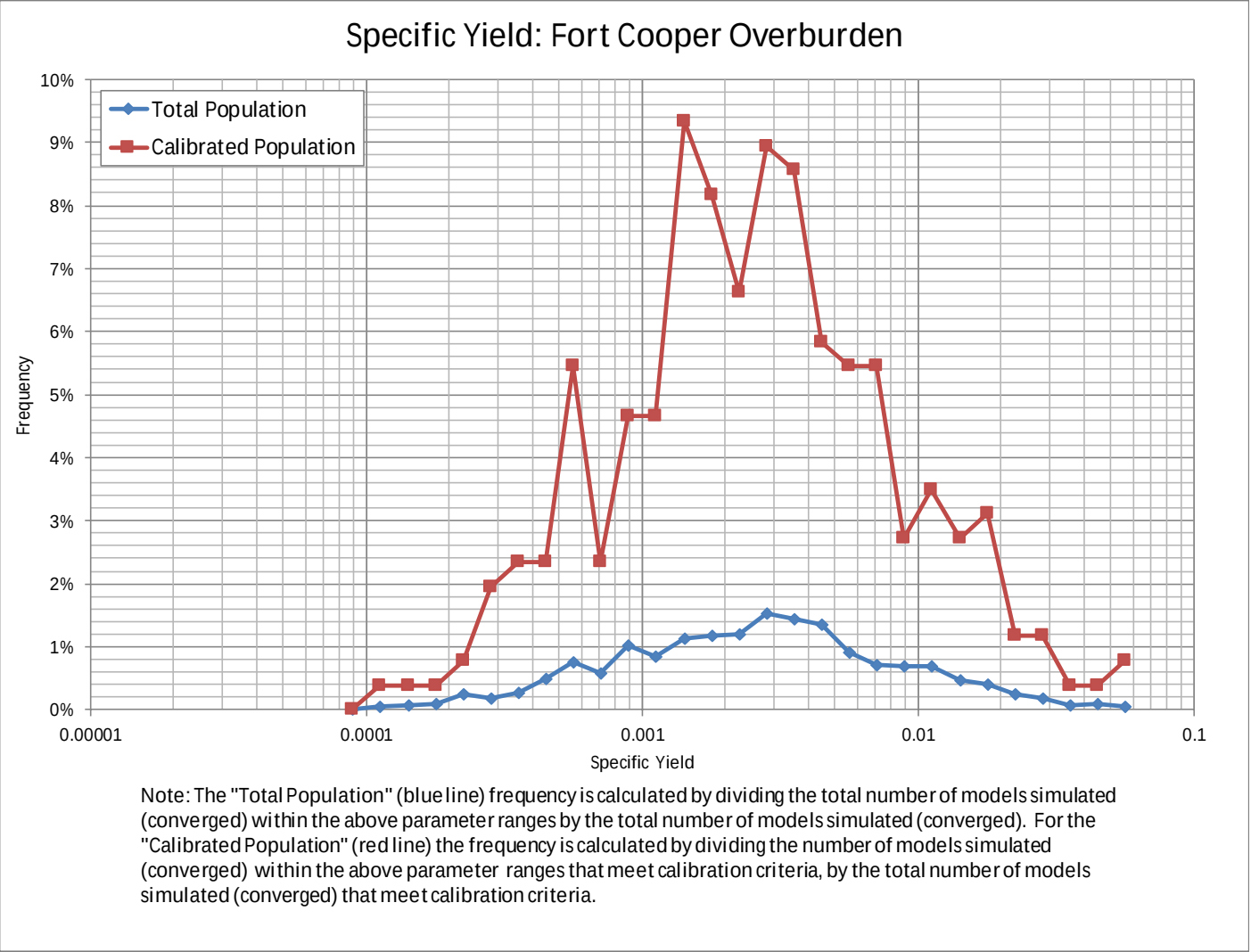


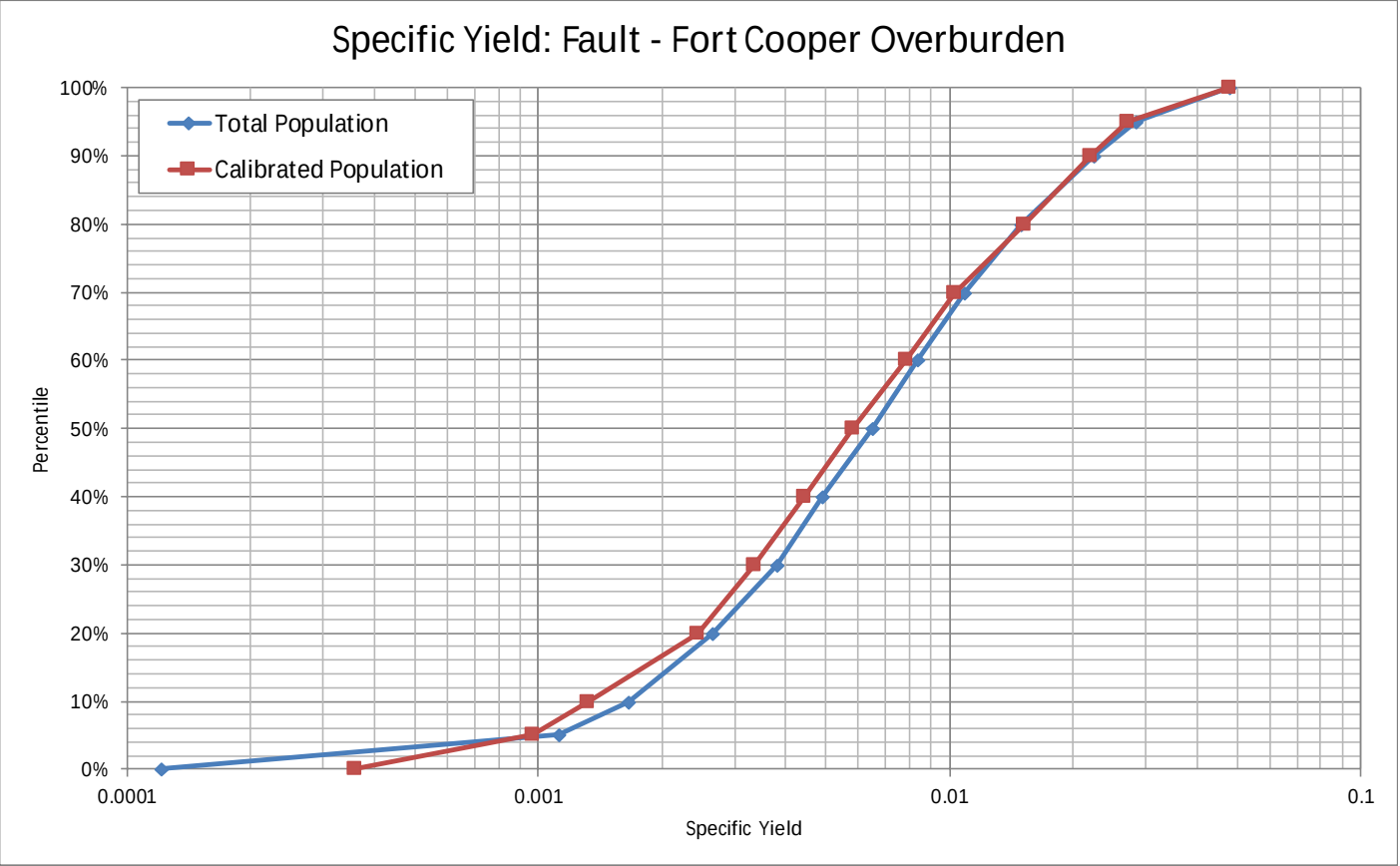
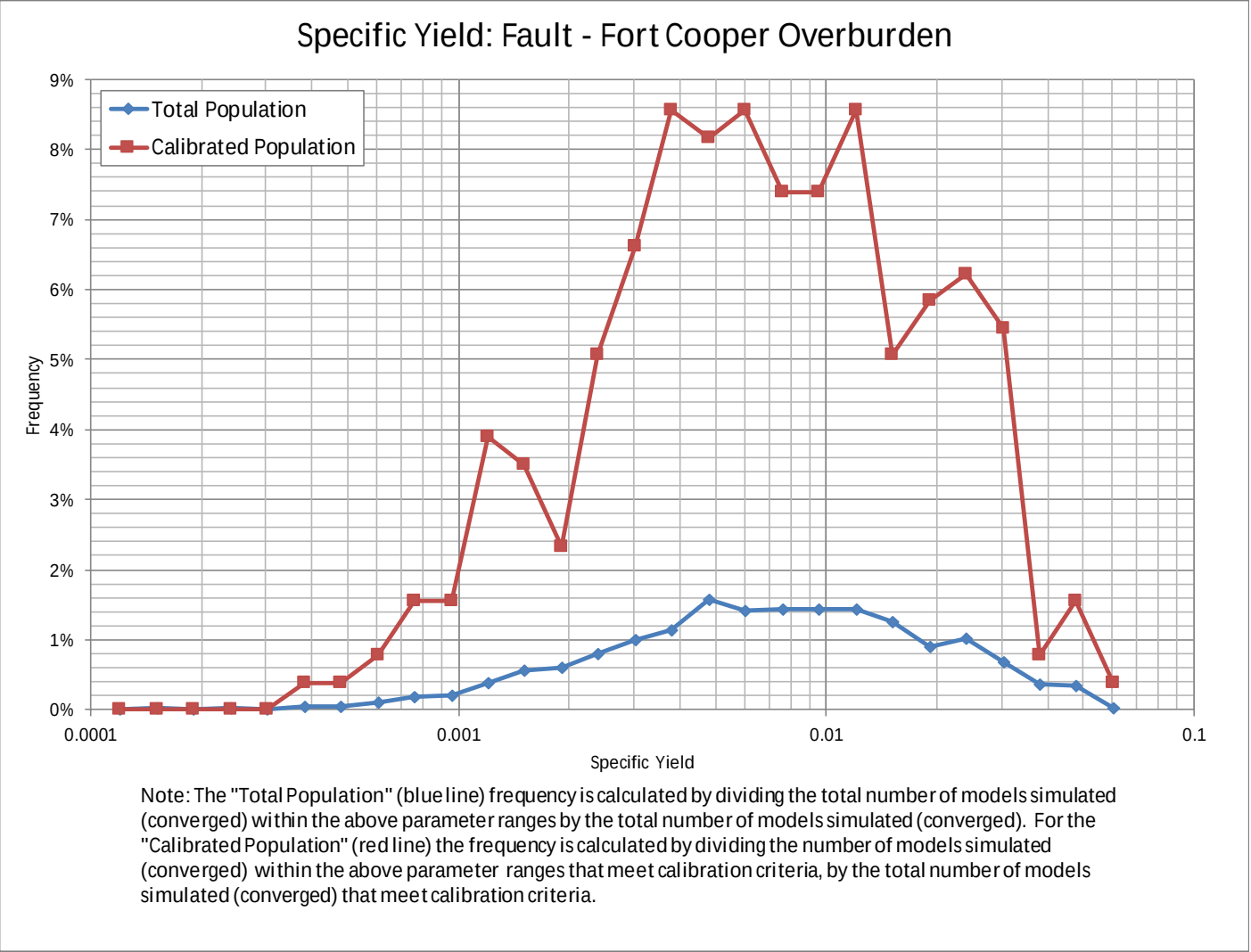


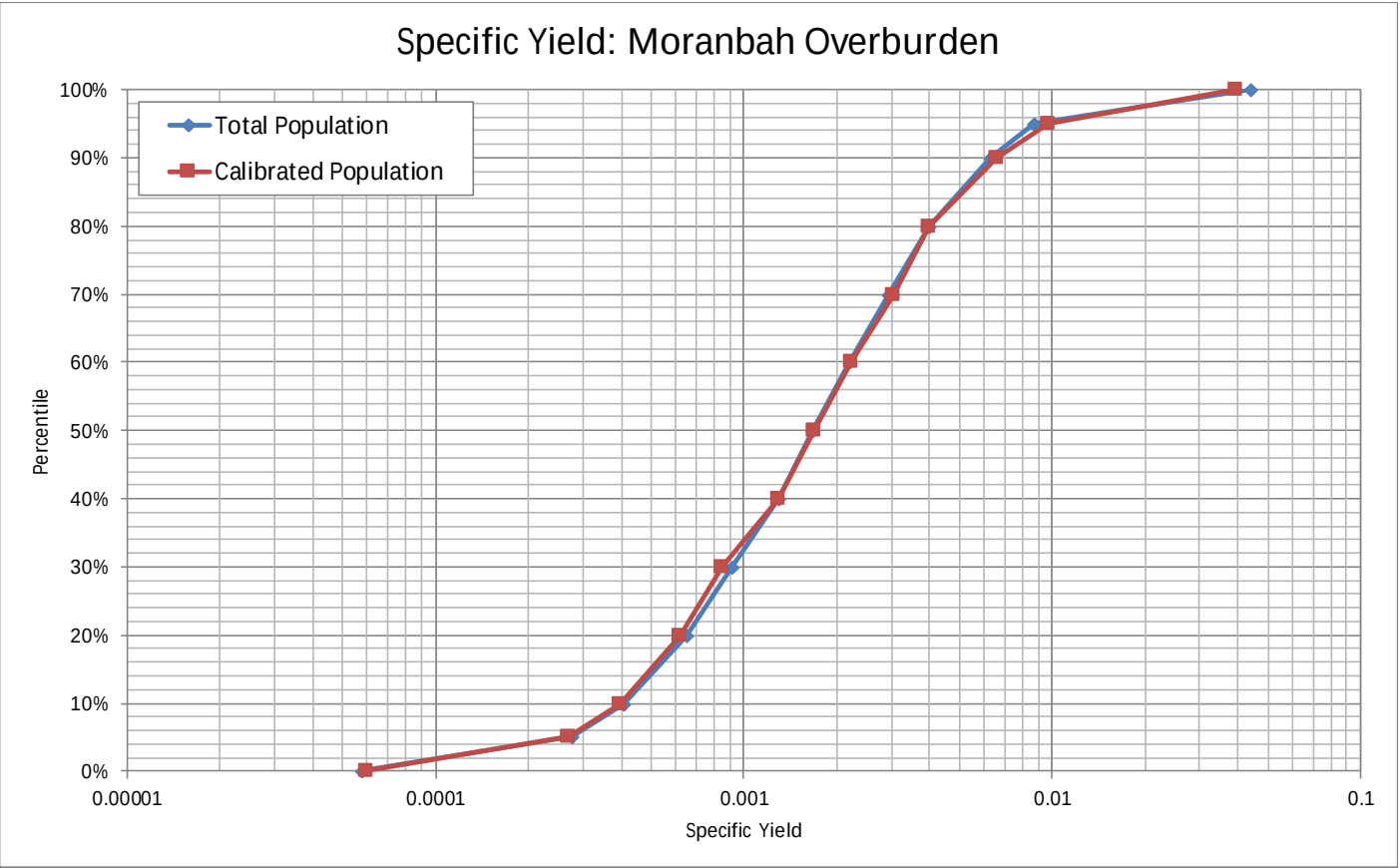
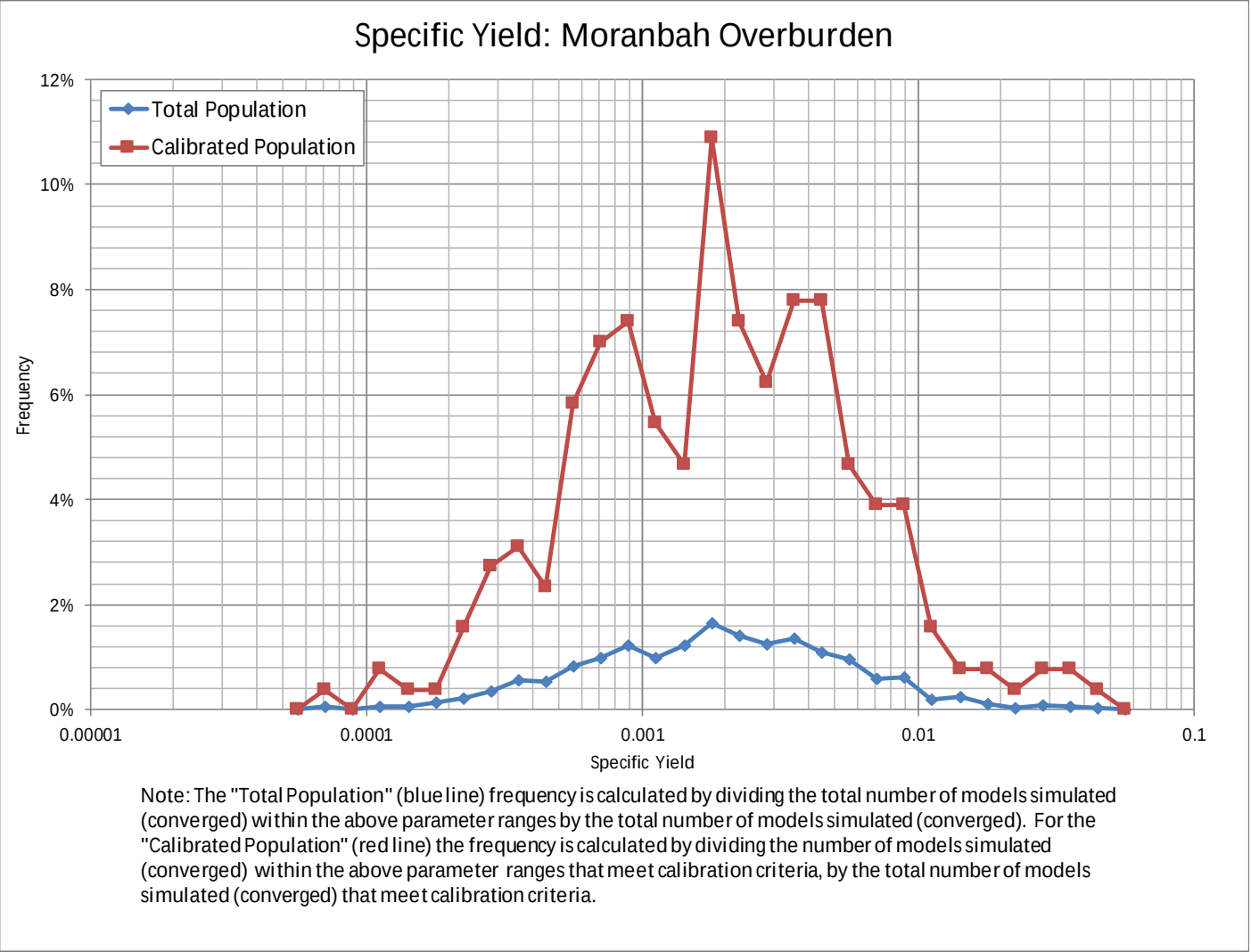


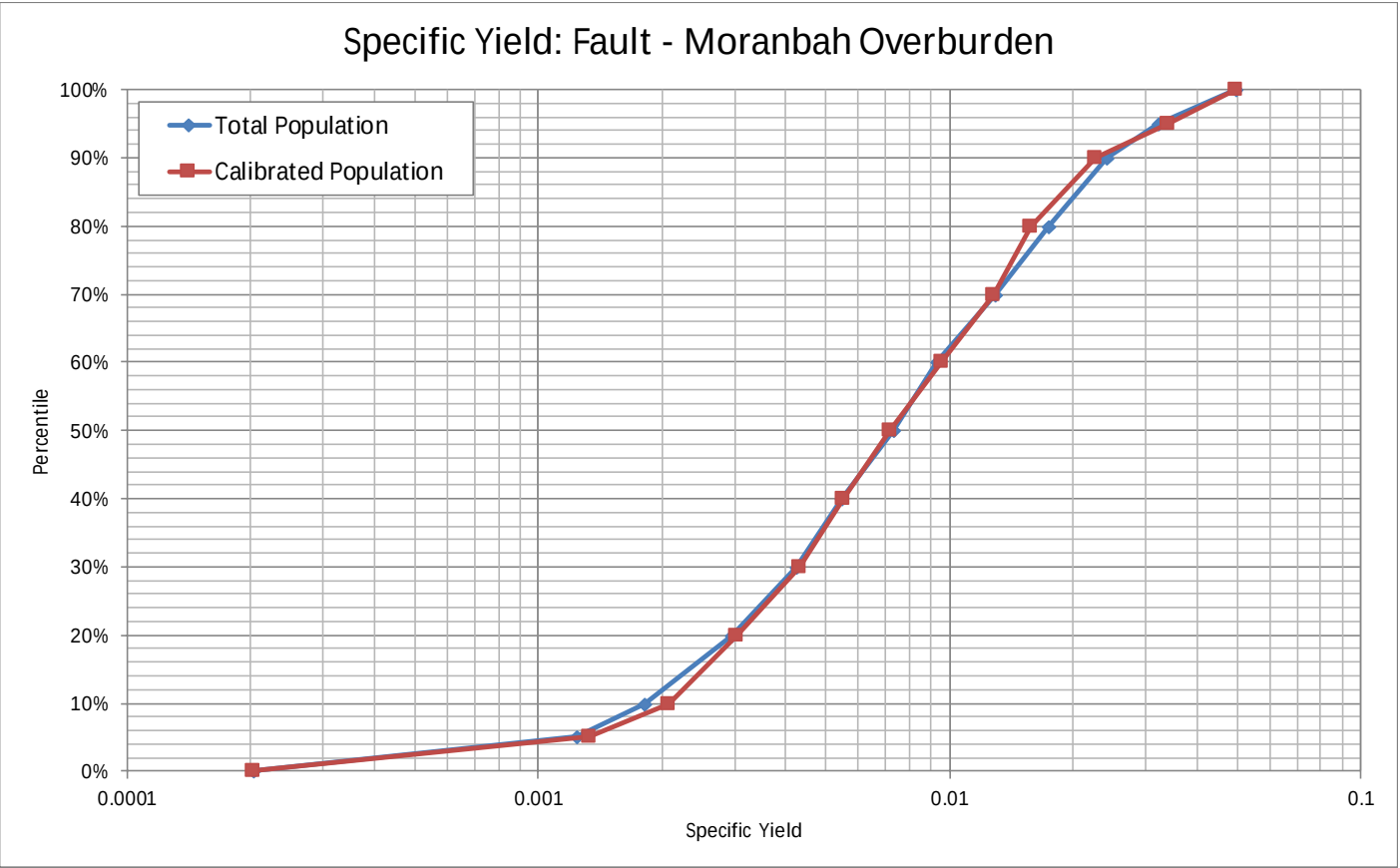
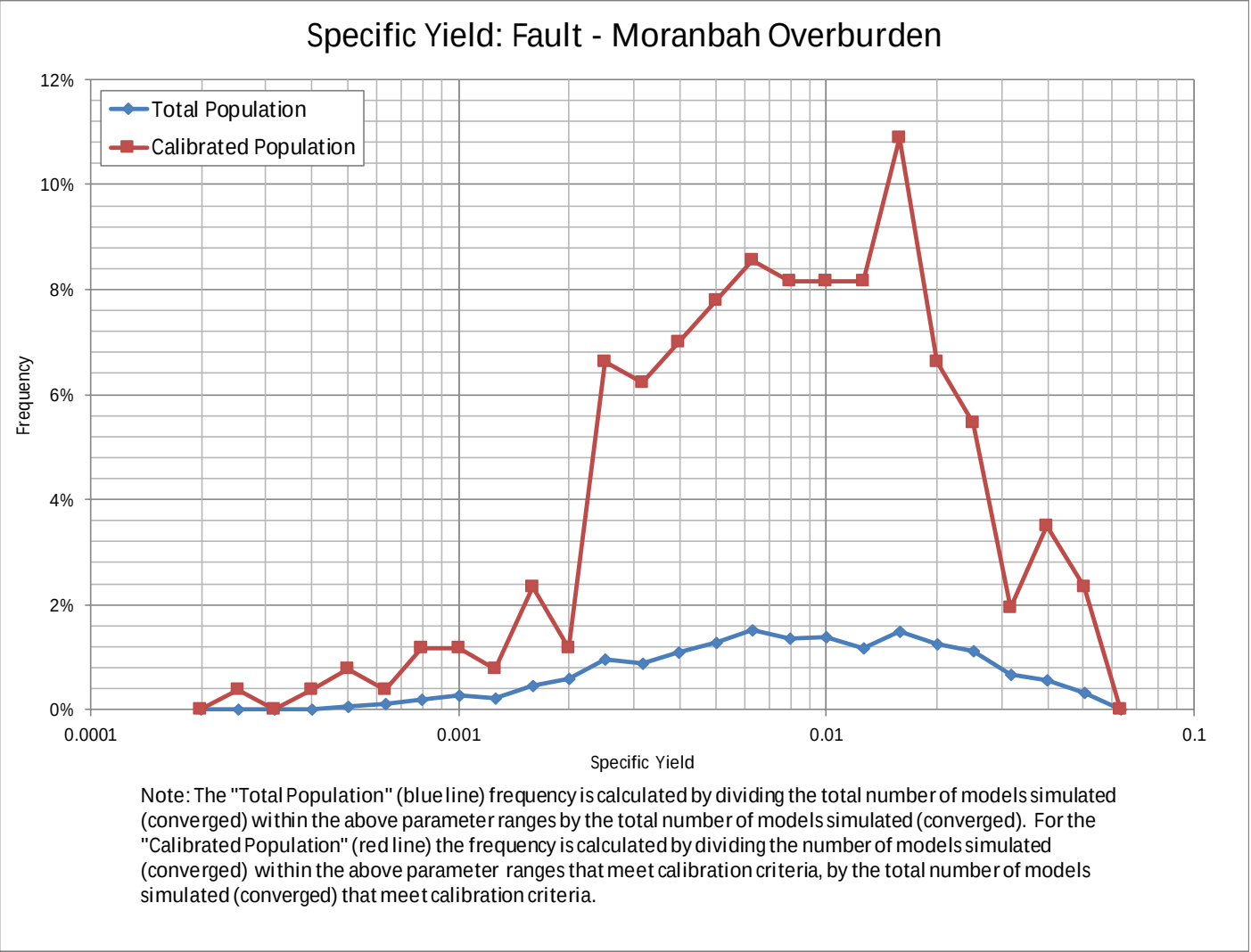




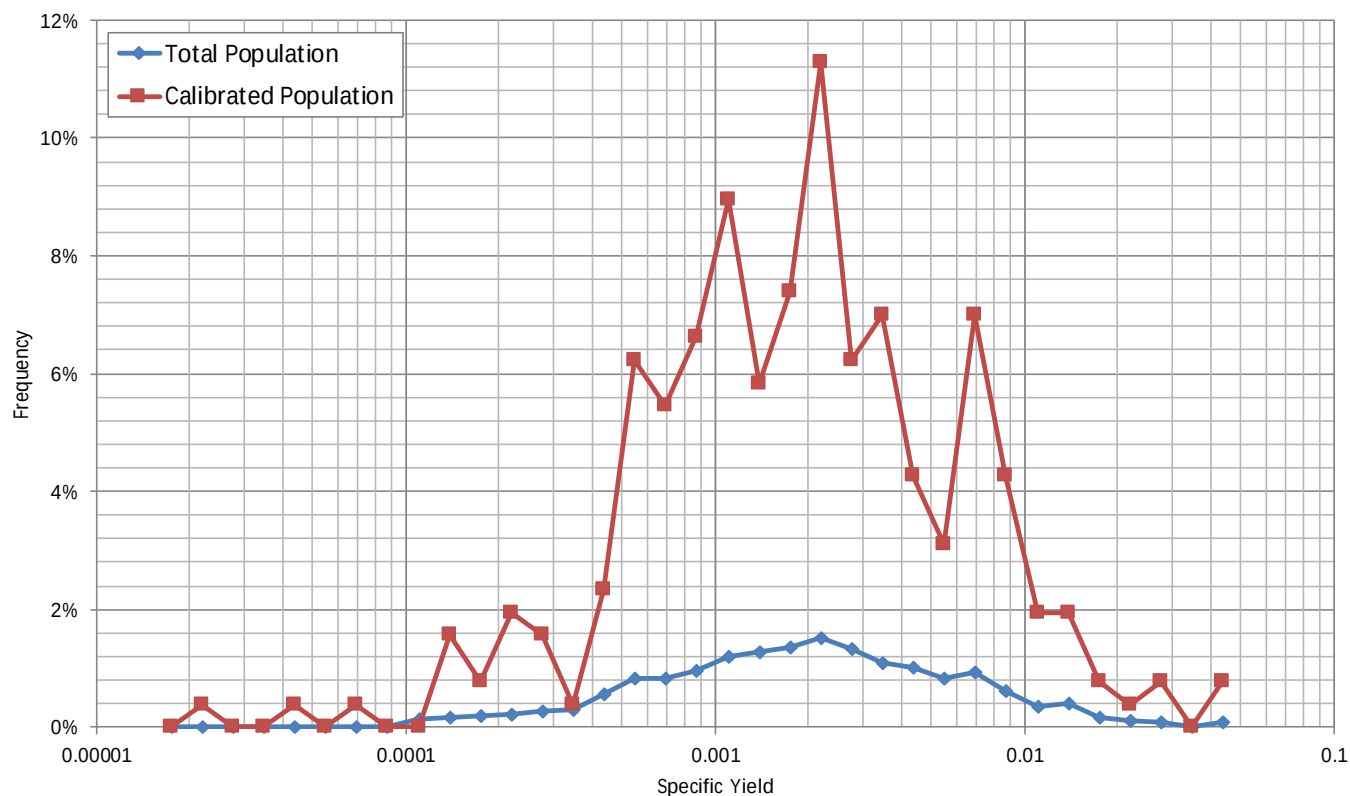






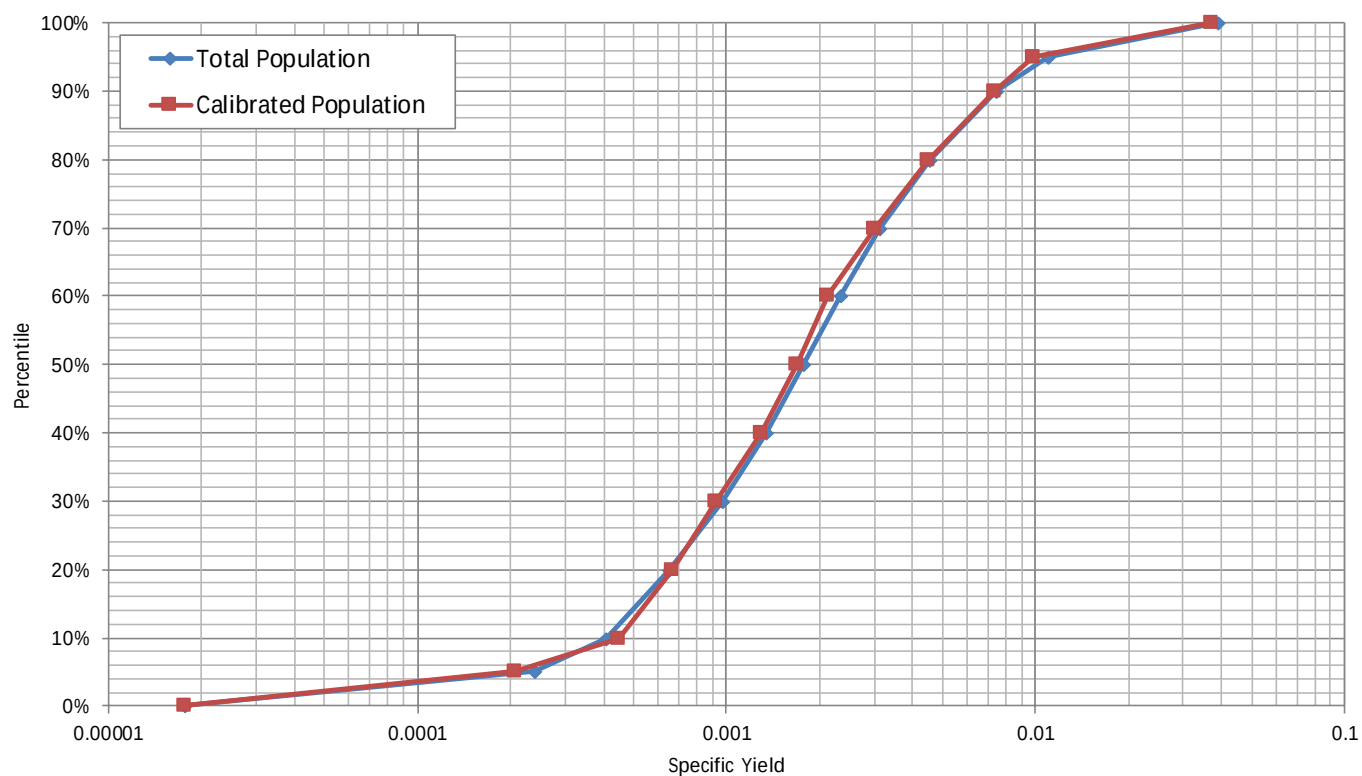


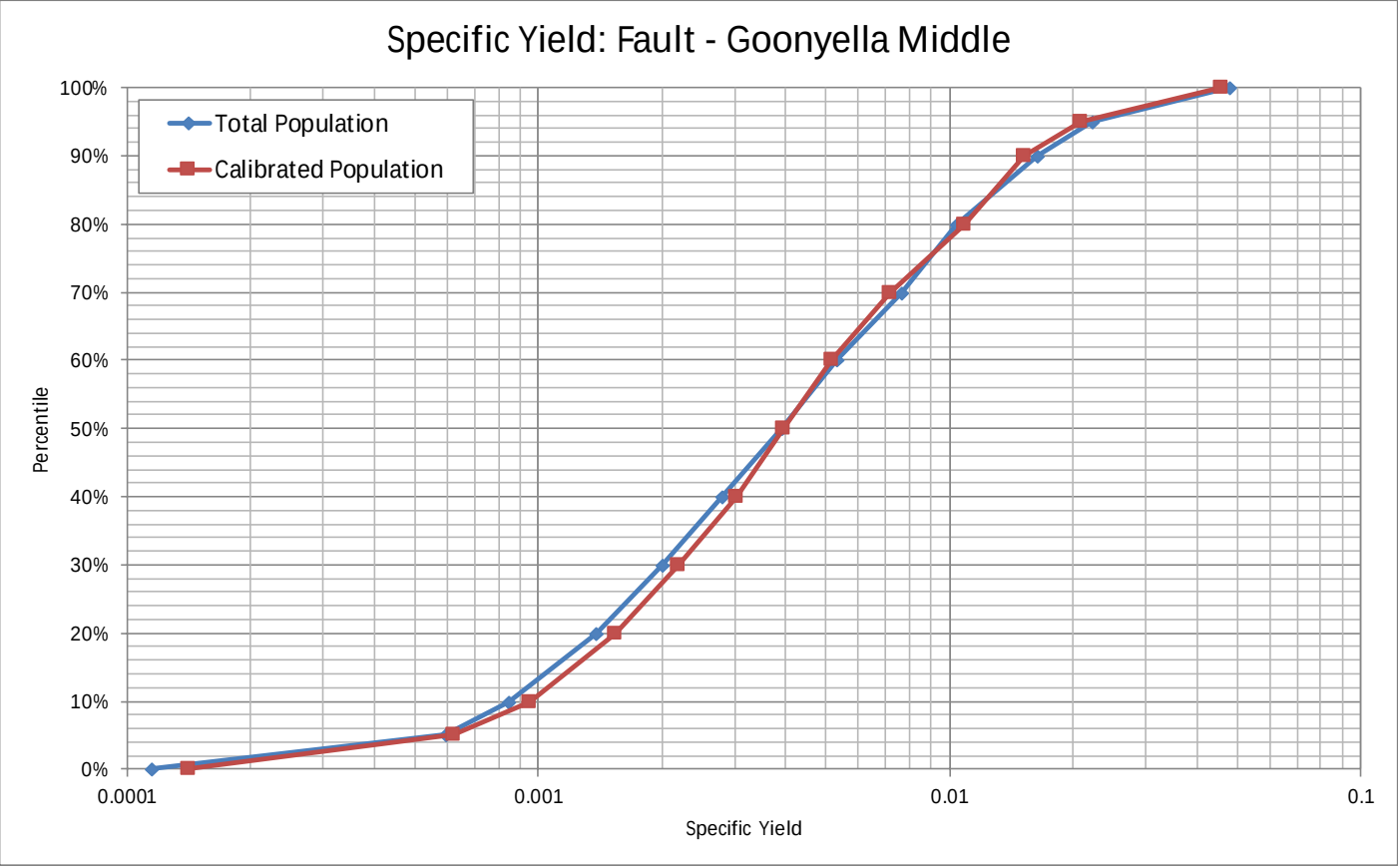
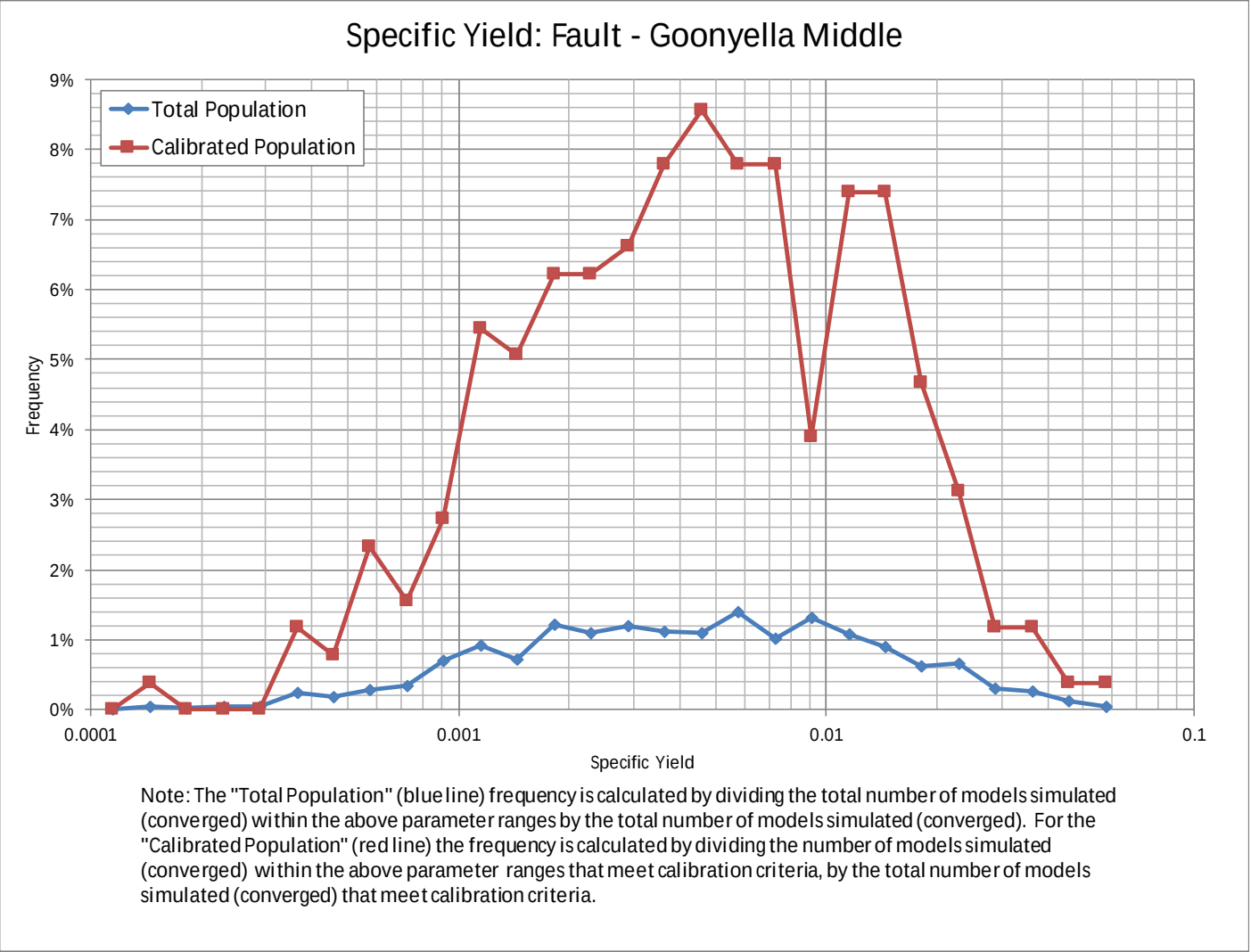
Specific Yield: Goonyella Middle

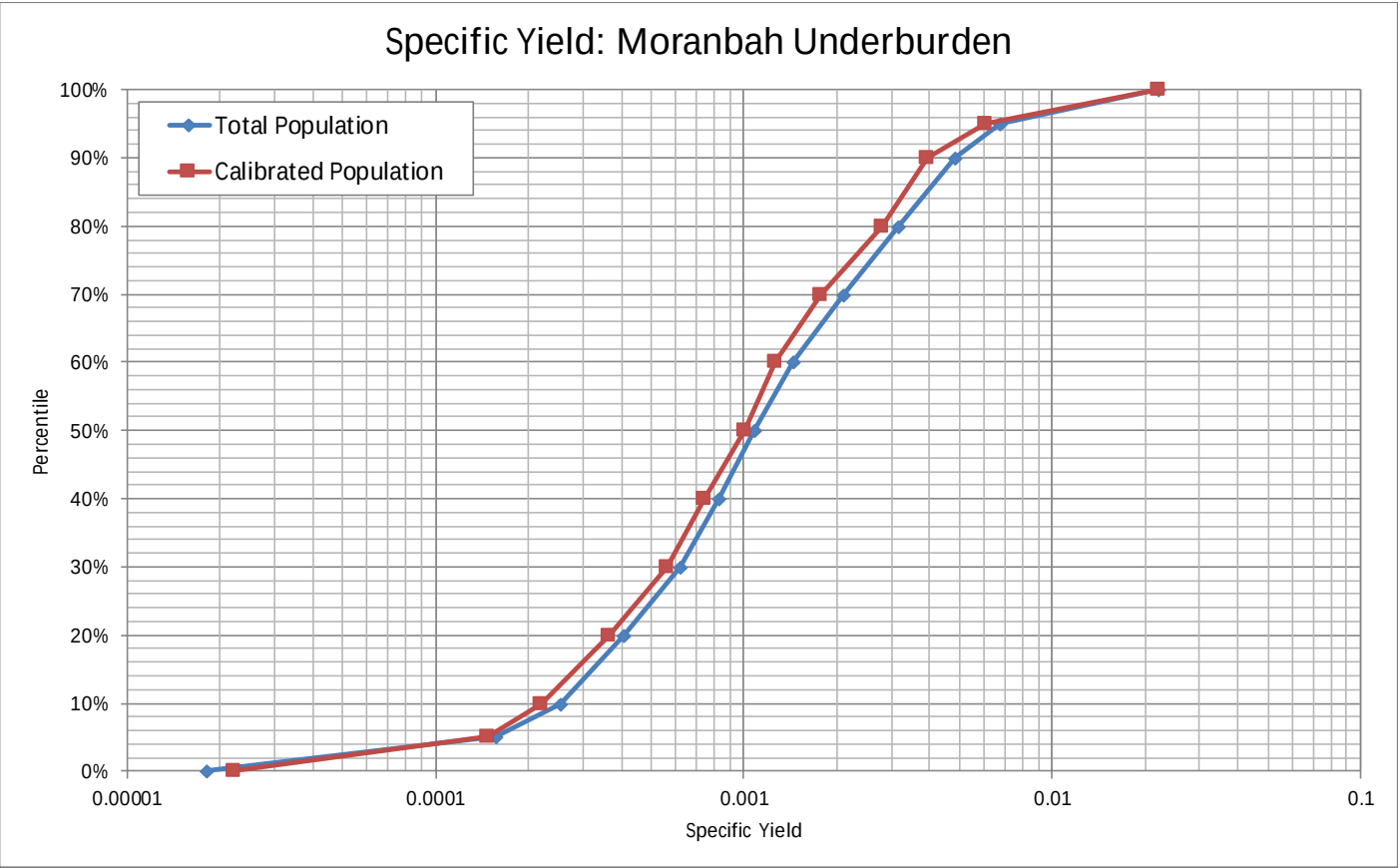
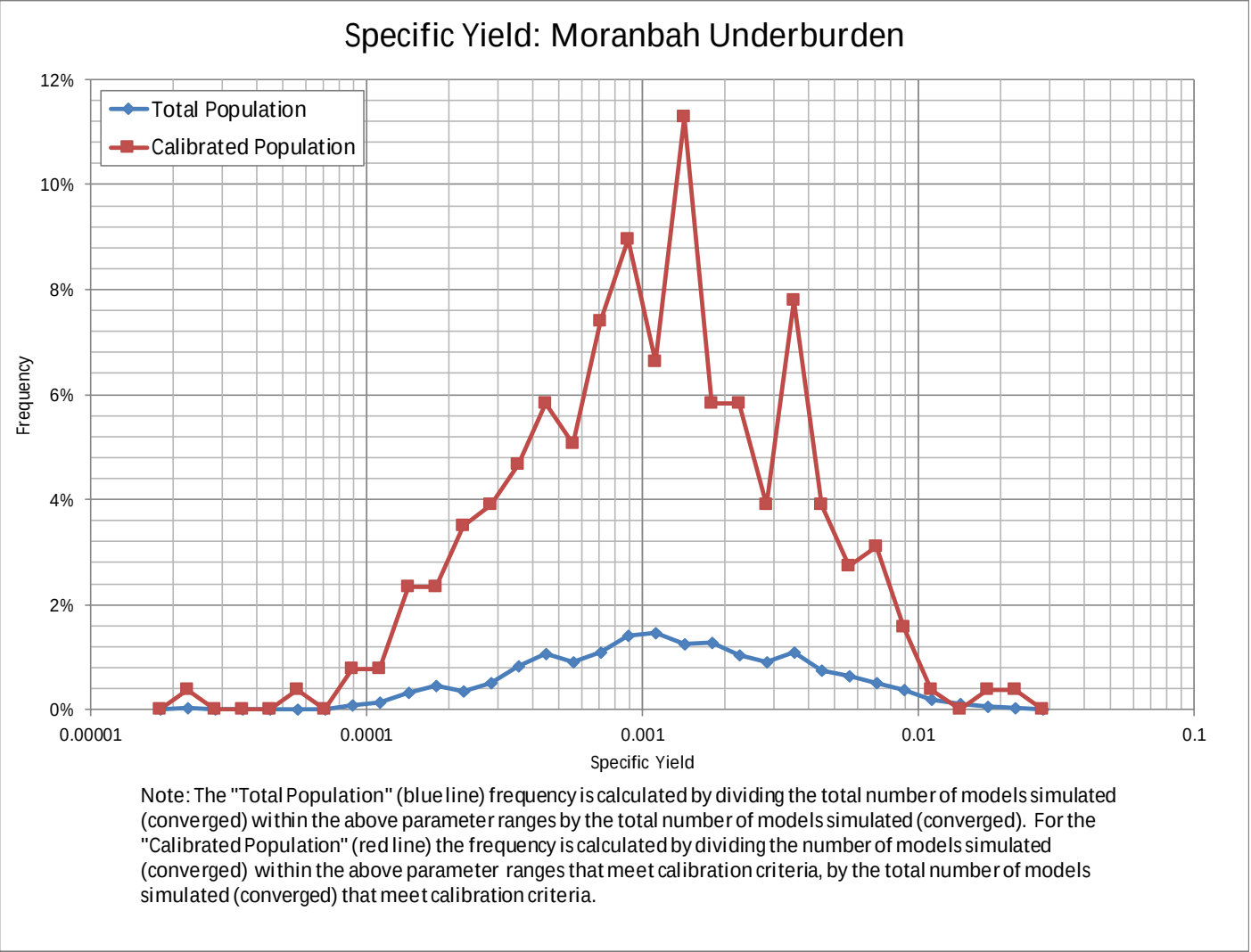


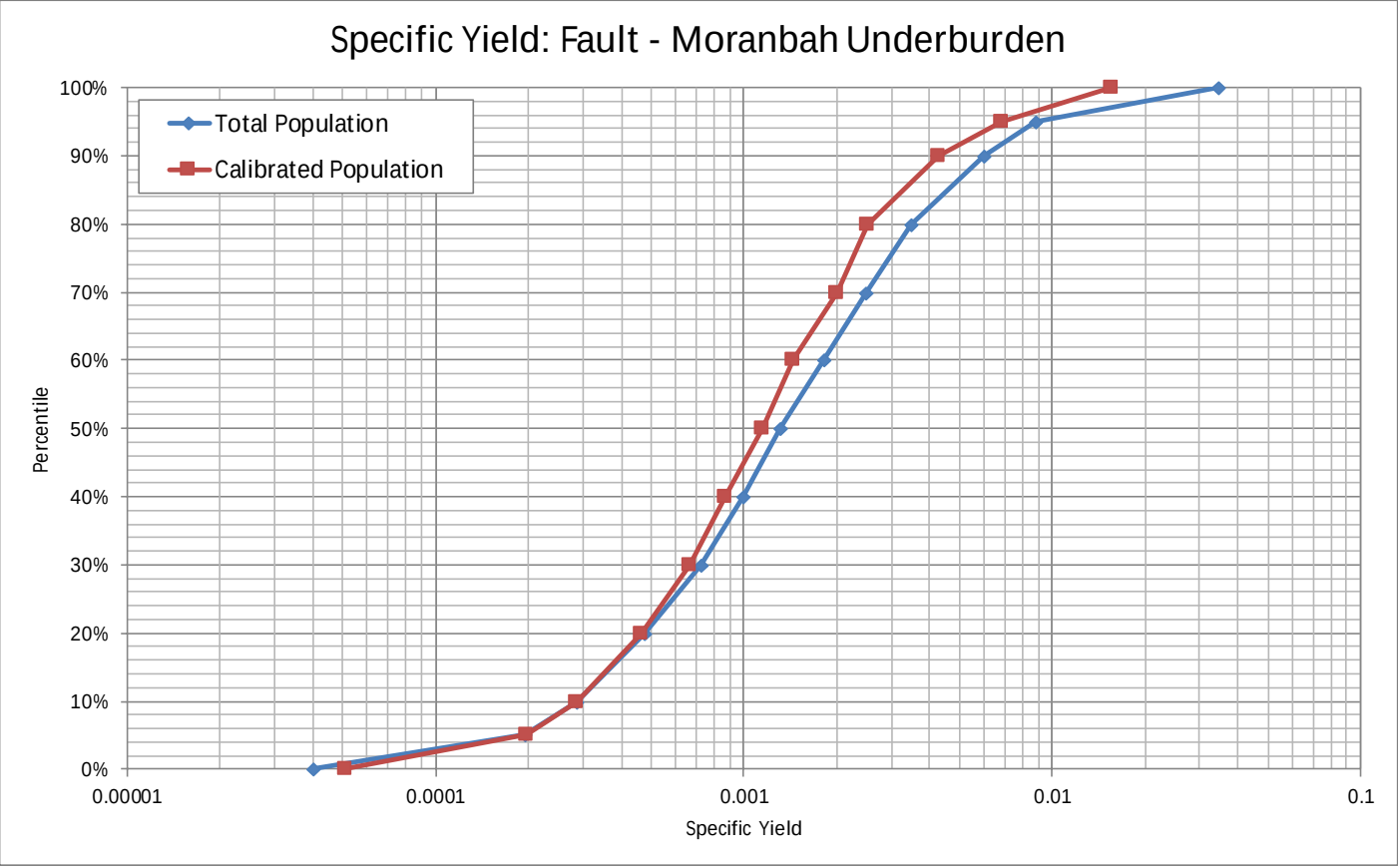
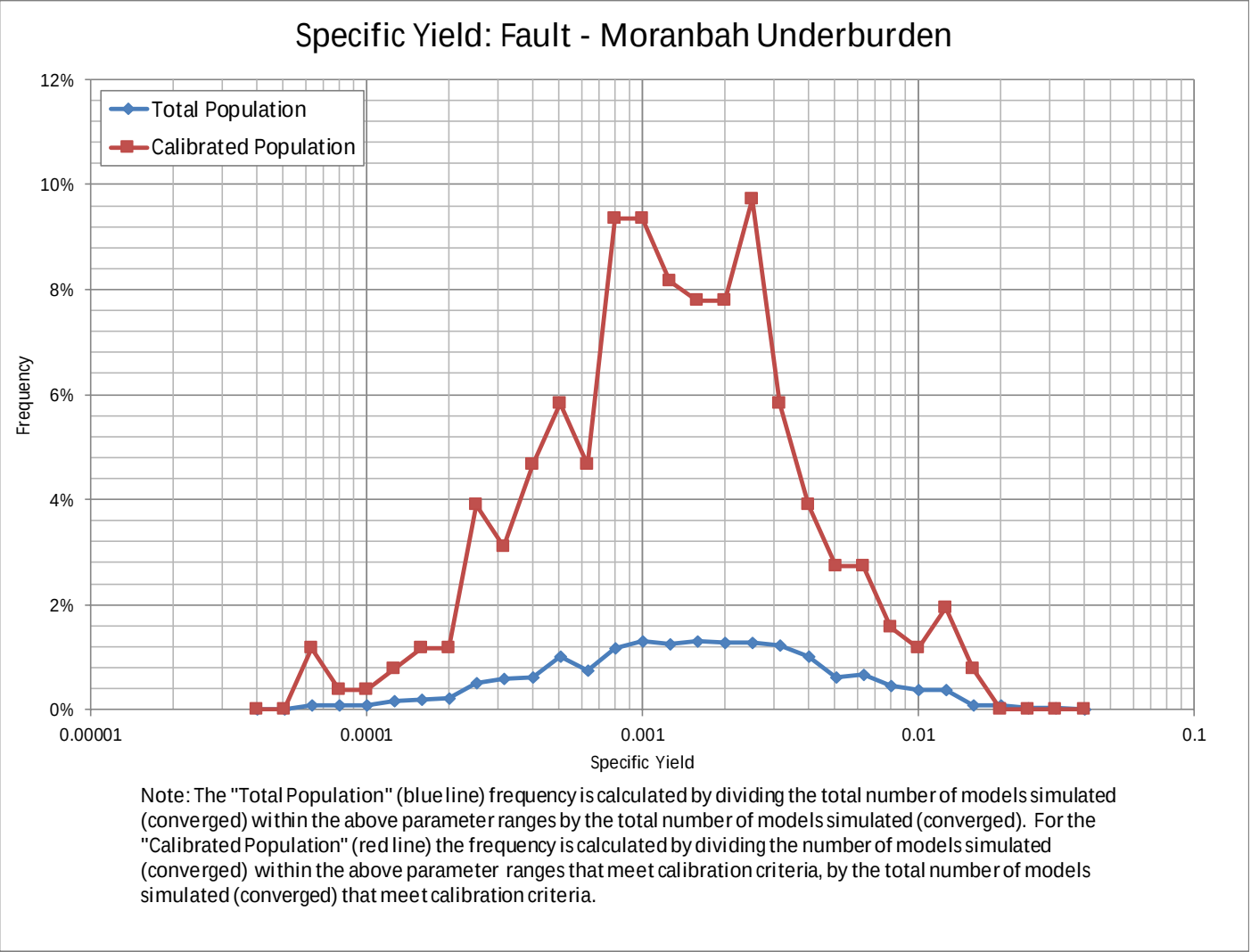
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Specific Yield: Goonyella Middle

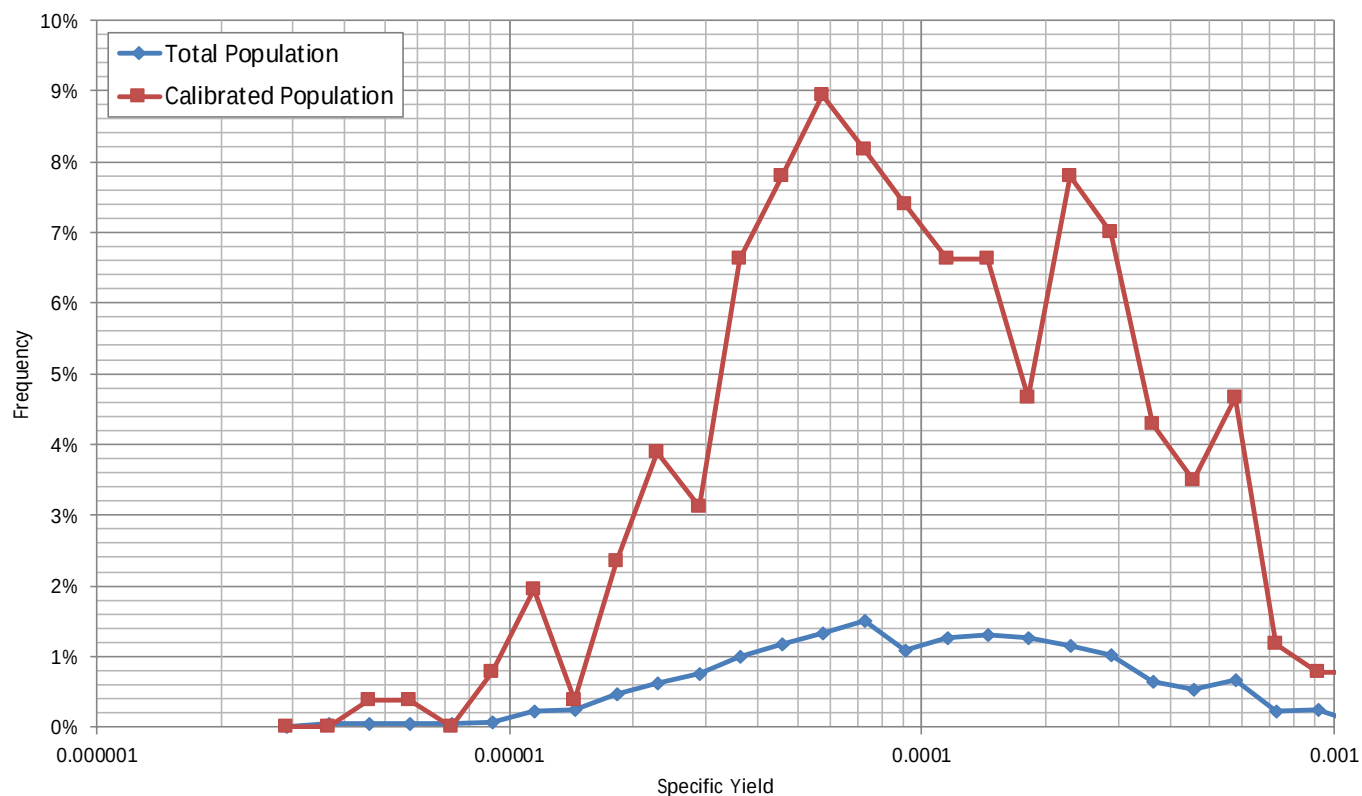






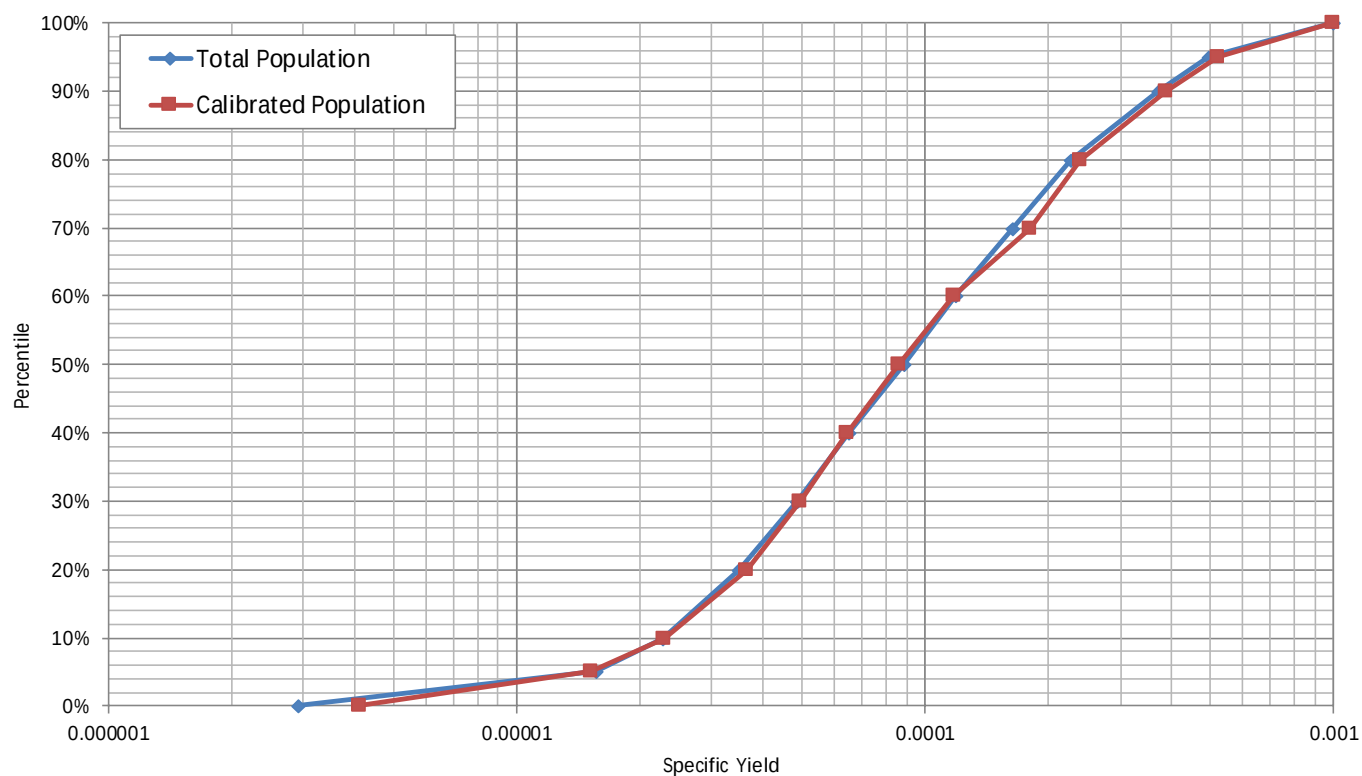


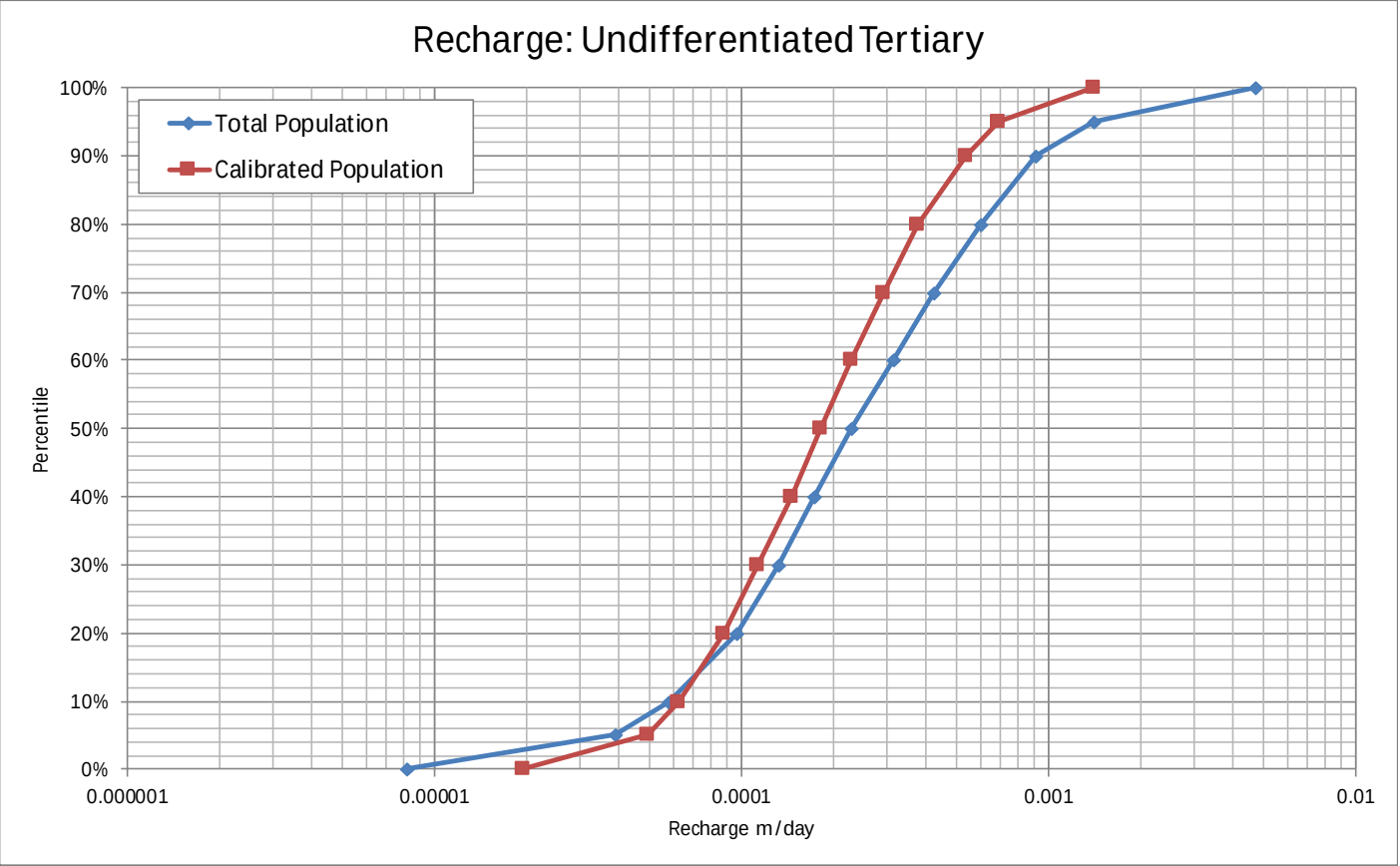
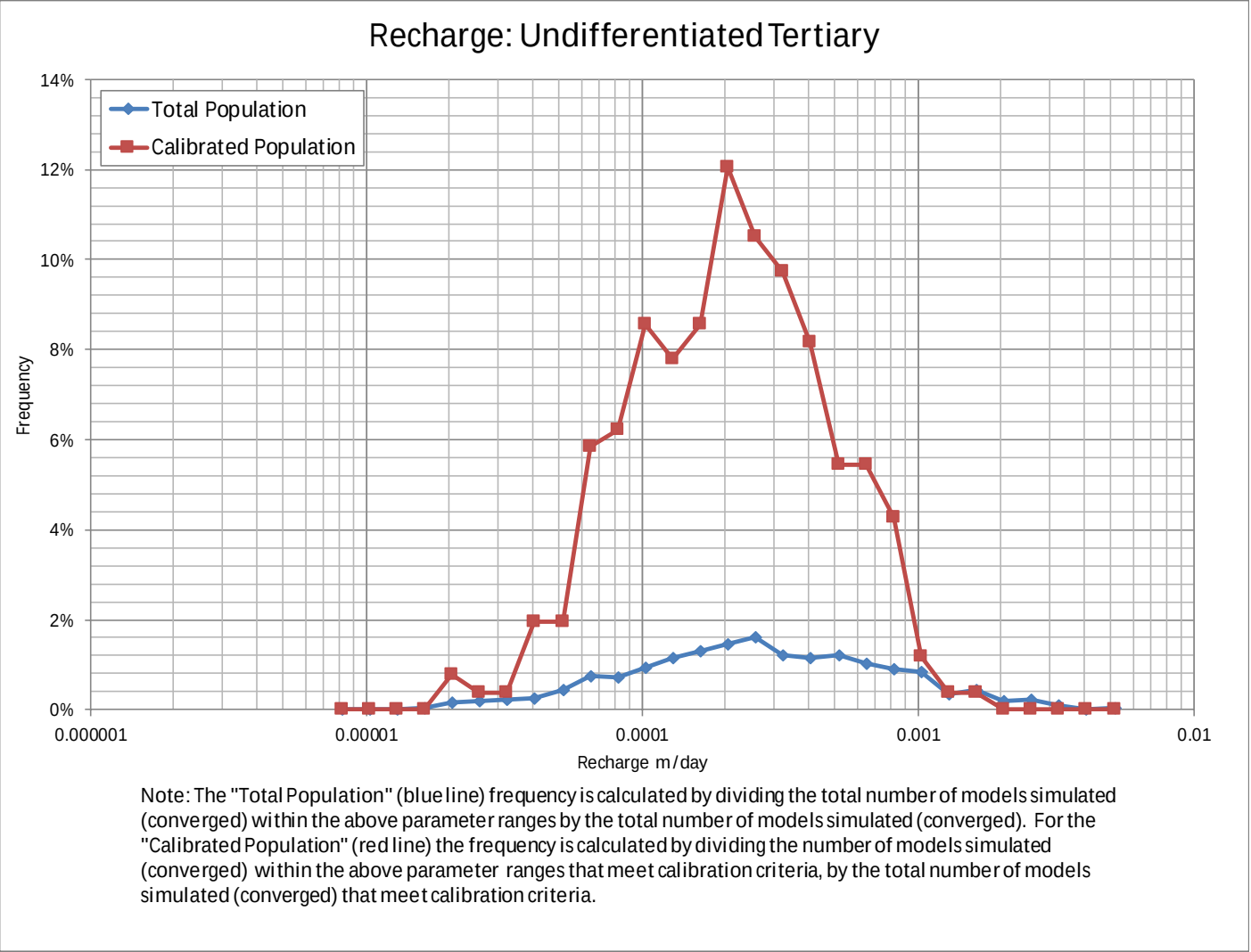
Specific Yield: Undivided Intrusives

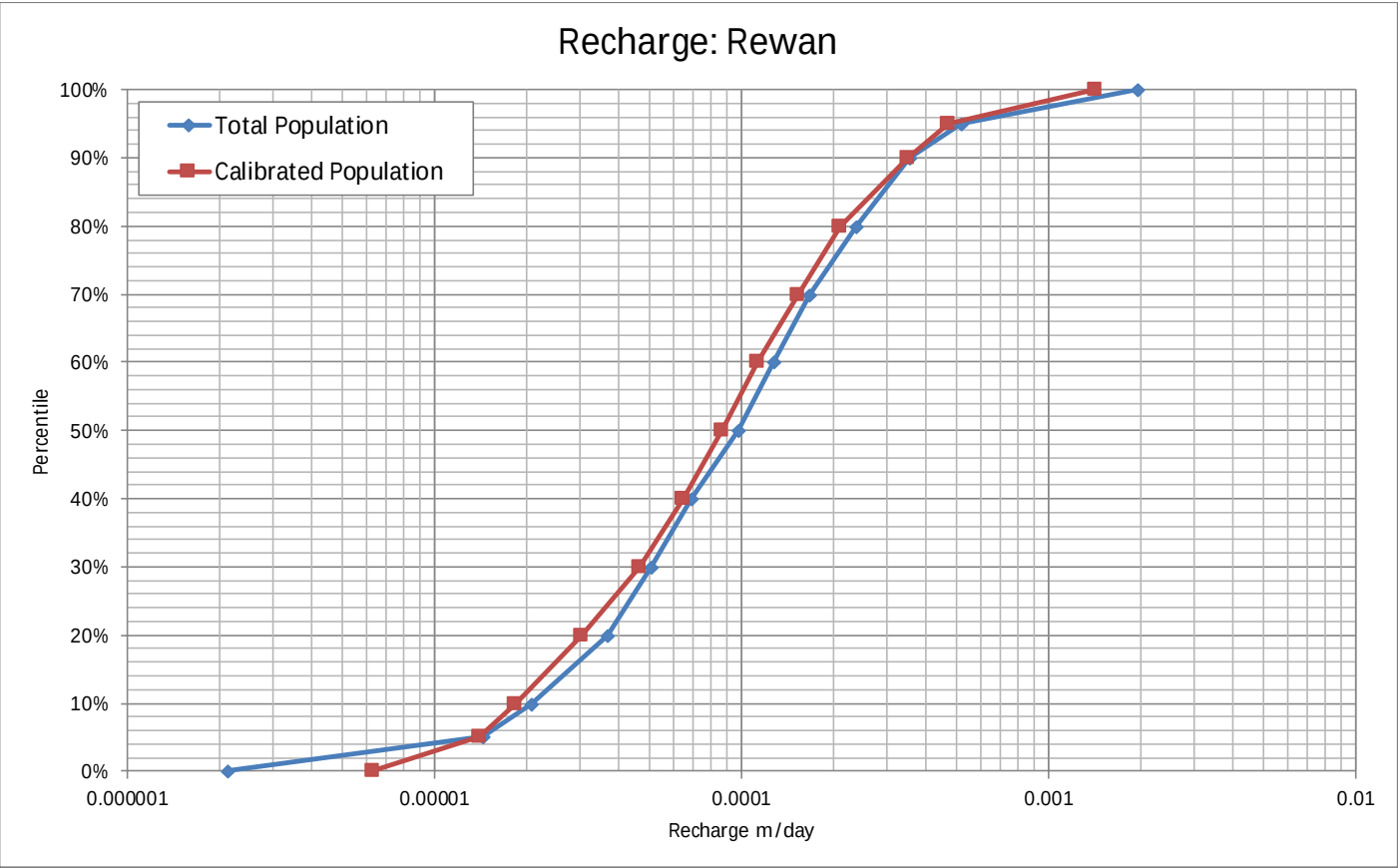
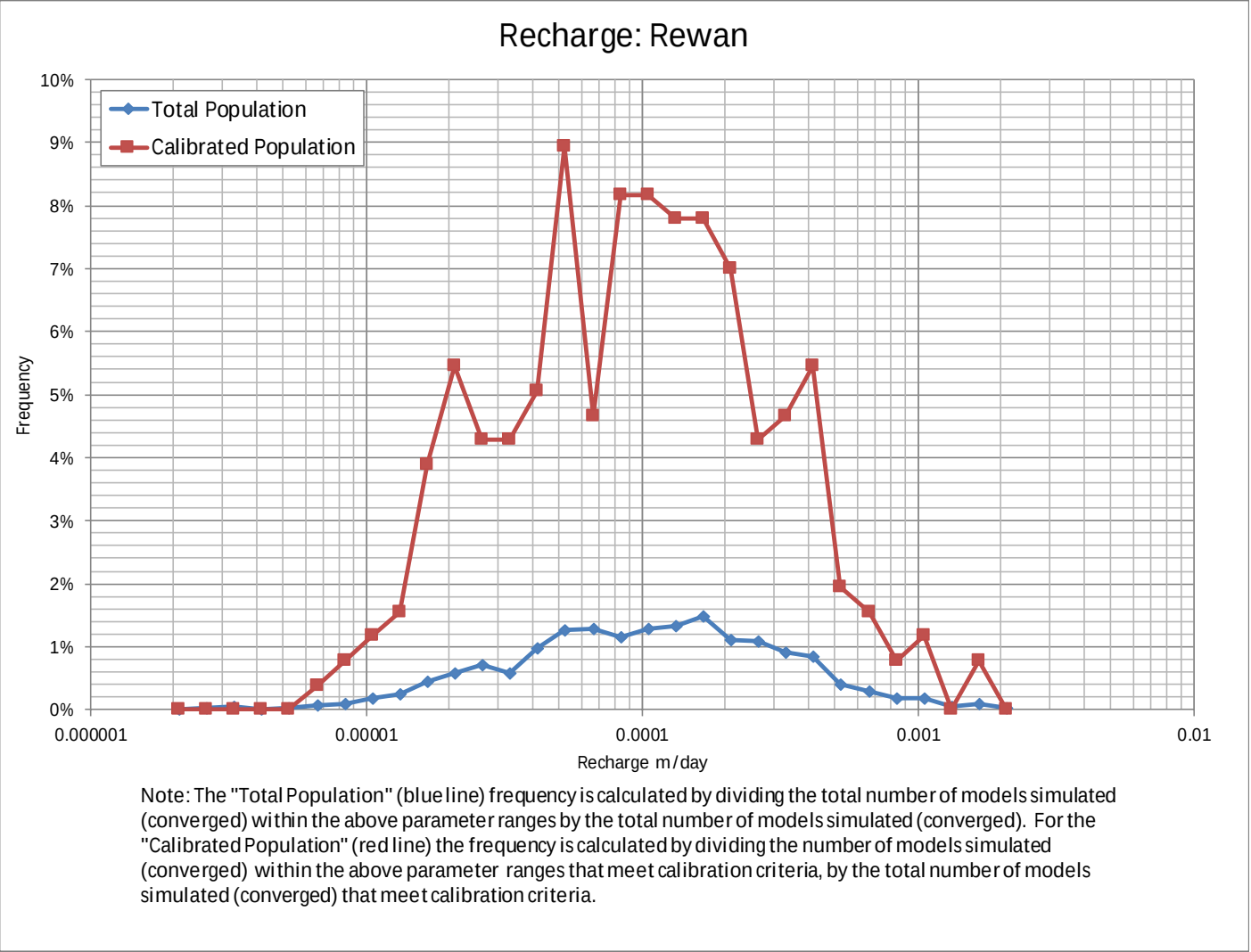


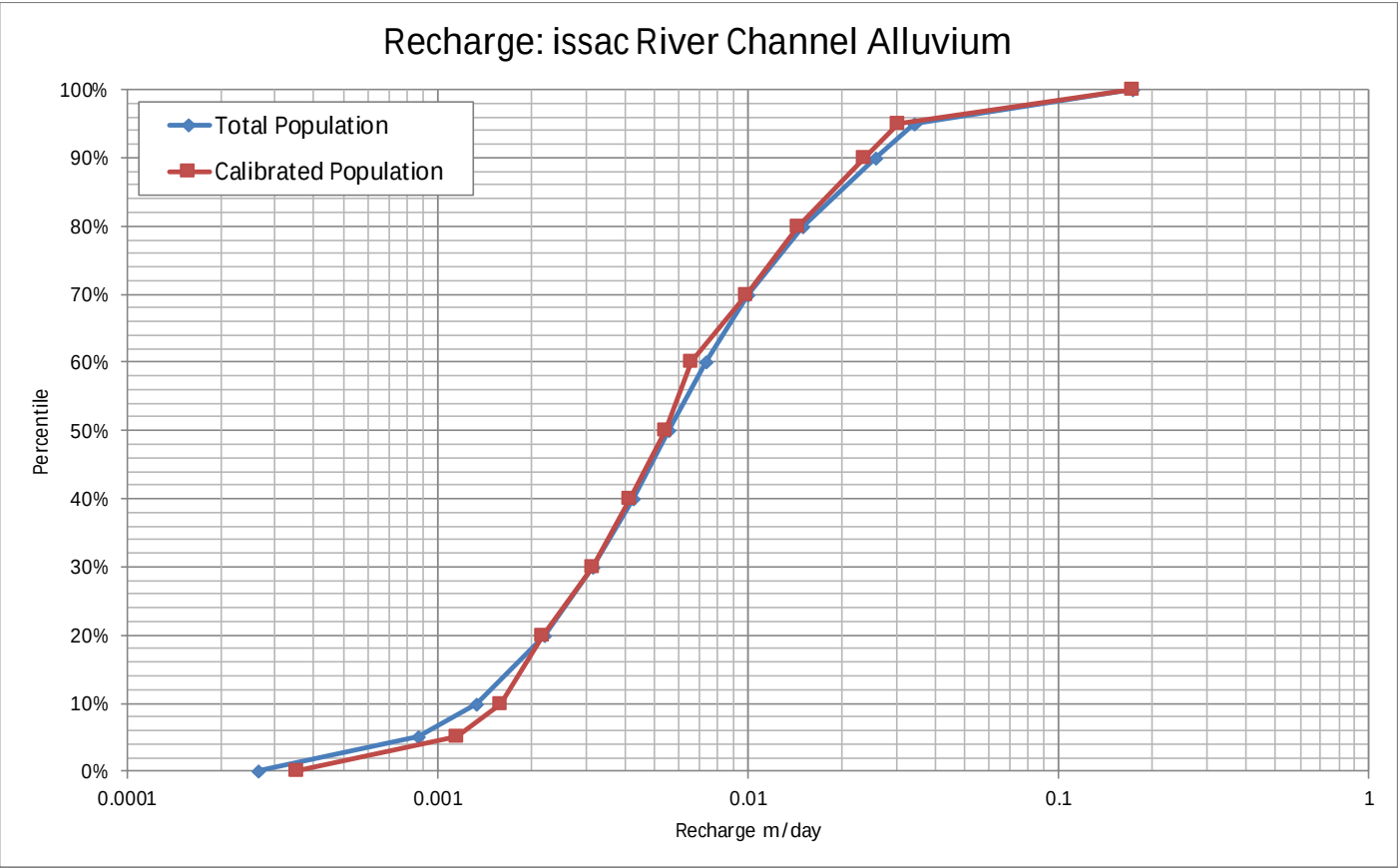
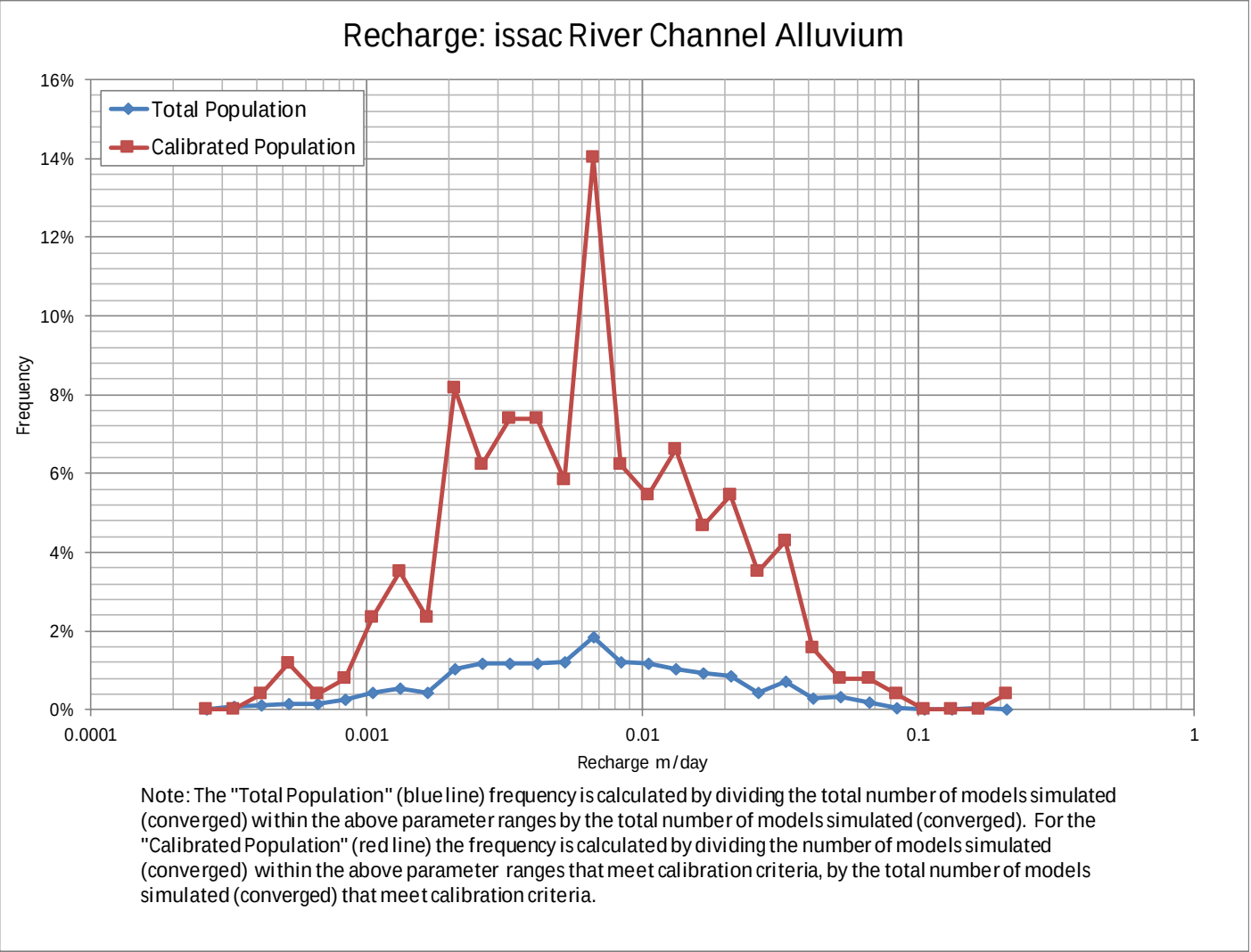
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Specific Yield: Undivided Intrusives

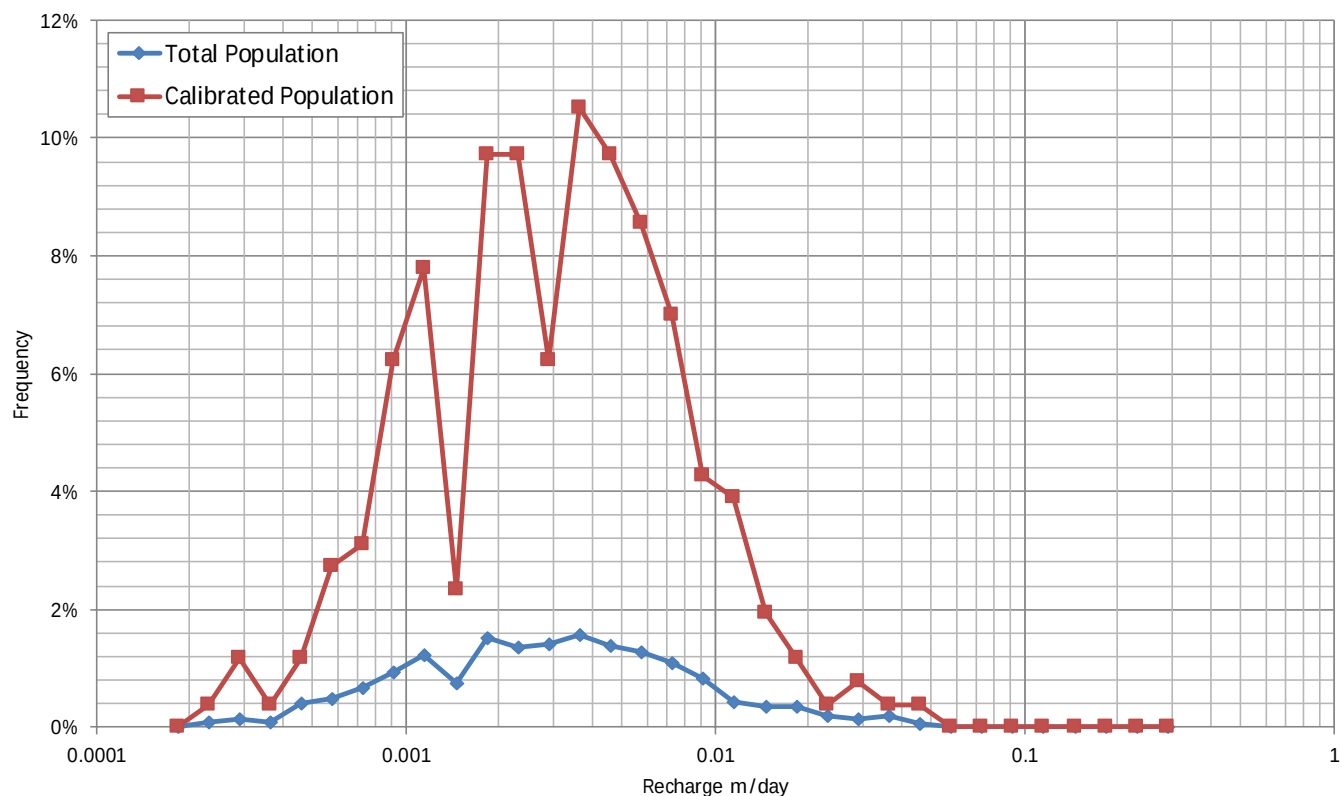






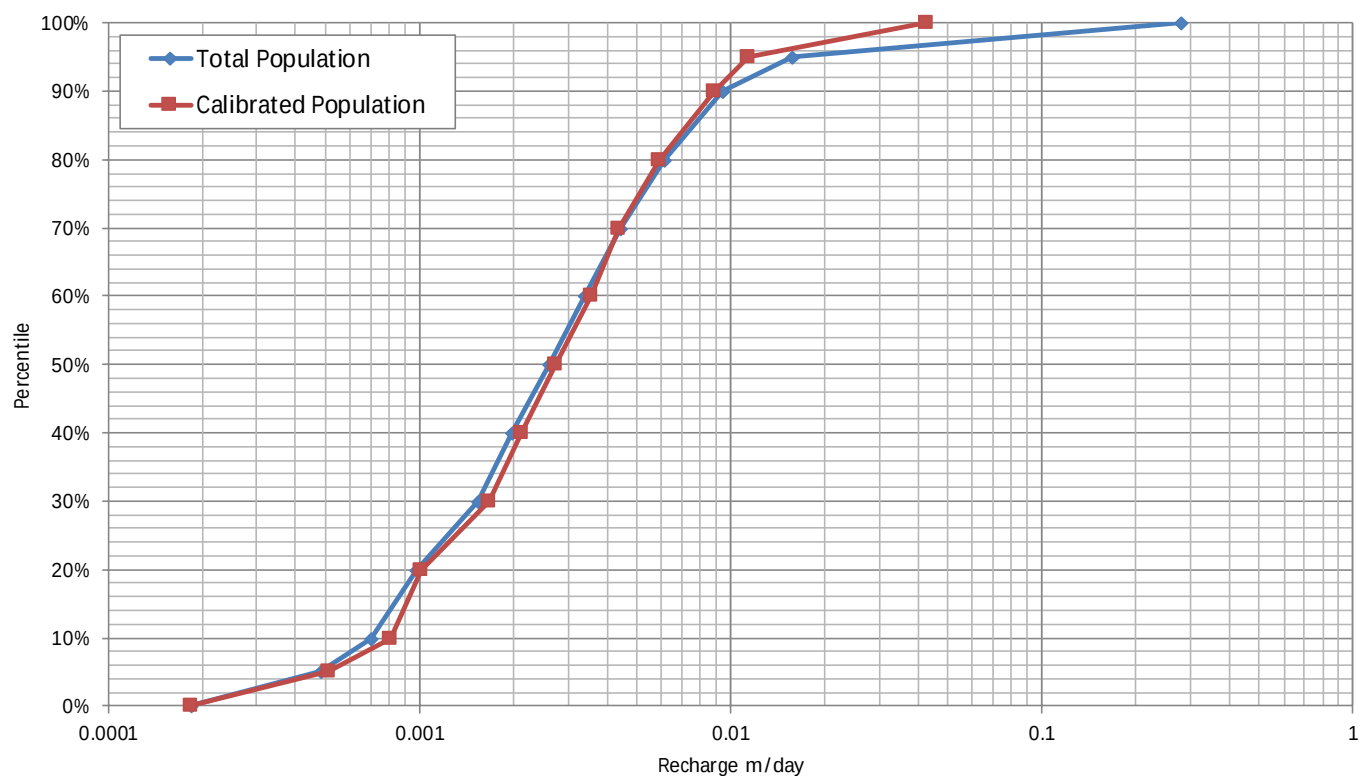


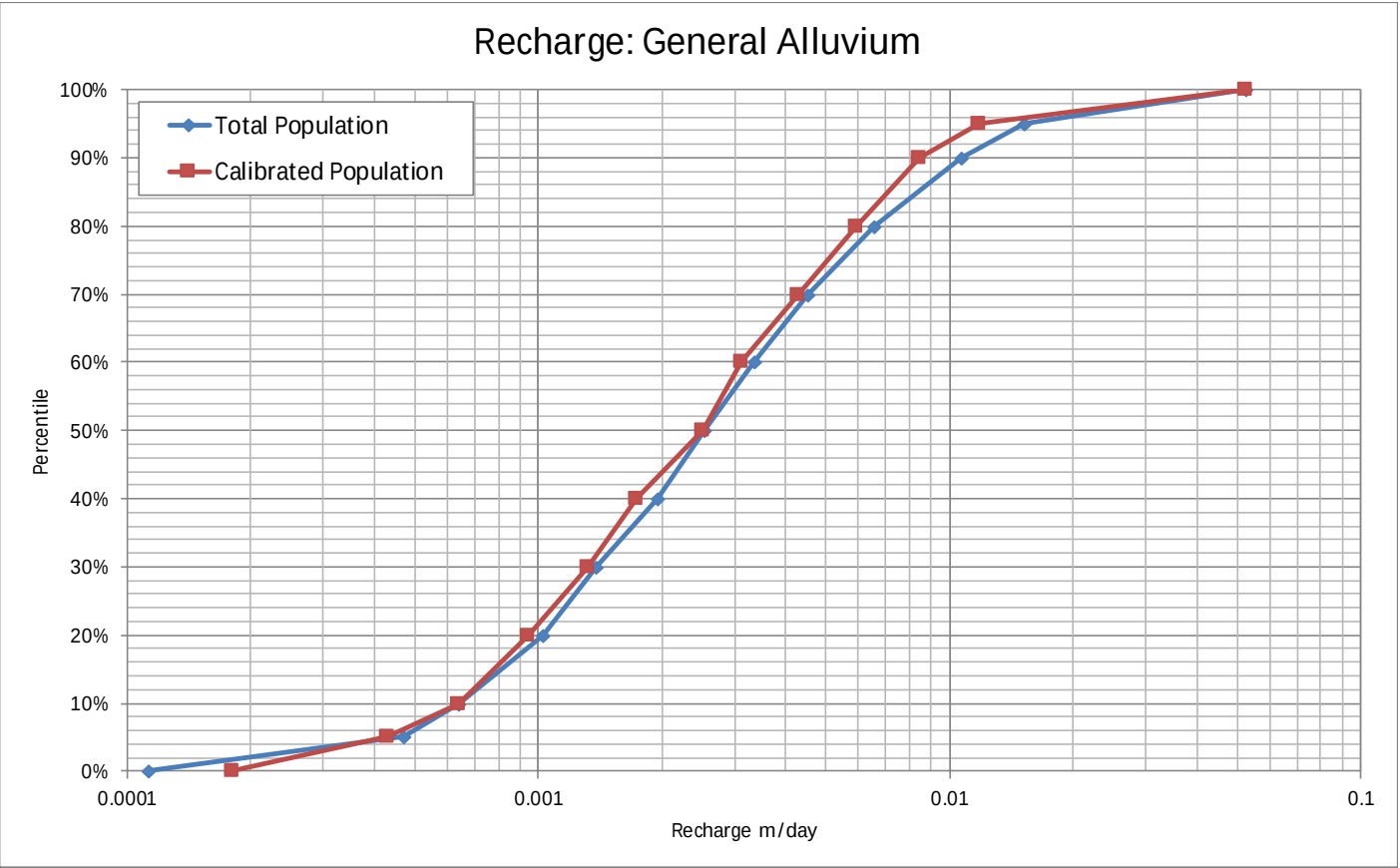
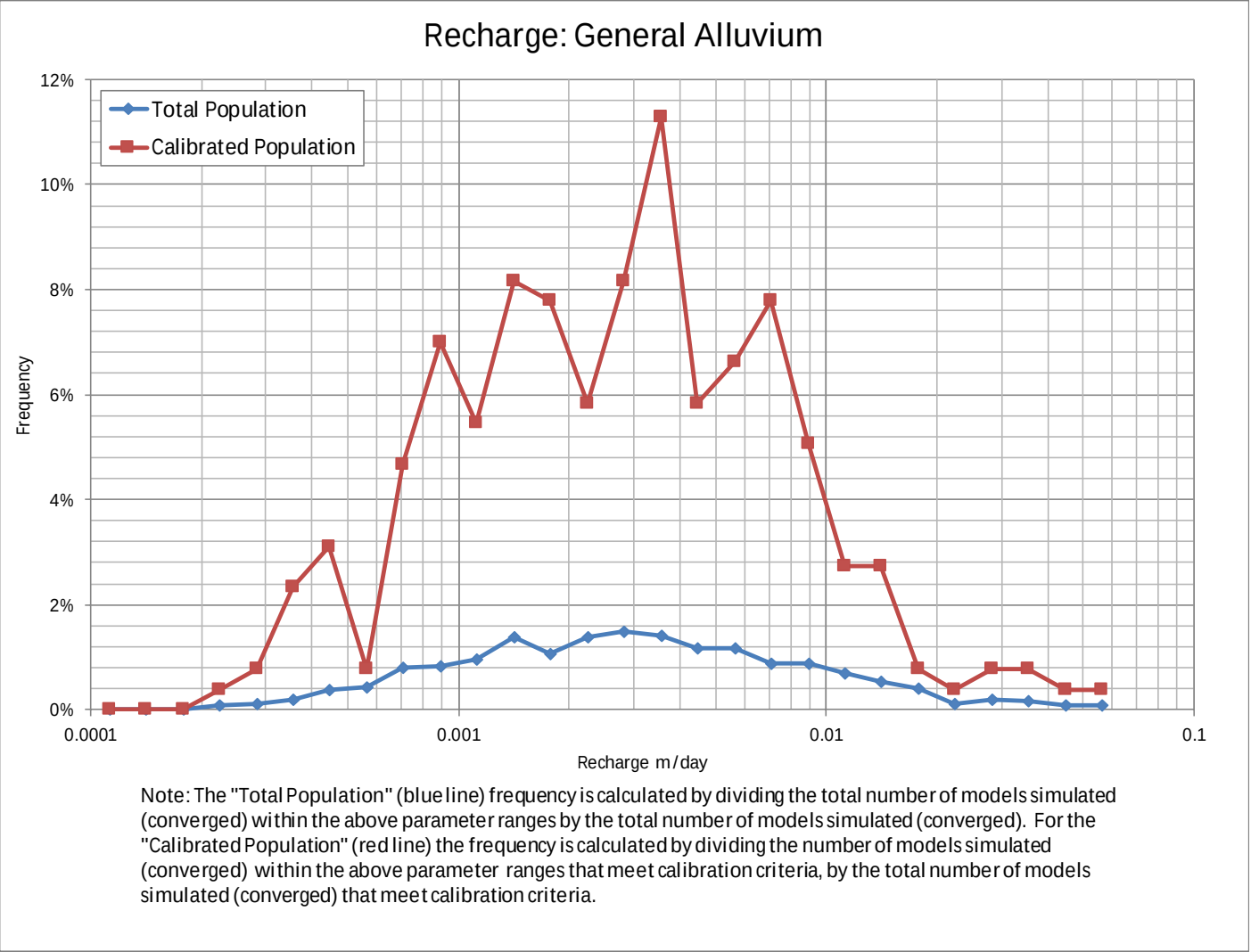
Recharge: Isaac River Alluvium

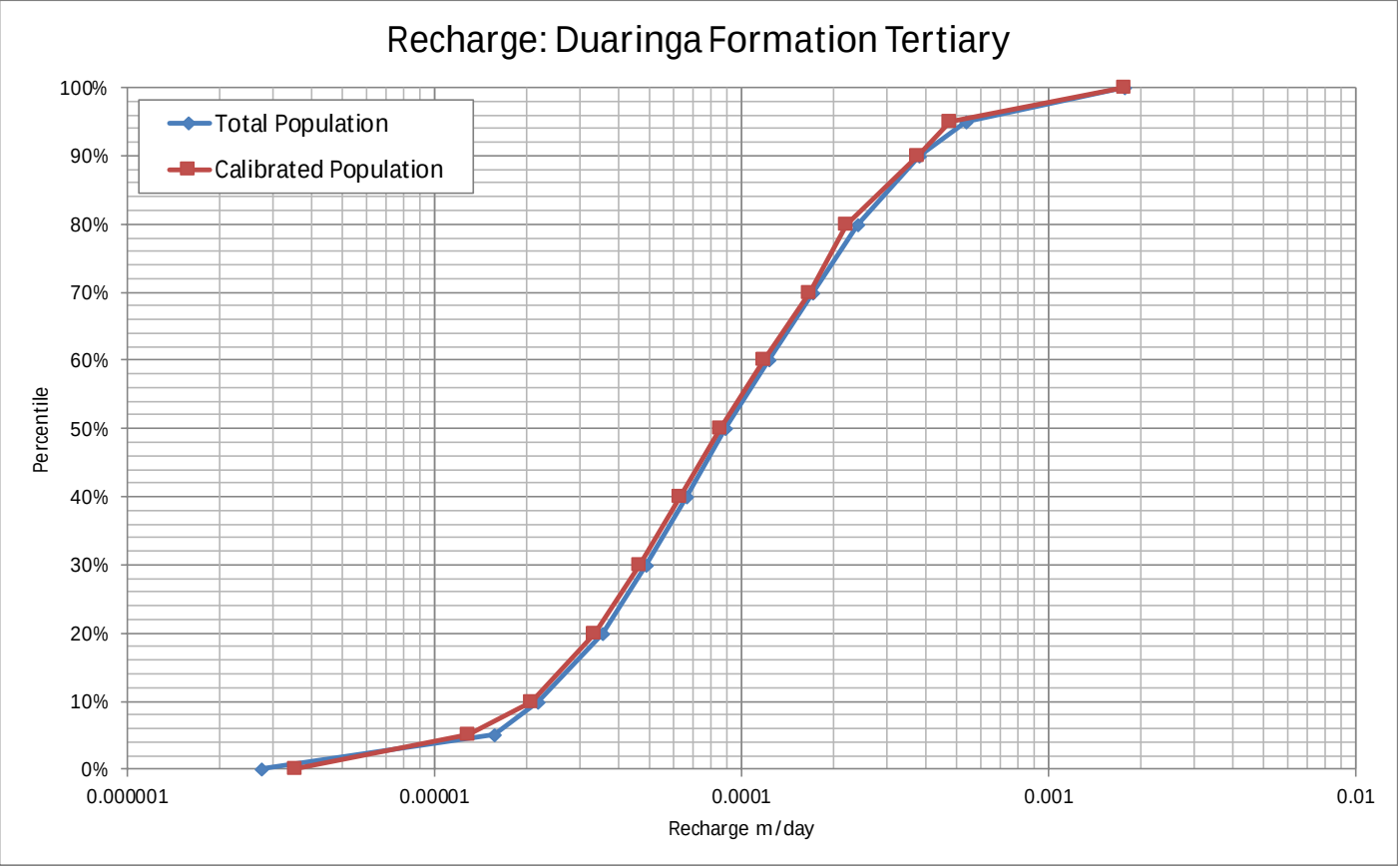
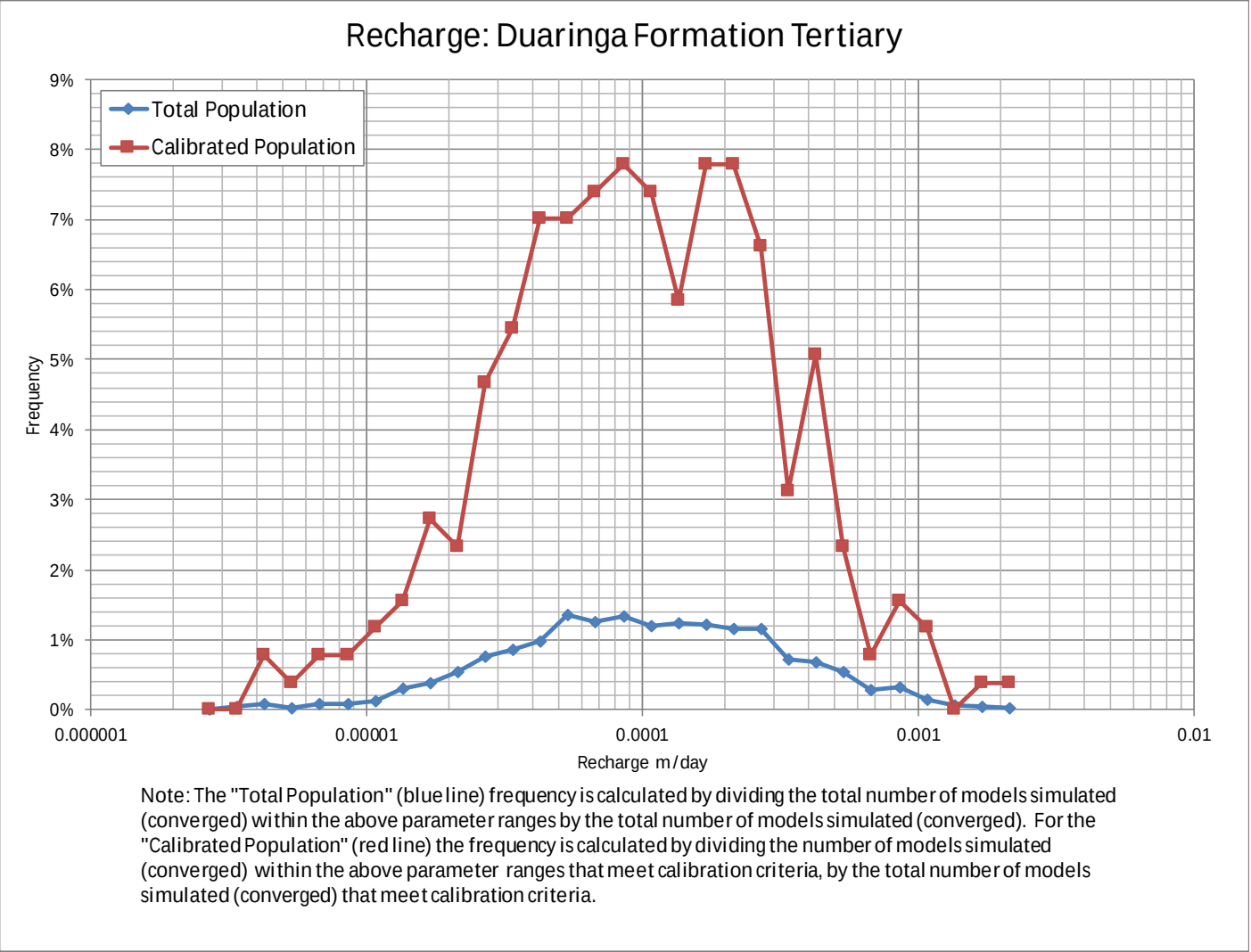


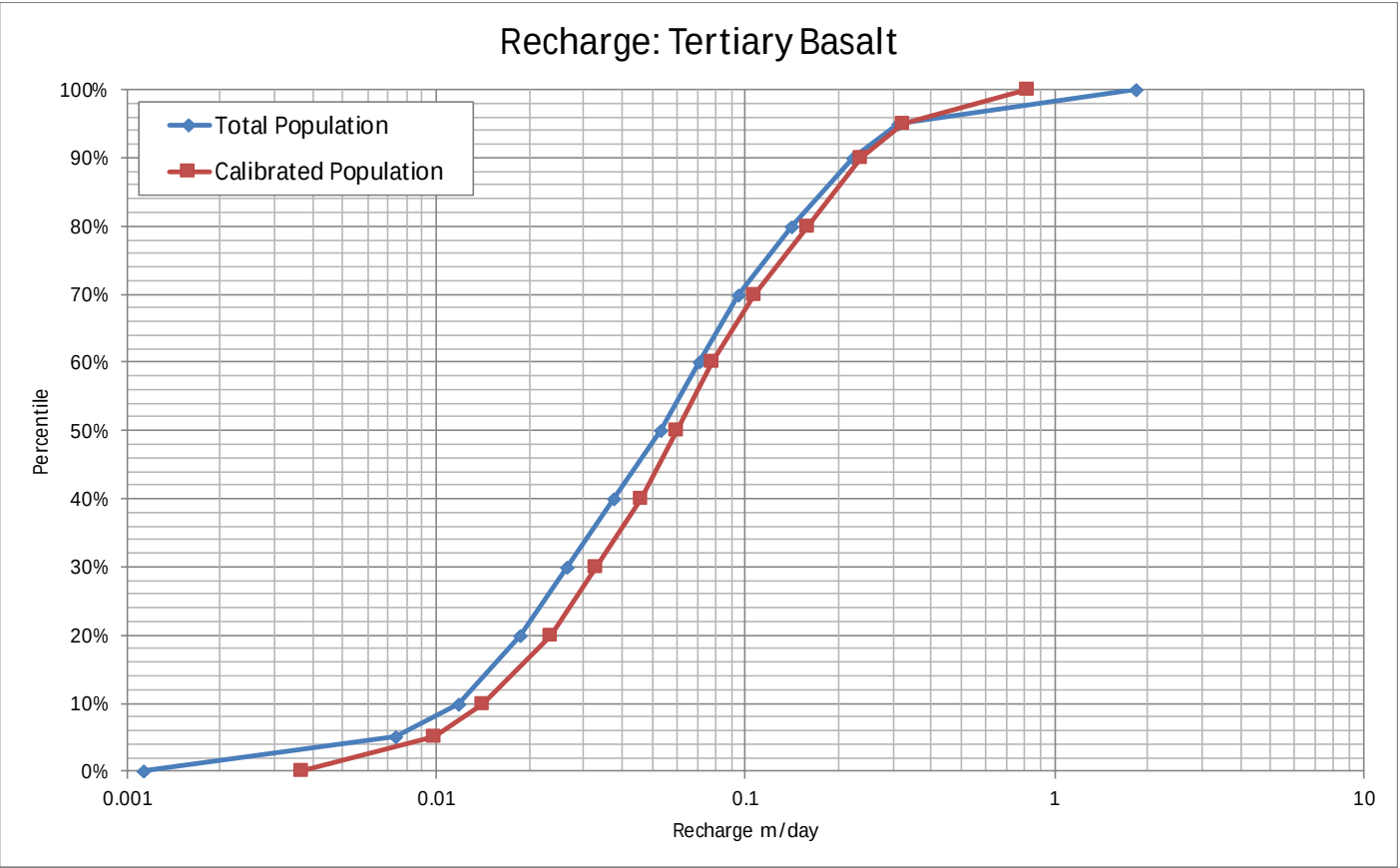
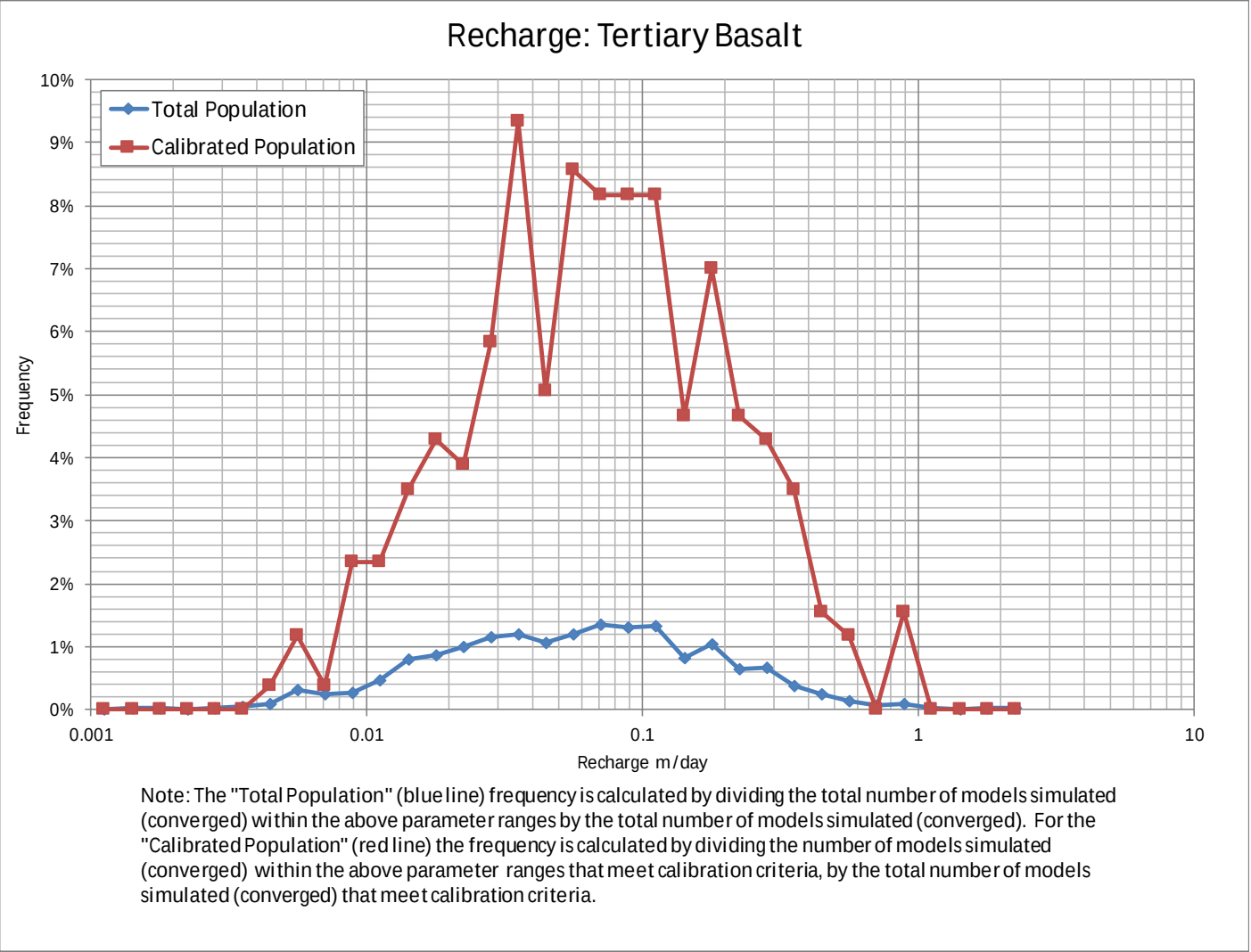
Note: The "Total Population" (blue line) frequency is calculated by dividing the total number of models simulated (converged) within the above parameter ranges by the total number of models simulated (converged). For the "Calibrated Population" (red line) the frequency is calculated by dividing the number of models simulated (converged) within the above parameter ranges that meet calibration criteria, by the total number of models simulated (converged) that meet calibration criteria.

Recharge: Isaac River Alluvium









APPENDIX F

Sensitivity Derivatives

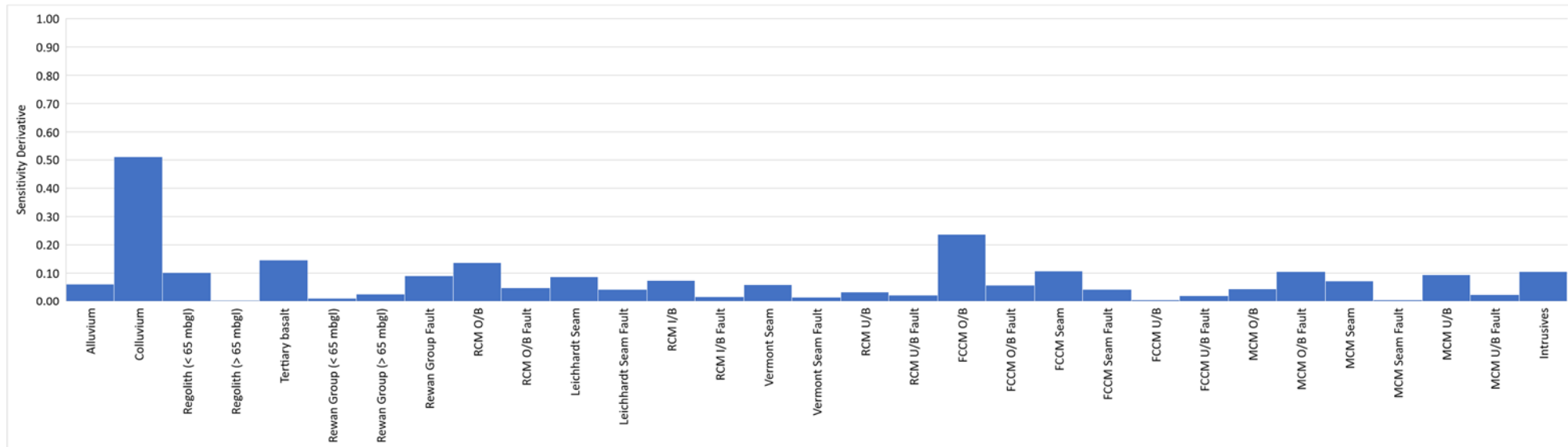


Figure F-1 Impacted Alluvium / Colluvium Drawdown Area (Layer 1) Sensitivity Derivatives – Horizontal Hydraulic Conductivity

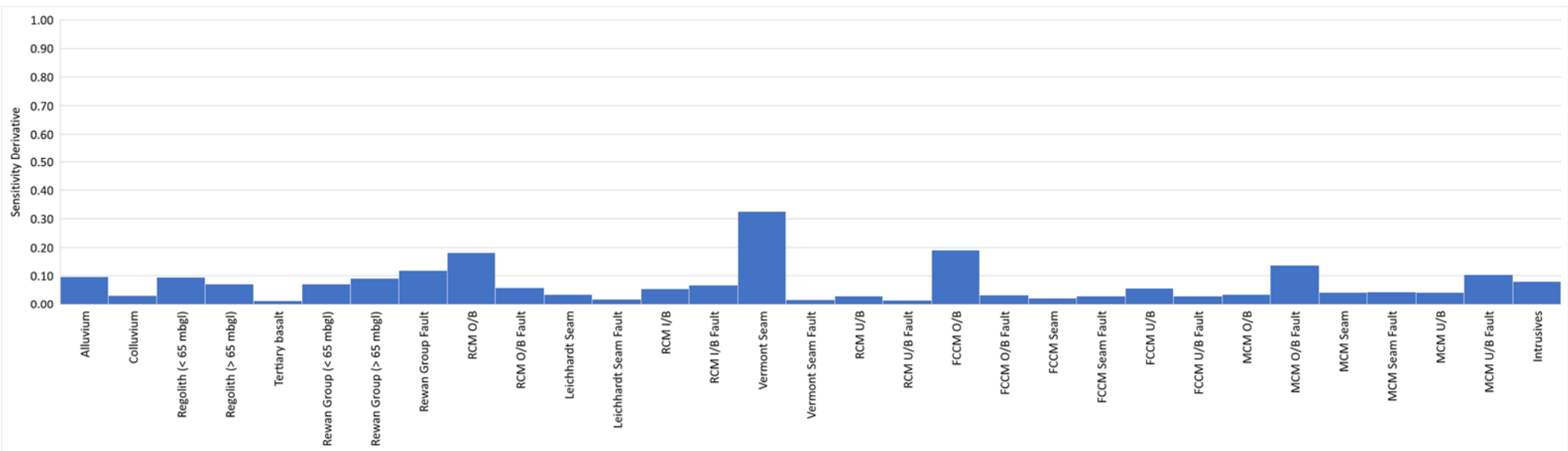


Figure F-2 Impacted Coal Seam Drawdown Area (Layer 7) Sensitivity Derivatives – Horizontal Hydraulic Conductivity

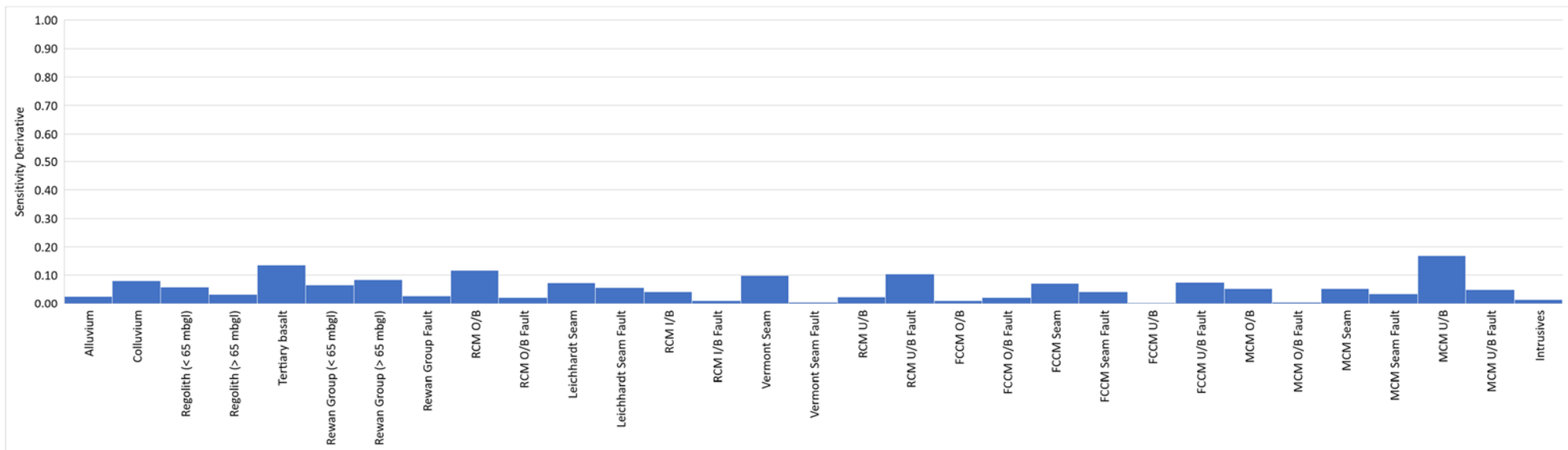


Figure F-3 Project Inflow Sensitivity Derivatives – Horizontal Hydraulic Conductivity

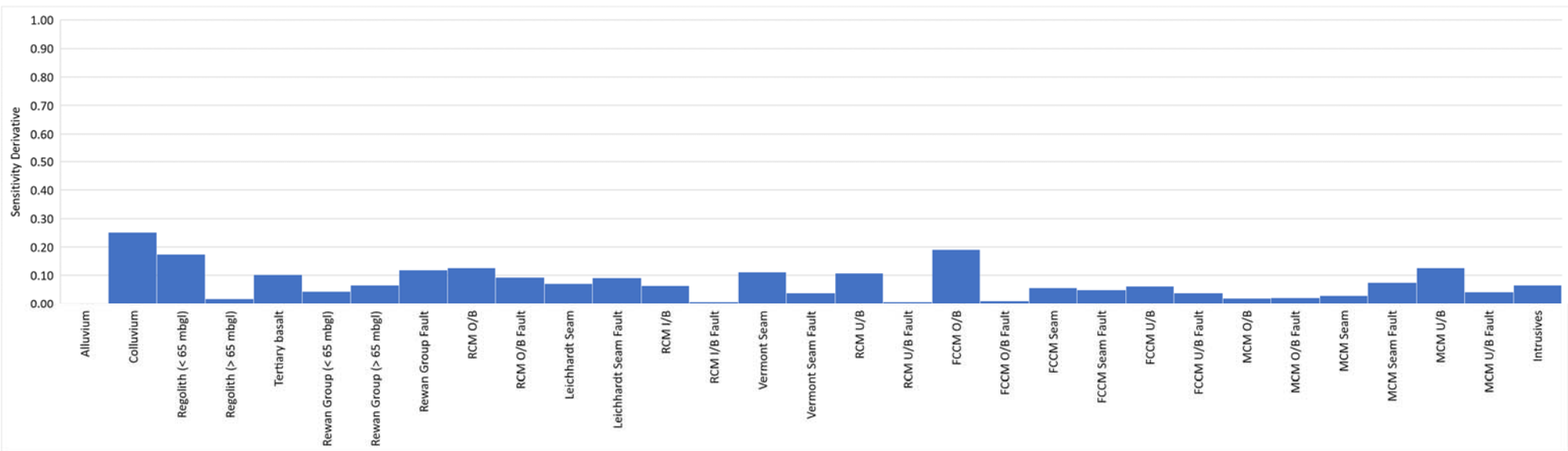


Figure F-4 Alluvial Drawdown Magnitude Sensitivity Derivatives – Horizontal Hydraulic Conductivity

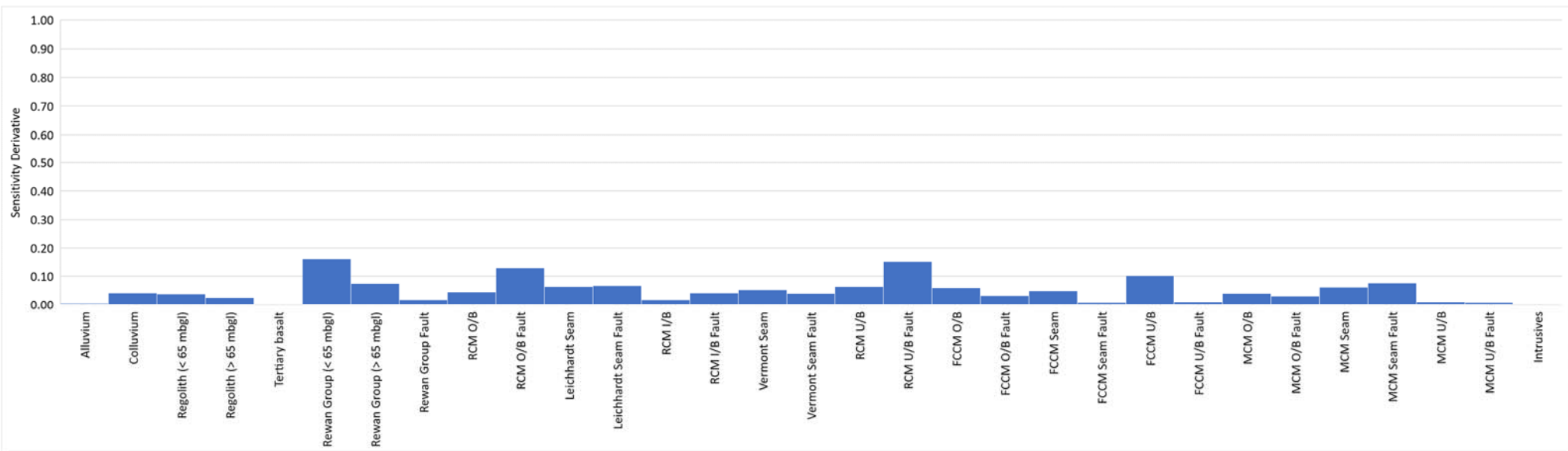


Figure F-5 Impacted Alluvium / Colluvium Drawdown Area (Layer 1) Sensitivity Derivatives – Anisotropy (Kv/Kx)

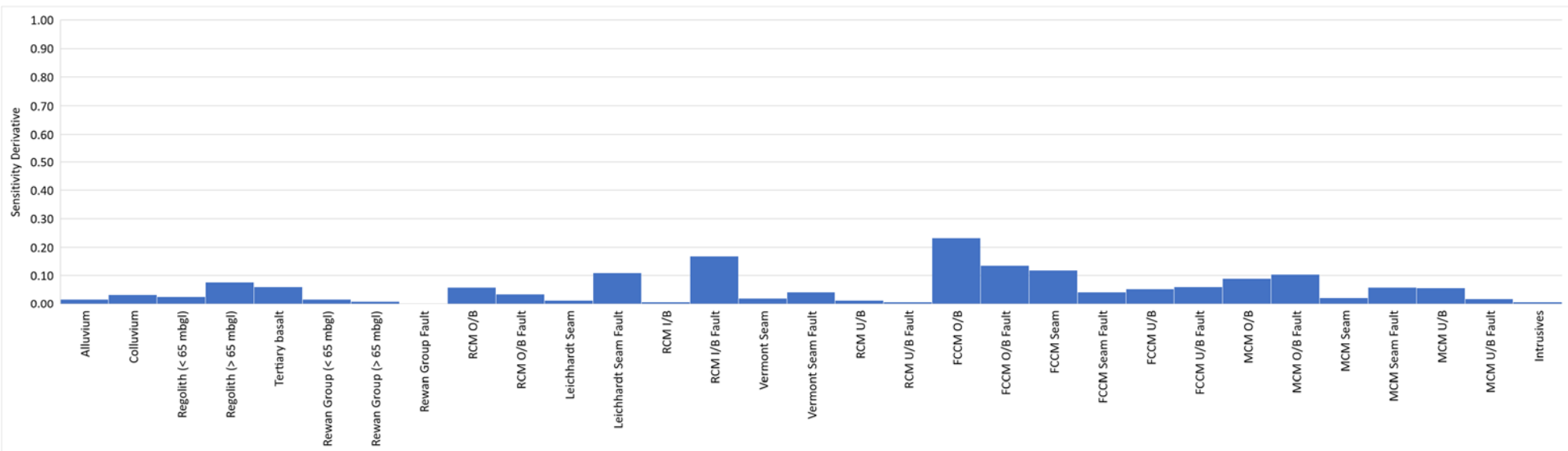


Figure F-6 Impacted Coal Seam Drawdown Area (Layer 7) Sensitivity Derivatives – Anisotropy (K_v/K_x)

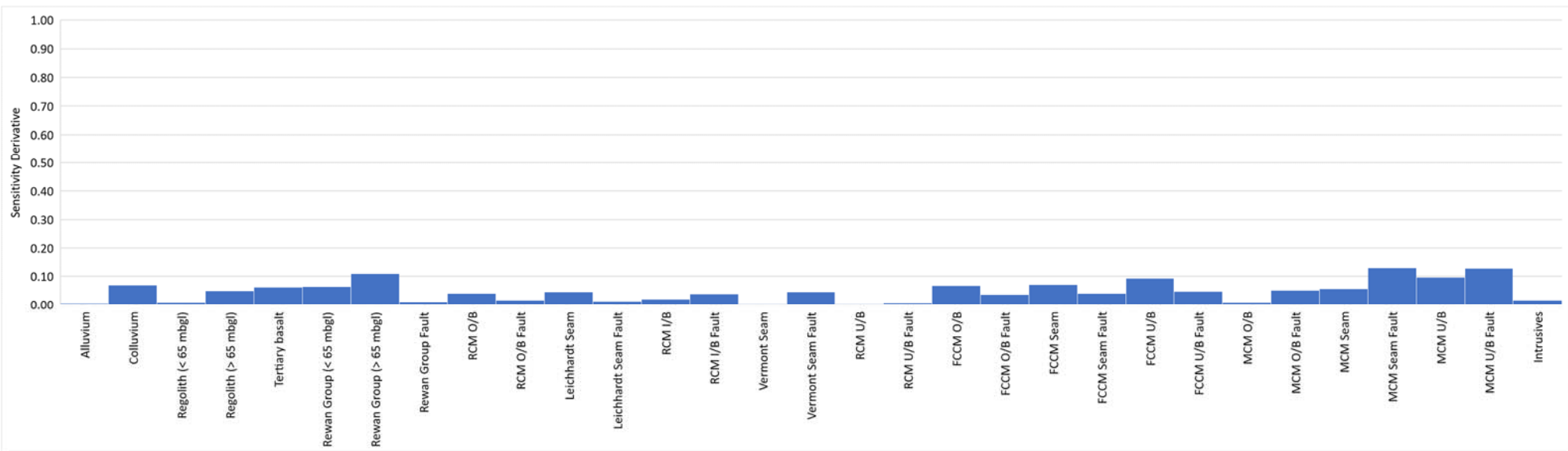


Figure F-7 Project Inflow Sensitivity Derivatives – Anisotropy (Kv/Kx)

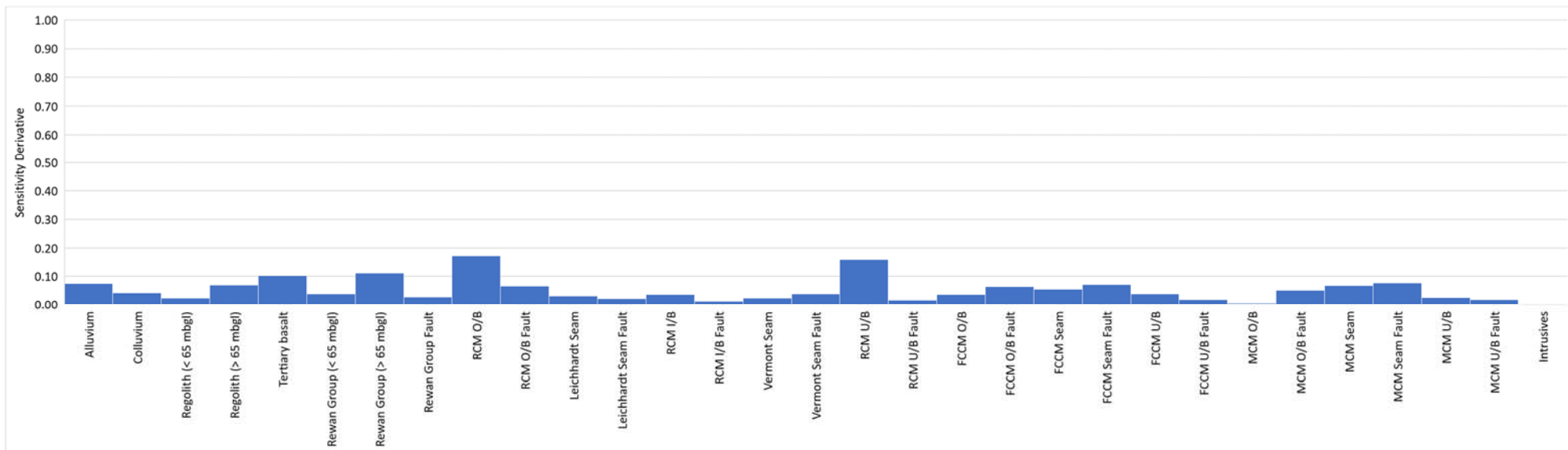


Figure F-8 Alluvial Drawdown Magnitude Sensitivity Derivatives – Anisotropy (K_v/K_x)

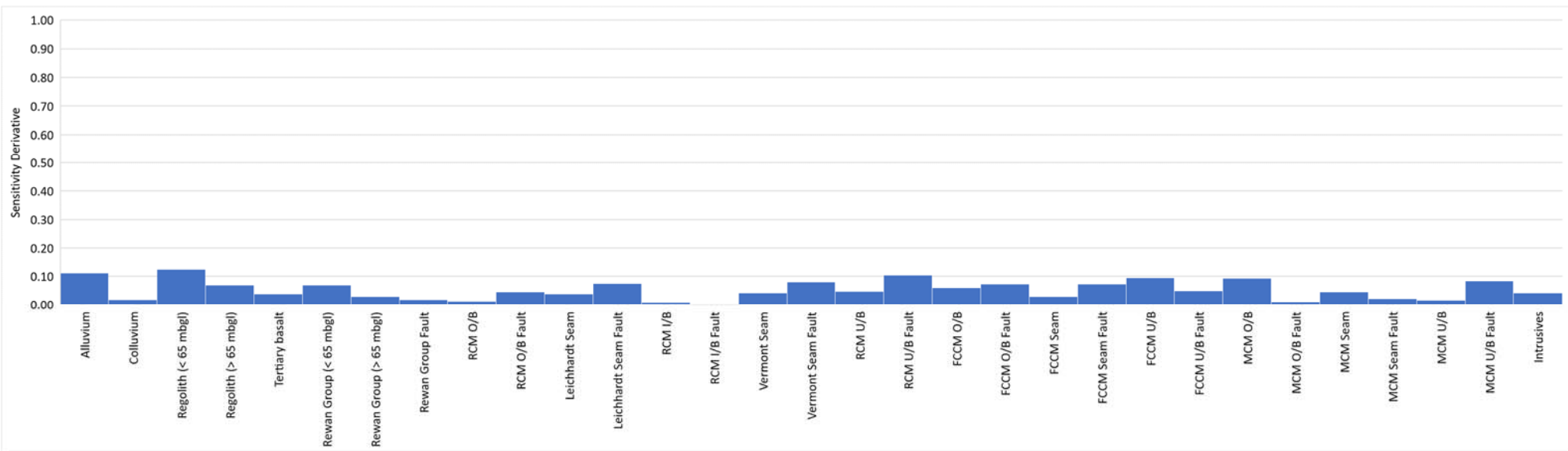


Figure F-9 Impacted Alluvium / Colluvium Drawdown Area (Layer 1) Sensitivity Derivatives – Specific Yield

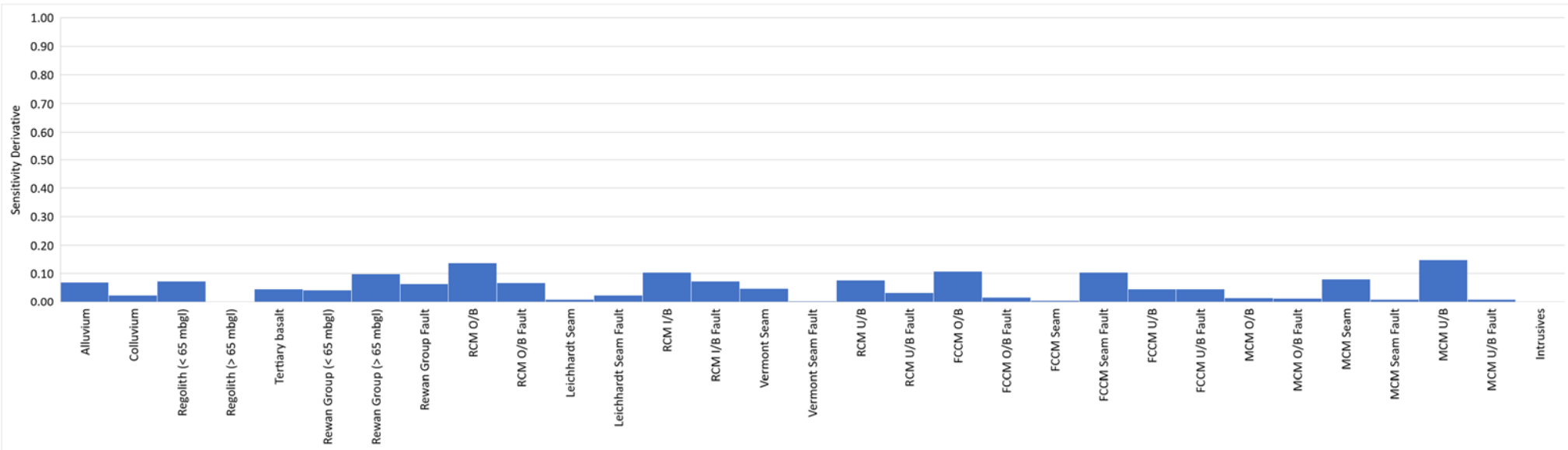


Figure F-10 Impacted Coal Seam Drawdown Area (Layer 7) Sensitivity Derivatives – Specific Yield

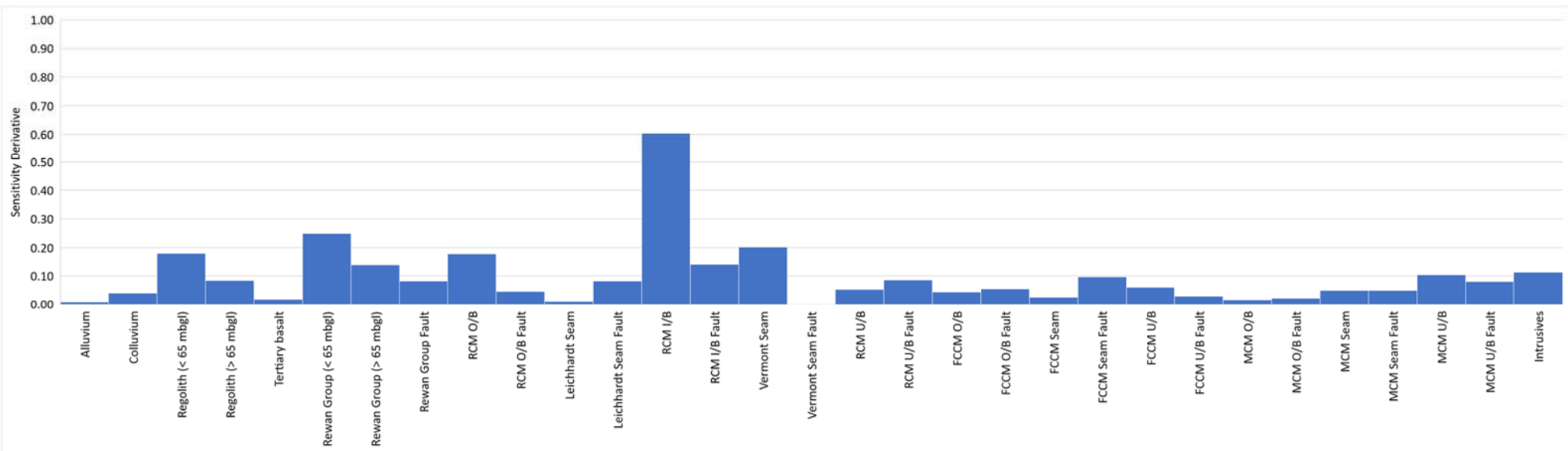


Figure F-11 Project Inflow Sensitivity Derivatives – Specific Yield

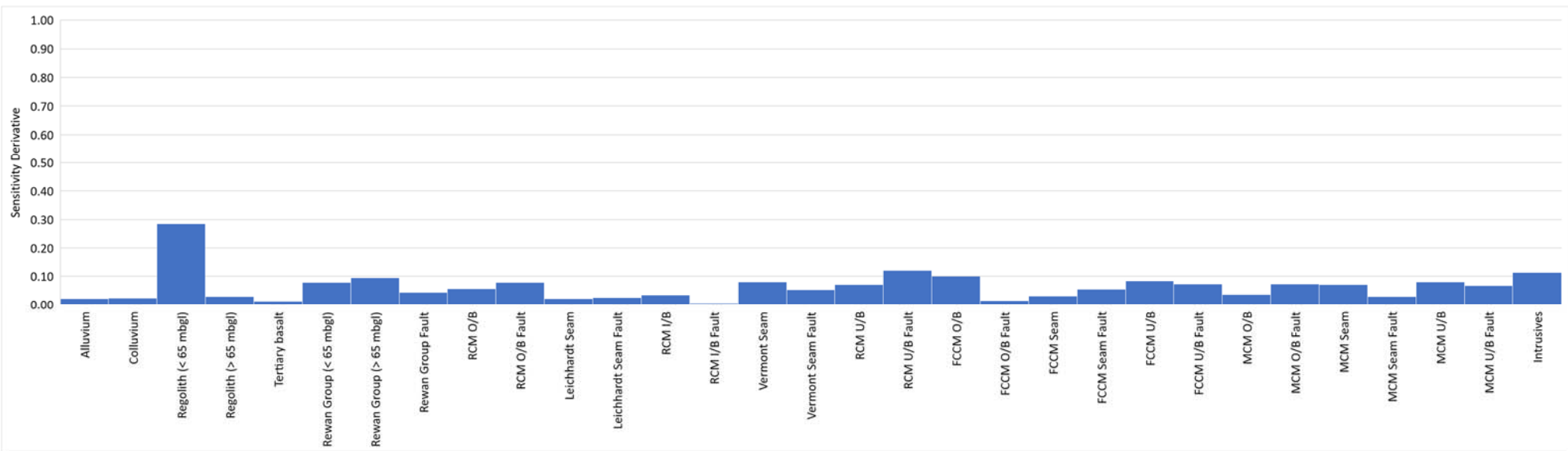


Figure F-12 Alluvial Drawdown Magnitude Sensitivity Derivatives – Specific Yield

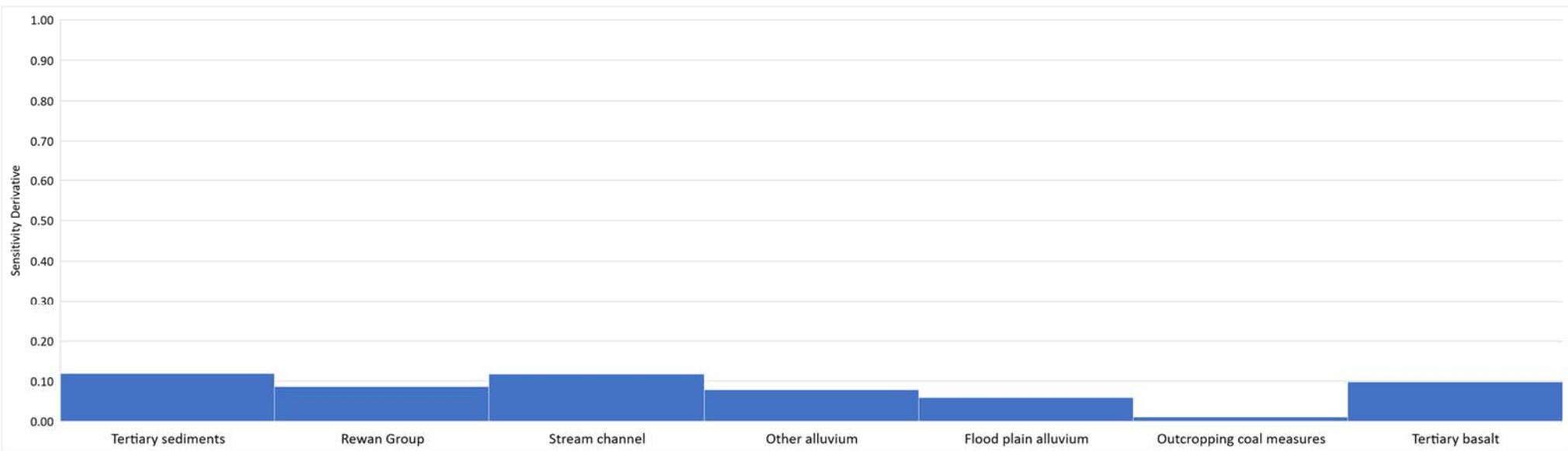


Figure F-13 Impacted Alluvium / Colluvium Drawdown Area (Layer 1) Sensitivity Derivatives – Recharge

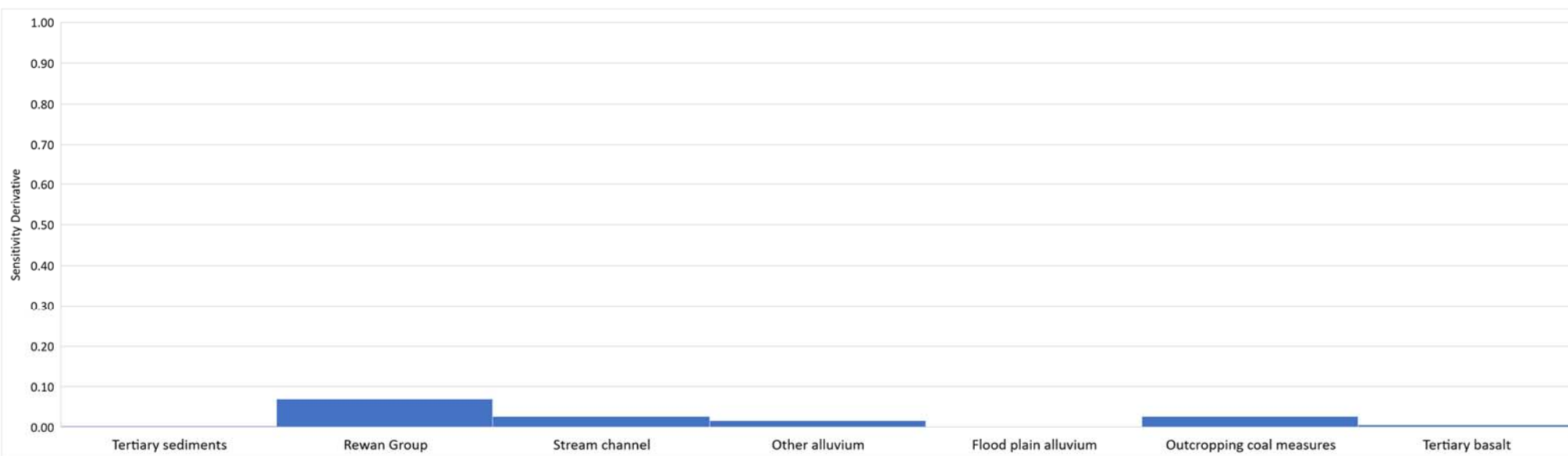


Figure F-14 Impacted Coal Seam Drawdown Area (Layer 7) Sensitivity Derivatives – Recharge

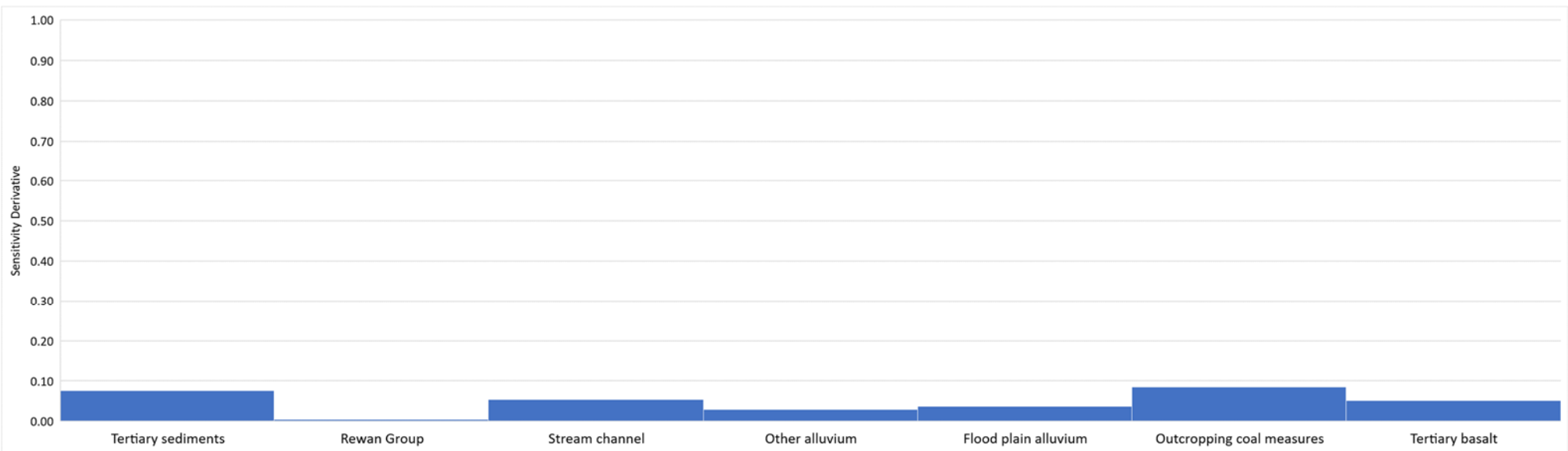


Figure F-15 Project Inflow Sensitivity Derivatives – Recharge

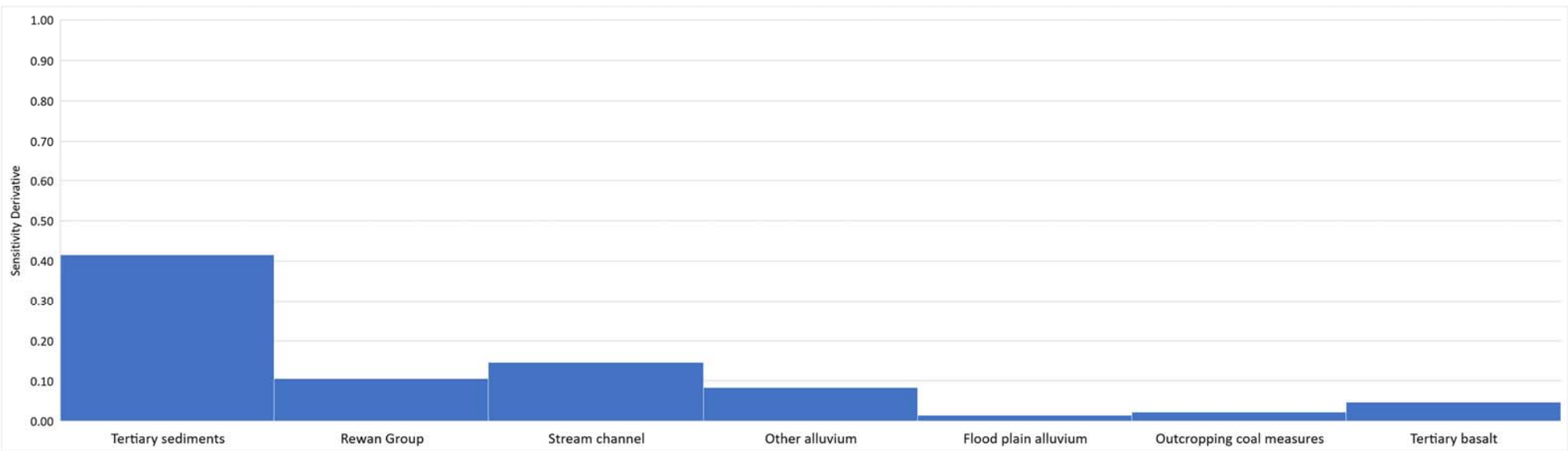


Figure F-16 Sensitivity Derivative – Recharge vs Alluvial Drawdown Magnitude

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